SMITHFIELD IRRIGATION COMPANY DIVERSION SYSTEM IMPROVEMENTS PROJECT

SMITHFIELD, UT

2025

SMITHFIELD IRRIGATION COMPANY

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Chris Chambers	Vice President
Dennis Thornley	Board Member
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ENGINEERING CONSULTANTS (SE)

Sunrise Engineering

Scott Archibald, P.E.	Principal Engineer

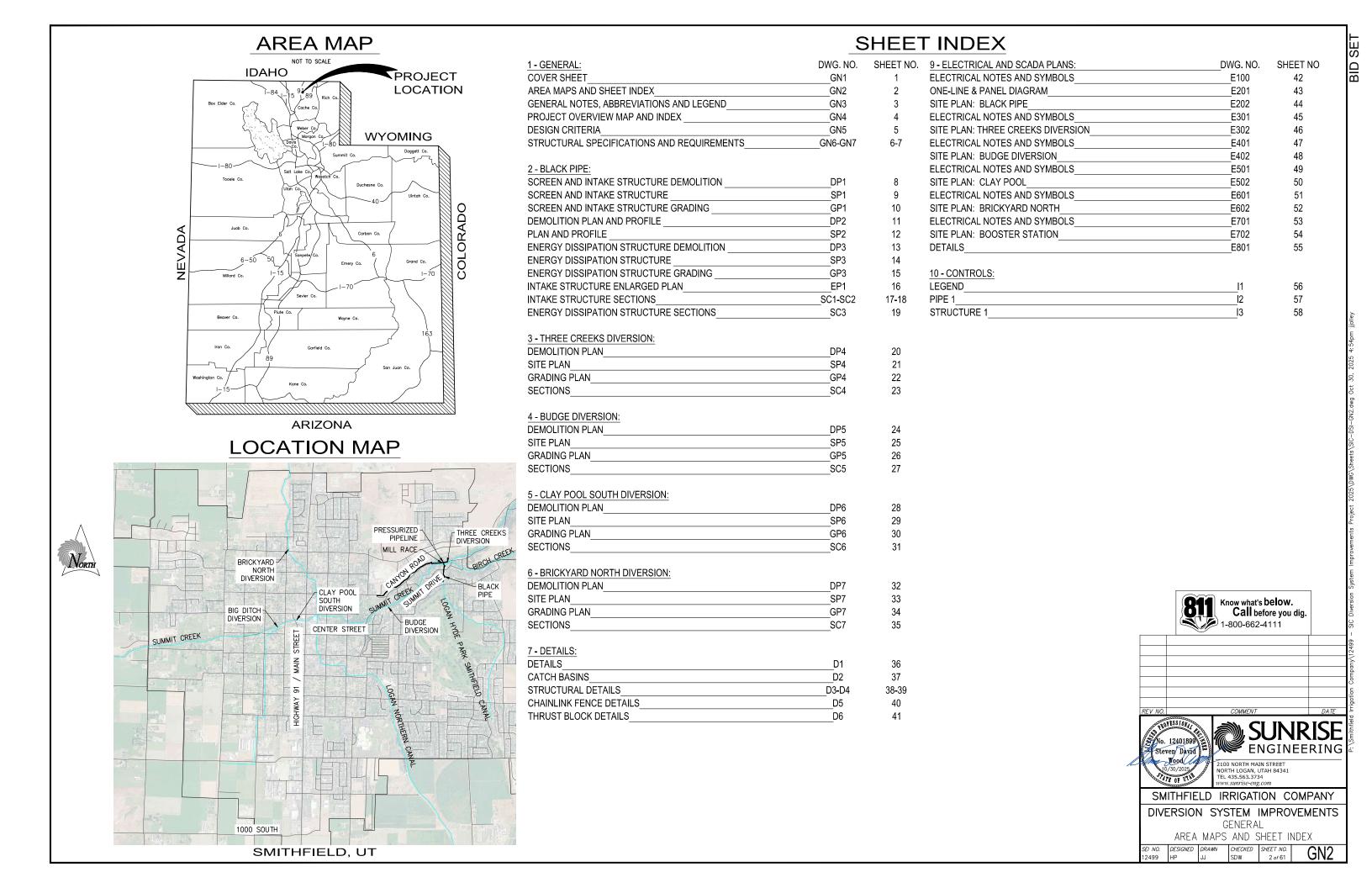
Steven Wood, P.E. Project Manager

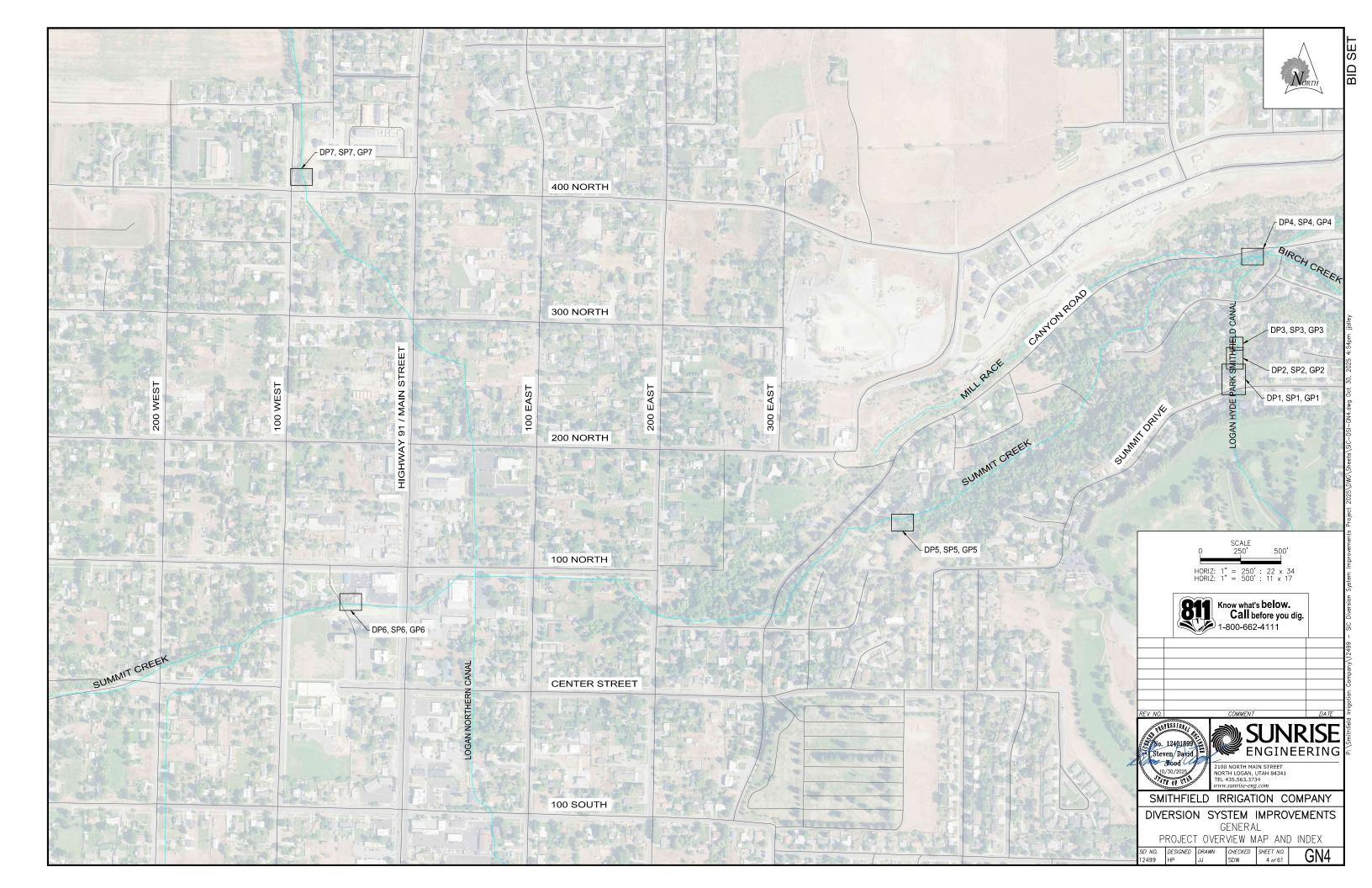
Haiming Peng, P.E. Project Engineer





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ROTARY DRUM SCREEN, SEDIMENT BASIN

0.005" (129 MICROS)

- SEDIMENT REMOVAL EFFICIENCY: 77.9%

BUDGE DIVERSION 13 CFS - DESIGN FLOW: - TREATMENT FACILITIES: OBERMEYER WEIR, SEDIMENT BASIN, WEDGE WIRE SCREEN OBERMEYER WEIR VENDOR: OBERMEYER HYDRO INC - DIMENSION: 16' L x 3' W x 3' H SETTLING BASIN - SETTLING BASIN DIMENSION: 25' L x 15' W x 3.5' D - TARGET REMOVED PARTICLE: 0.005" (124 MICROS) - SEDIMENT REMOVAL EFFICIENCY: 77.8% WEDGE WIRE SCREEN
- VENDOR: HENDRICK SCREEN COMPANY - SCREEN SURFACE AREA: 68.6 S.F. @ 29° - INTAKE DEMAND: 5834 GPM (13 CFS) - MAXIMUM FILTERED FLOW: 6189 GPM (13.8 CFS) - SAFETY FACTOR: 1.06 CLAY POOL SOUTH DIVERSION 6 CFS - DESIGN FLOW: - TREATMENT FACILITIES: SEDIMENT BASIN, WEDGE WIRE SCREEN SETTLING BASIN - SETTLING BASIN DIMENSION: 20' L x 8' W x 3.5' D - TARGET REMOVED PARTICLE: 0.005" (126 MICROS) - SEDIMENT REMOVAL EFFICIENCY: 69.9% WEDGE WIRE SCREEN HENDRICK SCREEN COMPANY

2500 GPM

33" - DRUM DIMENSION:

ROTARY DRUM SCREEN

- FILTER CAPACITY:

50 CFS

SHARPE CRESTED WEIR

HENDRICK SCREEN COMPANY

16' W x 6' H

30 S.F. @ 25°

1.5

1795 GPM (4 CFS)

2717 GPM (6.1 CFS)

2'

- DESIGN FLOW:

- WEIR TYPE:

- WEIR DIMENSION:

WEDGE WIRE SCREEN
- VENDOR:

– INTAKE DEMAND:

- SAFETY FACTOR:

- SCREEN SURFACE AREA:

- MAXIMUM FILTERED FLOW:

- FREE BOARD @ DESIGN FLOW:

SETTLING BASIN - SETTLING BASIN DIMENSION: 25' L x 10' W x 3.5' D

- TARGET REMOVED PARTICLE: 0.005" (127 MICROS)

- SEDIMENT REMOVAL EFFICIENCY: 94.6%

- SCREEN SURFACE AREA: - INTAKE DEMAND: 2693 GPM (6 CFS) - MAXIMUM FILTERED FLOW: 3609 GPM (8 CFS)

- SAFETY FACTOR:

40 S.F. @ 37°

1.34

Know what's below. **Call** before you dig. 1-800-662-4111

No. 12401899 ENGINEERING Steven David -Wood 2100 NORTH MAIN STREET

NORTH LOGAN, UTAH 84341 TEL 435.563.3734 SMITHFIELD IRRIGATION COMPANY

DIVERSION SYSTEM IMPROVEMENTS **GENERAL**

DESIGN CRITERIA SEI NO. DESIGNED DRAWN

CHECKED SHEET NO. GN5 SDW

DESIGN CRITERIA BLACK PIPE INTAKE STRUCTURE BLACK PIPE REHABILITATION BRICKYARD NORTH DIVERSION 50 CFS - DESIGN FLOW: - PIPE SIZE: 36" DESIGN FLOW: - EMERGENCY PEAK FLOW: 60 CFS - TREATMENT FACILITIES: - REHABILITATION METHOD: PVC FOLD-AND-FORM - CONCRETE ANCHOR WITH STEEL STRIPE: ROTARY DRUM SCREEN
- FILTER CAPACITY: TRAVELING WIRE MESH SCREEN INTERNATIONAL WATER SCREENS (IWS) - QUANTITY: VENDOR: - MESH OPENING: 6" WITH 79.6% CLEAR AREA - DIMENSION: 7' L x 2.5' T x 3' UNDERGROUND - DRUM DIMENSION: - SCREEN DIMENSIONS: 4' W @ 30° SETTLING BASIN - SETTLING BASIN DIMENSION: - HEADLOSS @ 50 CFS: 1.74" BLACK PIPE ENERGY DISSIPATION STRUCTURE
- ENERGY DISSIPATION TYPE: - TARGET REMOVED PARTICLE: BAFFLED BLOCKS EMERGENCY BYPASS WEIR - SEDIMENT REMOVAL EFFICIENCY: 79.3% - WEIR DIMENSIONS: 23' L x 2' H x 1' T - DIMENSION: - WATER DEPTH @ 50 CFS: 2 FT - CONCRETE CHUTE: 5' x 3' - WATER DEPTH @ 60 CFS: 3 FT TOP 9" LENGTH x 15" BOTTOM LENGTH x - CONCRETE BLOCK (TRAPEZOID): BIG DITCH DIVERSION

- DESIGN FLOW: 18" WIDTH x 12" HEIGHT 6 CFS SETTLING BASIN – DESIGN FLOW: 50 CFS - TREATMENT FACILITIES: 53' L x 6' D x 12' W (AVG) - BASIN DIMENSIONS: THREE CREEKS DIVERSION
- DESIGN FLOW: ROTARY DRUM SCREEN - FLOW VELOCITY: 0.69 FT / S - FILTER CAPACITY: 2500 GPM 5 CFS - HYDRAULIC RETENTION TIME: 76 S - DRUM DIMENSION: 33" - TREATMENT FACILITIES: OBERMEYER WEIR, ROTARY DRUM SCREEN, SEDIMENT BASIN - MINIMAL SETTLING VELOCITY: 0.08 FT / S OBERMEYER WEIR SETTLING BASIN - SETTING PARTICLE SIZE: 0.008" (202 MICROS) VENDOR: OBERMEYER HYDRO INC - SETTLING BASIN DIMENSION: 17' L x 8' W x 4' D REMOVAL EFFICIENCY: 68.6% - DIMENSION: 12' L x 3' W x 3' H - TARGET REMOVED PARTICLE: WEIR AT WEDGE WIRE SCREEN

- 2. RISK CATEGORY III PER IBC TABLE 1604.5
- DESIGN DEAD LOADS: CONCRETE = 150 PSE SOIL LADING PER IBC
- 4. LIVE LOAD: DECKING LL = 20 PSF
- 5. SNOW LOAD: PER ASCE 7-16 IMPORTANCE FACTOR Is = 1.5GROUND SL = 30 PSFFLAT/SLOPED ROOF SL USED FOR DESIGN = N/A
- WIND LOADING: N/A FOR BURIED STRUCTURE SEISMIC GOVERNS
- SEISMIC LOADING: PER ASCE 7-16 SITE CLASS D "STIFF SOIL", SEISMIC IMPORTANCE FACTOR $I_{e} = 1.00$ $S_S = 1.08g$ $S_1 = 0.32g$

 $S_{DS} = 0.83q$ $S_{D1} = 0.53g$

SFISMIC DESIGN CATEGORY D PER ASCE 7-16 TABLE 11.6-2 LATERAL FORCE RESISTING SYSTEM IS AS FOLLOWS:

- SPECIAL REINFORCED CONCRETE SHEAR WALLS
- A.7 SPECIAL REINFORCED MASONRY SHEAR WALLS
- A.15 LIGHT-FRAME (WOOD) WALLS SHEATHED WITH WOOD STRUCTURAL PANELS RATED FOR SHEAR RESISTANCE
- A.16 LIGHT-FRAME (COLD-FORMED STEEL) WALLS SHEATHED WITH WOOD STRUCTURAL PANELS RATED FOR SHEAR RESISTANCE OR STEEL SHEETS

GENERAL REQUIREMENTS:

- DIMENSIONS: CONTRACTOR TO VERIFY ALL DIMENSIONS ON SITE PRIOR TO CONSTRUCTION AND REPORT ANY DISCREPANCIES TO THE ENGINEER OF RECORD.
- THE CONTRACTOR MUST SUBMIT IN WRITING ANY REQUESTS FOR MODIFICATIONS TO THE PLANS AND SPECIFICATIONS. SHOP DRAWINGS SUBMITTED TO THE STRUCTURAL ENGINEER FOR REVIEW DO NOT CONSTITUTE "IN WRITING" UNLESS IT IS CLEARLY NOTED THAT SPECIFIC CHANGES ARE
- 3. LOADS FROM CONSTRUCTION MATERIALS SHALL BE SPREAD OUT IF PLACED ON FRAMED FLOORS OR ROOFS. CONSTRUCTION LOADS SHALL NOT EXCEED THE DESIGN LIVE LOAD PER SQUARE FOOT LISTED IN THE DESIGN CRITERIA. CONTRACTOR SHALL PROVIDE ADEQUATE SHORING AND OR BRACING WHERE STRUCTURE HAS NOT ATTAINED DESIGN STRENGTH.
- THESE DOCUMENTS HAVE BEEN PREPARED USING STANDARDS OF PROFESSIONAL CARE AND COMPLETENESS AS REQUIRED FOR THIS OR SIMILAR LOCALITIES. THEY ASSUME THAT THE WORK DEPICTED WILL BE PERFORMED BY AN EXPERIENCED CONTRACTOR AND/OR WORKMEN WHO HAVE A WORKING KNOWLEDGE OF THE APPLICABLE CODE STANDARDS AND REQUIREMENTS AND OF INDUSTRY ACCEPTED STANDARD GOOD PRACTICE. AS NOT EVERY CONDITION OR ELEMENT IS (OR CAN BE) EXPLICITLY SHOWN ON THESE DRAWINGS, THE CONTRACTOR SHALL USE INDUSTRY ACCEPTED STANDARD GOOD PRACTICE FOR MISCELLANFOUS WORK NOT EXPLICITLY SHOWN.
- THESE DRAWINGS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR CONSTRUCTION MEANS. METHODS. TECHNIQUES. SEQUENCES. PROCEDURES. LAGGING. SHORING. BRACING. FORM-WORK. ETC. AS REQUIRED FOR THE PROTECTION OF LIFE AND PROPERTY DURING CONSTRUCTION.

C. FOUNDATION REQUIREMENTS:

FOLINDATIONS FOR THE STRUCTURES SHOWN IN THE PLANS WERE DESIGNED BASED ON THE RECOMMENDATIONS OF THE INTERNATIONAL BUILDING CODE 2021, CHAPTER 18. LOAD-BEARING VALUES OF THE SOILS ARE PROVIDED IN THE CODE. CONTINUOUS AND SPREAD FOOTINGS DESIGNED FOR ALLOWABLE BEARING PRESSURE OF 1,500 PSF PER THE CODE. IF SOIL CONDITIONS ARE FOUND TO BE DIFFERENT THAN THE TYPES LISTED ABOVE OR OF UNUSUAL MAKE-UP, OR SUB-STANDARD, PLEASE CONTACT THE ENGINEER OF RECORD.

D. CONCRETE REQUIREMENTS:

ALL CONCRETE CONSTRUCTION SHALL BE PERFORMED IN ACCORDANCE WITH ACI 318 AND ACI 301, EXCEPT AS MODIFIED BY THE CONSTRUCTION DOCUMENTS

W /O DATIO

CONCRETE SHALL HAVE THE FOLLOWING COMPRESSIVE STRENGTHS:

MIN. TC (28 DAYS)	SLUMP	W/C RATIO	CLASS
4500 PSI	3" TO 5"	0.45	F2
4000 PSI	3" TO 5"	0.45	F2
4500 PSI	3" TO 5"	0.45	F2
	4500 PSI 4000 PSI	4500 PSI 3" TO 5" 4000 PSI 3" TO 5"	4500 PSI 3" TO 5" 0.45 4000 PSI 3" TO 5" 0.45

SLUMP: CONCRETE w/ ADMIXTURES SHALL HAVE A MAXIMUM SLUMP OF 7".

ADMIXTURES:

- 3.1. AIR ENTRAINMENT ASTM C-260
- CALCIUM CHLORIDE NOT PERMITTED
- ALUMINUM PRODUCTS NOT PERMITTED 3.3
- CONCRETE MIXES SHALL BE DESIGNED BY A CERTIFIED LABORATORY, STAMPED BY AN APPROPRIATELY LICENSED SPECIALTY ENGINEER, AND APPROVED BY THE ENGINEER OF RECORD. MIX DESIGNS SHALL INCLUDE THE PROJECT NAME AND INDICATE THEIR USE WITHIN THE STRUCTURE. MIX DESIGNS SHALL BE PROPORTIONED TO MINIMIZE SHRINKAGE AND HAVE PROVEN. SHRINKAGE CHARACTERISTICS OF 0.05% OR LESS BASED ON TESTING PER ASTM C157.
- IF USED, EARLY STRENGTH CONCRETE SHALL BE PROPORTIONED TO DEVELOP THE 28 DAY COMPRESSIVE STRENGTH AT THE AGE REQUIRED BY THE CONTRACTOR. CONTRACTOR SHALL SUBMIT TEST DATA FOR REVIEW BY THE STRUCTURAL ENGINEER TO SUBSTANTIATE THE CONCRETE STRENGTH AT THE REQUIRED AGE.
- 6. ALL CONCRETE SHALL BE NORMAL WEIGHT OF 145 POUNDS PER CUBIC FOOT USING HARD ROCK AGGREGATES CONFORMING TO ASTM C33 U.N.O. WHERE LIGHTWEIGHT CONCRETE IS SPECIFIED, CONCRETE SHALL BE 110 POUNDS PER CUBIC FOOT USING AGGREGATES CONFORMING TO ASTM C330. LARGEST NOMINAL AGGREGATE SIZE SHALL BE 1-1/2" OR GREATER FOR SLABS ON GRADE AND 3/4" OR GREATER FOR ALL OTHER CONCRETE U.N.O.
- CEMENT SHALL CONFORM TO THE "STANDARD SPECIFICATION FOR PORTLAND CEMENT," ASTM C 150, TYPE II LOW-ALKALI OR ASTM C595 "STANDARD SPECIFICATION FOR BLENDED HYDRAULIC CEMENT"
- 8. FLY ASH MAY BE USED IN CONCRETE, SUBJECT TO APPROVAL BY THE ARCHITECT AND ENGINEER, PROVIDED THE FOLLOWING CONDITIONS ARE MET:
 - 8.1. FLY ASH SHALL COMPLY WITH ASTM C618.
 - CEMENT CONTENT SHALL BE REDUCED A MINIMUM OF 15 PERCENT UP TO A MAXIMUM OF 25 PERCENT WHEN COMPARED TO AN EQUIVALENT CONCRETE MIX DESIGN WITHOUT FLY ASH. FLY ASH CONTENT SHALL NOT COMPRISE MORE THAN 35 PERCENT OF THE TOTAL CEMENTITIOUS CONTENT. THE WATER-CEMENT RATIO SHALL BE CALCULATED BASED ON
 - 8.3. CLASS F FLY ASH SHALL BE USED IN SULFATE RESISTANT CONCRETE WITH f'c EQUAL TO OR GREATER THAN 4000 PSI. CLASS C FLY ASH MAY BE USED ELSEWHERE.
- WATER SOLUBLE CHLORIDE ION CONCENTRATIONS IN CONCRETE SHALL BE LIMITED PER ACI 318. SECTION 4.4.
- 10. ALL CONCRETE EXPOSED TO FREEZE/THAW CYCLES OR DEICING CHEMICALS SHALL CONFORM TO ACI 318, SECTION 4.2.
- 11. TIME BETWEEN CONCRETE BATCHING AND PLACEMENT SHALL BE IN ACCORDANCE WITH ASTM C94.
- 12. CONCRETE MIXING, PLACEMENT, AND QUALITY SHALL BE PER IBC SECTION 1905. MECHANICALLY VIBRATE ALL CONCRETE WHEN PLACED. SLABS ON GRADE NEED TO BE VIBRATED ONLY AROUND AND UNDER FLOOR DUCTS OR SIMILAR ELEMENTS. REMOVE ALL DEBRIS FROM FORMS BEFORE PLACING CONCRETE. CONCRETE SHALL NOT BE DROPPED THROUGH REINFORCING STEEL SO AS TO CAUSE SEGREGATION OF AGGREGATES. UNCONFINED FALL OF CONCRETE SHALL NOT EXCEED 5
- 13. PROTECT CONCRETE FROM DAMAGE OR REDUCED STRENGTH DUE TO COLD OR HOT WEATHER IN ACCORDANCE WITH ACI 305 AND 306. CONTRACTOR SHALL TAKE SPECIAL CURING PRECAUTIONS
- 14. ALL REINFORCING STEEL SHALL BE SET AND TIED IN PLACE PRIOR TO POURING OF CONCRETE. FXCEPT VERTICAL DOWELS FOR MASONRY WALL REINFORCING MAY BE "FLOATED" IN PLACE. DO NOT FIELD BEND BARS PARTIALLY EMBEDDED IN HARDENED CONCRETE UNIESS SPECIFICALLY NDICATED OR APPROVED BY THE ENGINEER OF RECORD.
- 15. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE PLACEMENT AND LOCATION OF ANY AND ALL EMBED ITEMS INCLUDING PLATES, BOLTS, AND OTHER INSERTS SPECIFIED IN THE DRAWINGS.
- 16. ALL ITEMS TO BE CAST IN CONCRETE SUCH AS REINFORCEMENT, DOWELS, BOLTS, ANCHORS, SLEEVES, ETC., SHALL BE SECURELY POSITIONED IN THE FORMS.

D. CONCRETE REQUIREMENTS CONTINUED:

- 17. MECHANICAL, ELECTRICAL, AND PLUMBING PENETRATIONS / EMBEDDED CONDUITS SHALL COMPLY
 - 17.1. ELECTRICAL CONDUITS MAY BE EMBEDDED IN STRUCTURAL CONCRETE ONLY AS NOTED IN TYPICAL DETAILS FOR WALLS AND CAST-IN-PLACE ELEVATED SLABS (EMBEDDED CONDUITS IN CONCRETE OVER STEEL DECK ARE NOT PERMITTED) OR WHERE SPECIFICALLY APPROVED IN WRITING BY THE ENGINEER PIPING SHALL NOT BE EMBEDDED IN STRUCTURAL CONCRETE U.N.O. EMBEDDED ITEMS SHALL NOT IMPAIR THE
- 17.2. REFER TO TYPICAL DETAILS FOR ACCEPTABLE CONDUIT, PIPING, AND DUCT PENETRATIONS THRU SLABS AND WALLS. DO NOT CUT ANY REINF. THAT MAY INTERFERE WITH PERMITTED PENETRATIONS. OPENINGS SHALL NOT BE CORED WITHOUT PRIOR WRITTEN APPROVAL OF ENGINEER. PENETRATIONS THRU BEAMS AND COLUMNS ARE PERMITTED ONLY WHERE SPECIFICALLY DETAILED.
- 17.3. CONTRACTOR SHALL SUBMIT SHOP DRAWING SHOWING SIZES AND DIMENSIONED LOCATIONS OF ALL PENETRATIONS AND EMBEDDED CONDUITS IN WALLS AND ELEVATED SLABS. SHOP DRAWING MUST BE APPROVED BY ENGINEER PRIOR TO CONCRETE PLACEMENT. PENETRATIONS AND EMBEDDED CONDUITS NOT SHOWN ON APPROVED SHOP DRAWING WILL NOT BE PERMITTED UNLESS SPECIFICALLY APPROVED IN WRITING BY THE
- 18 FORMWORK SHORING AND RESHORING SHALL BE DESIGNED PER ACL 347 RECOMMENDATIONS BY AN APPROPRIATELY LICENSED SPECIALTY ENGINEER EXPERIENCED IN THIS TYPE OF WORK AND SHALL BE SUBMITTED TO ENGINEER OF RECORD FOR REVIEW. FOR MULTISTORY CONSTRUCTION, SHORING/RESHORING DESIGN SHALL DEMONSTRATE THAT SHORES/RESHORES WILL BE PROVIDED FOR A SUFFICIENT NUMBER OF FLOORS TO DISTRIBUTE IMPOSED CONSTRUCTION LOADS TO SEVERAL SLAB LEVELS WITHOUT CAUSING EXCESSIVE STRESSES AND SLAB DEFLECTIONS. FOR PURPOSES OF SHORING/RESHORING CALCULATIONS, MAGNITUDES OF REDUCED LIVE LOADS SHALL BE TAKEN TO BE 60% OF VALUES INDICATED IN BASIS FOR DESIGN U.N.O.
- 19. CONSTRUCTION JOINTS OR POUR JOINTS IN STRUCTURAL ELEMENTS (BEAMS, COLUMNS, ELEVATED SLABS, ETC.) NOT SPECIFICALLY SHOWN OR NOTED ON THE DRAWINGS REQUIRE PRIOR APPROVAL OF THE ENGINEER. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS SHOWING PROPOSED JOINTS TO
- 20. CONSTRUCTION JOINT SURFACES SHALL BE CLEANED AND LAITANCE REMOVED. HORIZONTAL JOINT SURFACES SHALL BE ROUGHENED TO 1/4" AMPLITUDE. THOROUGHLY WET ALL JOINT SURFACES AND REMOVE STANDING WATER IMMEDIATELY PRIOR TO NEW CONCRETE PLACEMENT.
- CONCRETE SHALL BE CURED IN ACCORDANCE WITH ACL 318, SECTIONS 5.11.1 OR 5.11.2 WHICHEVER IS APPLICABLE, UNLESS ALTERNATE METHODS HAVE BEEN APPROVED BY THE ARCHITECT AND ENGINEER. WHERE CURING COMPOUNDS HAVE BEEN APPROVED FOR SLAB CURING. CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING COMPATIBILITY OF COMPOUNDS WITH ANTICIPATED FLOOR FINISH (e.g., RESILIENT TILE) PRIOR TO CURING COMPOUND APPLICATION.

22. CONCRETE FINISH DESIGNATION:

- F1 AS CAST FORM FINISH F2 - ROUGH FINISH
- SMOOTH FINISH
- SMOOTH RUBBED FINISH
- F5 GROUT CLEANED RUBBED FINISH
- F6 CORK FLOATED RUBBED FINISH
- UNFORMED FINISH FR - RIASTED FINISH
- F9 ARCHITECTURAL FINISH
- F10 TOOLED FINISH
- S1 FLOATED FINISH
- S2 TROWEL FINISH
- S3 BROOM FINISH
- EXPOSED AGGREGATE FINISH S5 - CHEMICAL HARDENER FINISH

REINFORCING STEEL REQUIREMENTS:

STRUCTURAL SLABS, WALLS, JOISTS

NO. 11 AND SMALLER

- REINFORCING STEEL SHALL BE DETAILED AND PLACED IN ACCORDANCE WITH ACI 318 AND CRSI'S MANUAL
- REINFORCING STEEL SHALL CONFORM TO ASTM A615 OR ASTM A706 (A706 REQUIRED FOR ALL REINFORCING TO BE WELDED) AND SHALL BE GRADE 60 (fy = 60 KSI) DEFORMED BARS U.N.O. REINFORCING IN SLABS ON GRADE MAY BE GRADE 40 (fy = 40 KSI) DEFORMED BARS FOR ALL BARS #4 AND SMALLER U.N.O. ON
- ALL DIMENSIONS SHOWING THE LOCATION OF REINFORCING STEEL NOT NOTED AS "CLEAR" OR "CLR." ARE TO CENTER OF STEEL. CLEAR COVER FOR NON-PRESTRESSED CONCRETE REINFORCING SHALL BE AS NOTED BELOW, U.N.O. ON PLANS OR DETAILS. CLEAR COVER FOR PRESTRESSED CONCRETE AND FOR PRECAST CONCRETE MANUFACTURED UNDER PLANT CONTROL CONDITIONS SHALL BE PER ACI 318, SECTIONS 7.7.2 AND 7.7.3. RESPECTIVELY.

EXPOSURE CONDITION: COVER CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH EXPOSED TO WEATHER (INCLUDES SLABS ON GRADE) NO 5 AND SMALLER 1 1/2" NO. 6 AND LARGER NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND

LAP SPLICES OF REINFORCING STEEL SHALL CONFORM TO TYPICAL REBAR LAP SCHEDULE U.N.O. NO TACK WELDING OF REINFORCING BARS ALLOWED. LATEST ACI CODE AND DETAILING MANUAL APPLY. AT WALLS AND FOOTINGS, PROVIDE BENT CORNER BARS TO MATCH AND LAP WITH HORIZ. BARS AT ALL CORNERS AND INTERSECTIONS U.N.O. VERT, WALL BARS SHALL BE SPLICED AT OR NEAR FLOOR LINES. SPLICE TOP BARS AT CENTER LINE OF SPAN AND BOTTOM BARS AT THE SUPPORT IN SPANDRELS, BEAMS, GRADE BEAMS, ETC., U.N.O. ON PLANS OR DETAILS.

E. REINFORCING STEEL REQUIREMENTS CONTINUED:

- MECHANICAL SPLICE COUPLERS SHALL HAVE CURRENT ICC APPROVAL AND SHALL BE CAPABLE OF DEVELOPING 125% OF THE SPLICED BAR'S YIELD STRENGTH.
- 6. ALL REINFORCING SHALL BE BENT COLD. BARS SHALL NOT BE UN-BENT AND RE-BENT. FIELD BENDING OF REBAR SHALL NOT BE ALLOWED UNLESS SPECIFICALLY NOTED.
- WELDING OF REINFORCING BARS. METAL INSERTS. AND CONNECTIONS SHALL BE MADE ONLY AT LOCATIONS SHOWN ON PLANS OR DETAILS. SEE WELDING SECTION OF G.S.N. FOR ADDITIONAL
- REINFORCING BAR SPACINGS SHOWN ON PLANS ARE MAX. ON CENTER DIMENSIONS. DOWEL ALL VERT. REINFORCING TO FOUNDATION. SECURELY TIE ALL BARS IN LOCATION BEFORE PLACING CONCRETE. MIN. CLEAR SPACING BETWEEN PARALLEL REINFORCEMENT SHALL BE THE LARGER OF 1-1/2 TIMES NOMINAL BAR DIA. OR 1-1/3 TIMES MAX. AGGREGATE SIZE OR 1-1/2". CLEAR SPÁCING LIMITATION APPLIES ALSO TO CLEAR DISTANCE BETWEEN A CONTACT LAP SPLICE AND ADJACENT SPLICES OR BARS.
- MIN. REINFORCING AT EDGES OF CONCRETE WALL OPENINGS SHALL BE (2) #5 BARS. EXTEND THE GREATER OF THE DEVELOPMENT LENGTH OF THE BAR PER TYPICAL REBAR LAP SCHEDULE OR 24" MIN. PAST EDGES OF OPENING U.N.O. HOOK ENDS AT INTERFERENCE WITH EXTENSION.

G. <u>EARTHWORK REQUIREMENTS:</u>

- 1. CONSULT THE CURRENT ADOPTED BUILDING CODE FOR RECOMMENDED EARTHWORK REQUIREMENTS.
- 2. ALL SOFT SPOTS SHALL BE REMOVED AND REPLACED WITH COMPACTED STRUCTURAL FILL.
- THE IN-PLACE DRY DENSITY OF THE COMPACTED STRUCTURAL FILL SHALL NOT BE LESS THAN 90 PERCENT OF THE MAXIMUM DRY DENSITY AT OPTIMUM MOISTURE CONTENT. THE CONTRACTOR SHALL HAVE ALL FILL TESTED.
- 4. ALL STRUCTURES SHALL HAVE STRUCTURAL FILL PLACED BENEATH FOOTINGS/SLABS.
- 5. ALLOWABLE DEAD PLUS LIVE LOAD SOIL PRESSURE = 1,500 PSF (ASSUMED).
- 6. TRENCHES AND EXCAVATIONS UNDER OR ADJACENT TO FOUNDATIONS SHALL BE PROPERLY
- MITIGATE SOIL CORROSION POTENTIAL ON METAL CONSTRUCTION MATERIALS. CONSULT A QUALIFIED CORROSION ENGINEER FOR RECOMMENDATIONS FOR MITIGATING CORROSIVE EFFECTS, IF
- 8. WATER PROOFING AS MAY BE REQUIRED AT SOIL FACE OF WALLS BELOW GRADE SHALL BE BY



REV NO.		COMMENT	DATE
No.	12401899 ven David	SUNR ENGINEE	ISE RING

NORTH LOGAN, UTAH 84341 STIFF OF UTAIL TEL 435 563 3734 SMITHFIELD IRRIGATION COMPANY

DIVERSION SYSTEM IMPROVEMENTS GENERAL

STRUCTURAL SPECIFICATIONS AND REQUIREMENTS SEI NO. DESIGNED DRAWN CHECKED SHEET NO.

SDW

K. EPOXY DOWEL REQUIREMENTS:

- LISE SIMPSON SET-XP OR SET-3G FPOXY ANCHORING SYSTEMS (OR APPROVED FOLIAL) FOR ANY EPOXY ANCHORING INTO CONCRETE, U.N.O ON PLANS OR IN DETAILS. USE SIMPSON SET EPOXY ANCHOR SYSTEM FOR ANY EPOXY ANCHORING INTO MASONRY.
- HOLES SHALL BE DRILLED TO THE REQUIRED DEPTHS LISTED IN THE EPOXY ANCHOR SCHEDULE USING ROTARY HAMMER DRILLS AND CARBIDE-TIPPED DRILL BITS, COMPLYING W/ ANSI
- 3. USE MINIMUM GRADE A307 GRADED THREADED ANCHORS U.N.O., AND ALL ROD THREADS MUST COMPLY W/ ANSI B1.1-74. THE MINIMUM BASE MATERIAL INSTALLATION TEMPERATURE FOR THESE SYSTEMS IS 40° FAHRENHEIT
- AFTER DRILLING, ALL HOLES MUST BE CLEANED FROM THE BOTTOM W/ FORCED AIR. A WIRE BRUSH SHALL THEN BE USED TO CLEAN DUST AND SLURRY FROM THE HOLE. FOLLOW THIS BY ANOTHER CLEANING W/ FORCED AIR, ENSURING THAT THERE ARE NO LOOSE DEBRIS NOR MATERIALS IN THE HOLÉ.
- 5. ADHESIVE SHALL THEN BE PUMPED INTO THE HOLE AND ANCHORS OR BARS INSTALLED, PER MANUFACTURER'S SPECIFICATIONS. ANCHORS OR BARS SHALL BE INSERTED BY ROTATING A MINIMUM OF TWO TURNS TO EVENLY DISTRIBUTE EPOXY.
- CURE TIMES SHALL BE DETERMINED PER MANUFACTURER'S RECOMMENDATIONS. REFER TO RESPECTIVE ICC EVALUATION REPORTS.
- PER IBC REQUIREMENTS, SPECIAL INSPECTION IS REQUIRED FOR ALL ADHESIVE ANCHOR INSTALLATIONS. THE SPECIAL INSPECTOR SHALL RECORD AND SUBMIT COMPLIANCE OF THE
 - DRILL BIT TYPE AND SIZE, PER ANSI B212,15-1994
 - 7.2. HOLE DEPTH AND CLEANLINESS
 - RODUCT DESCRIPTION, INCLUDING PRODUCT NAME, ROD DIAMETER, AND LENGTH
 - 7.4. ADHESIVE EXPIRATION DATE
 - VERIFICATION OF ANCHOR INSTALLATION PER THE MANUFACTURER'S PUBLISHED
- 8. MINIMUM EDGE, END, AND SPACING DISTANCES PER MANUFACTURER'S RECOMENDATIONS, AND SEI'S

STRUCTURAL STEEL REQUIREMENTS:

STRUCTURAL STEEL SHALL HAVE THE FOLLOWING MATERIALS UNLESS NOTED OTHERWISE:

WIDE FLANGE SECTIONS ASTM A992 (50 ksi) OTHER ROLLED SHAPES AND PLATES ASTM A36

ASTM A307 WITH ASTM A563 HEAVY HEX HEADED STUD ANCHORS (HAS)

DEFORMED BAR ANCHORS (DBA) ASTM A496 BOLTED CONNECTIONS ASTM A325 ASTM A36, A193 GR, B7

ANCHOR BOLTS ASTM F1554 GR. 36 OR 55 ASTM A563 (HEAVY HEX) NUTS WASHERS ASTM F436 GRADE A (HARDENED) OR A36 AT ANCHOR RODS

2. THE FABRICATION AND CONSTRUCTION OF ALL STRUCTURAL STEEL SHALL COMPLY WITH THE LATEST EDITION OF THE FOLLOWING CODES:

IBC 2021 SECTION 2205

AISC SPECIFICATIONS FOR THE DESIGN. FABRICATION AND ERECTION OF STRUCTURAL STEEL

AISC CODE OF STANDARD PRACTICE EXCLUDING SECTIONS 3.4, 4.4, AND 4.4.1.

AISC SPECIFICATIONS FOR STRUCTURAL JOINTS. AWS WELDING CODE.

AISC SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS.

PROVIDE SHOP DRAWINGS FOR REVIEW PRIOR TO FABRICATION. ALL SHOP FABRICATION BE BY AISC-APPROVED FABRICATORS

3. ALL STRUCTURAL WELDING SHALL CONFORM WITH THE FOLLOWING SPECIFICATIONS:

WELDING RODS E-70 XX (typical)

E-60 XX (roof decks) STRUCTURAL WELDING AWS D1.1 (performed by AWS certified welder)

STRUCTURAL CUTTING AWS CERTIFIED WELDER

HSA's /DRA's MANUFACTURERS SPECIFICATIONS

REINFORCING BARS (REBAR) DO NOT WELD REBAR

DO NOT WELD ANCHOR BOLTS (including tack welds) ANCHOR BOLTS

ANY SUBSTITUTION OF ANY MEMBERS SHALL BE AT THE WRITTEN CONSENT OF THE STRUCTURAL ENGINEER.

L. STRUCTURAL STEEL REQUIREMENTS CONTINUED:

- ANY CONNECTIONS OF INTERSECTING STEEL MEMBERS SHALL BE A FILLET WELD AND SHALL BE SIZED BY SUBTRACTING 1/16" LESS THAN THE THINNEST CONNECTING MEMBER WITH A MINIMUM OF A 3/16" FILLET WELD. ALL WELDS SHALL BE FULL SURFACE WELDS. WHEN IN DOUBT
- ALL STRUCTURAL CONNECTIONS, SHOWN IN THE PLANS, SHALL BE BOLTED WITH A MINIMUM OF A 3/4" DIAMETER ASTM A325N BOLT, UNLESS NOTED OTHERWISE AND SHALL BE TIGHTENED TO A SNUG TIGHT FIT, AS DEFINED BY AISC SPECIFICATIONS.
- HARDENED WASHERS SHALL BE PROVIDED AT ALL TURNED ELEMENTS OF BOLTED CONNECTIONS. IF THE CONNECTION IS SLOPED OR SKEWED THEN PROVIDE BEVELED WASHERS AS REQUIRED PROVIDE WASHERS THAT COMPLETELY COVER ANY OVERSIZED HOLES OR SLOTS ER ASTM F-436.
- TIGHTEN ALL SLIP-CRITICAL BOLTS, MOMENT FRAME BOLTS AND BOLTS INSTALLED IN OVERSIZED OR SLOTTED HOLES BY THE "TURN OF THE NUT" OR THE "DIRECT TENSION" METHOD. PROVIDE HARDENED WASHERS UNDER ALL TURNED ELEMENTS.
- WHERE ASTM A490 OR STRONGER BOLTS ARE SPECIFIED IN THE DRAWINGS, BOTH THE BOLT HEAD AND THE NUT SHALL HAVE A HARDENED WASHER BETWEEN IT AND THE BASE STEEL AND REQUIRED IN AISC SPECIFICATIONS.
- 10. BOLTS MUST BE LONG ENOUGH FOR THREADS TO BE FLUSH WITH THE OUTSIDE FACE OF THE NUT AFTER TIGHTENING, NO MORE THAN 5 THREADS OF "STICKOUT" ARE PERMITTED. THE "TURN OF THE NUT" METHOD IS DESCRIBED BELOW (THIS IS FOR PERPENDICULAR SURFACES ONLY).
 - THE TURN OF THE NUT METHOD IS NOT TO BE USED FOR BOLTS LONGER THEN 12 BOLT DIAMETERS. TIGHTEN ALL BOLTS IN THE CONCO NECTION TO THE SNUG, TIGHT CONDITION. FROM THE SNUG, TIGHT CONDITION, TIGHTEN EACH BOLT AS SPECIFIED

AMOUNT OF TURN APPLIED TO NUT: BOLT LENGTH UP TO AND INCLUDING 1/3 TURN OF THE NUT 4 DIAMETERS OVER 4 DIAMETERS 1/2 TURN OF THE NUT

BUT NOT EXCEEDING 8 DIAMETERS

OVER 8 DIAMETERS BUT NOT EXCEEDING 12 DIAMETERS

- 11 DO NOT REUSE BOLTS NUTS OR WASHERS
- 12. FITTED STIFFENER PLATES SHALL BE PROVIDED AT ALL BEARING LOCATIONS AND SHALL MEET THE FOLLOWING REQUIREMENTS UNLESS NOTED OTHERWISE IN THE DRAWINGS:

2/3 TURN OF THE NUT

FLANGE WIDTH	STIFFENER PLATE THICKNESS	FILLET WELD THICKNESS
UP TO 8"	3/8"	1/4"
8" to 12"	3/8"	1/4"
12" to 16"	1/2"	5/16"
16" AND BIGGER	5/8"	3/8"

- 13. HOLES IN STEEL SHALL BE DRILLED OR PUNCHED. ALL SLOTTED HOLES SHALL BE PROVIDED WITH SMOOTH EDGES. BURNING OF HOLES AND TORCH CUTTING AT THE SITE IS NOT
- THE STRUCTURAL STEEL ERECTOR SHALL PROVIDE ANY REQUIRED TEMPORARY GUYING AND BRACING REQUIRED. COLUMNS, ANCHOR BOLTS, BASE PLATES, ETC. HAVE BEEN DESIGNED FOR THE FINAL COMPLETED CONDITION AND HAVE NOT BEEN INVESTIGATED FOR POTENTIAL LOADINGS ENCOUNTERED DURING STEEL ERECTION AND CONSTRUCTION. ANY INVESTIGATION OF THE COLUMNS, ANCHOR BOLTS, BASE PLATES, ETC.. FOR ADEQUACY DURING THE STEEL ERECTION AND CONSTRUCTION PROCESS IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR.
- 15. REFER TO THE "SPECIAL INSPECTION" SECTION OF THE GENERAL STRUCTURAL NOTES FOR ANY INSPECTION REQUIREMENTS.

M. QUALITY CONTROL AND INSPECTION REQUIREMENTS:

QUALITY CONTROL AND INSPECTIONS SHALL BE PERFORMED AS REQUIRED IN IBC 2021 CHAPTER 17. AS STATED IN IBC 2021 1704.2.1, "THE REGISTERED DESIGN PROFESSIONAL IN RESPONSIBLE CHARGE AND ENGINEERS OF RECORD INVOLVED IN THE DESIGN OF THE PROJECT ARE PERMITTED TO ACT AS THE APPROVED AGENCY AND THEIR PERSONNEL ARE PERMITTED TO ACT AS THE SPECIAL INSPECTORS FOR THE WORK DESIGNED BY THEM, PROVIDED THEY QUALIFY AS SPECIAL INSPECTORS."

N. SPECIAL INSPECTION REQUIREMENTS:

- 1 SPECIAL INSPECTION AND QUALITY ASSURANCE AS REQUIRED BY SECTION 1705 OF THE IBC. SHALL BE PROVIDED BY AN INDEPENDENT AGENCY EMPLOYED BY THE OWNER UNLESS WAIVED BY THE BUILDING OFFICIAL. THE CONTRACTOR SHALL COORDINATE AND COOPERATE WITH THE REQUIRED INSPECTIONS. ALL TESTING AND INSPECTION REPORTS SHALL BE SENT TO THE ENGINEER OF RECORD FOR REVIEW. ITEMS REQUIRING SPECIAL INSPECTION AND QUALITY ASSURANCE ARE SHOWN IN THIS SECTION.
- 2 SOILS PER IBC SECTION 1705 6 AND TABLE 1705 6 RELOW

PLACEMENT OF FILL.

SPECIAL INSPECTION SHALL BE PROVIDED PRIOR TO POURING CONCRETE FOOTINGS. SPECIAL INSPECTION SHALL BE PROVIDED PRIOR TO PLACEMENT OF FILL AND DURING

TABLE 1705.6 REQUIRED SPECIAL INSPECTIONS AND TESTS OF SOILS							
TYPE OF INSPECTION OR TEST	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION					
 VERIFY MATERIALS BELOW SHALLOW FOUNDATIONS ARE ADEQUATE TO ACHIEVE THE DESIGN BEARING CAPACITY. 	-	x					
2. VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIAL.	-	x					
3. PERFORM CLASSIFICATION AND TESTING OF COMPACTED FILL MATERIALS.	-	x					
4. VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.	x	-					
PRIOR TO PLACEMENT OF COMPACTED FILL, INSPECT SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	-	x					

T.D. E 4305.0

3. CONCRETE CONSTRUCTION PER IBC SECTION 1705.3 AND TABLE 1705.3 BELOW:

TA REQUIRED SPECIAL INSPECTIONS	ABLE 1705. AND TESTS	_	ETE CONST	RUCTION
TYPE OF INSPECTION OR TEST	CONTINUOUS SPECIAL INSPECTION	PERIODIC SPECIAL INSPECTION	REFERENCED STANDARD [®]	IBC REFERENCE
INSPECT REINFORCEMENT, INCLUDING PRESTRESSING TENDONS, AND VERIFY PLACEMENT.	-	x	ACI 318: CH. 20, 25.2, 25.3, 26.6.1-26.6.3	1908.4
2. REINFORCING BAR WELDING: a. VERIFY WELDABILITY OF REINFORCING BARS OTHER THAN ASTM A706. b. INSPECT SINGLE-PASS FILLET WELDS, MAXIMUM 5/16. c. INSPECT ALL OTHER WELDS.			AWS D1.4 ACI 318: 26.6.4	-
3. INSPECT ANCHORS CAST IN CONCRETE.	-	х	ACI 318: 17.8.2	-
4. INSPECT ANCHORS POST—INSTALLED IN HARDENED CONCRETE WEBBERS. ³ o. Ante-sive Anchors Installed in Horzontally or Uphardly inclinded orientations to resist sustained tesison Loads. b. McCharical Anchors and Adhesive Anchors not deferred in 4.4.				-
5. VERIFY USE OF REQUIRED DESIGN MIX.	-	х	ACI 318: CH. 19, 26.4.3, 26.4.4	1904.1, 1904.2, 1908.2, 1908.3
6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGTH TESTS, PERFORM SLUMP AND AIR CONTENT TESTS, AND DETERMINE THE TEMPERATURE OF THE CONCRETE	x	-	ASTM C172 ASTM C31 ACI 318: 26.5, 26.12	1908.10
7. INSPECT CONCRETE AND SHOTCRETE PLACEMENT FOR PROPER APPLICATION TECHNIQUES.	x	-	ACI 318: 26.5	1908.6, 1908.7, 1908.8
8. VERIFY MAINTENANCE OF SPECIFIED CURING TEMPERATURE AND TECHNIQUES.	-	х	ACI 318: 26.5.3-26.5.5	1908.9
9. INSPECT PRESTRESSED CONCRETE OF: a. APPLICATION OF PRESTRESSING FORCES. b. GROUTING OF BONDED PRESTRESSING TENDONS.			ACI 318: 26.10	-
10. INSPECT ERECTION OF PRECAST CONCRETE MEMBERS.	-	х	ACI 318: 26.9	-
 VERIFY IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS. 	-	х	ACI 318: 26.11.2	-
12. INSPECT FORMWORK FOR SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEING FORMED.	-	х	ACI 318: 26.11.1.2(b)	-

a. BREE PHYLOLE, ES ESCINA (1982), SECOLA INSPECIONS FOR STORIC RESISTANCE.
b. SPECIFE REMINISTER OF SPECIA INSPECIALO SHALL BE ALIGINA THE RESEASON FROM THE AMOUNT SINCE OF ADVANCE OF ACCIDENCE WITH TALK IN ACT OFFICE CHARGESTON.
CONTRACTOR STORIC RESISTANCE WITH THOMAS, SPECIAL RESISTANCE SHALL BE SPECIFED OF THE RESISTANCE DESIGN AMOTESCIAL, AND SHALL SE APPRICAD OF THE SALDING OFFICIAL.

N. <u>DEFERRED SUBMITTAL REQUIREMENTS:</u>

SHOP DRAWINGS OR REPORTS FOR THE FOLLOWING ITEMS SHALL BE SUBMITTED TO THE STRUCTURAL ENGINEER OF RECORD PRIOR TO FABRICATION OR CONSTRUCTION (AS APPLICABLE)

CONCRETE CYLINDER TESTS REINFORCING STEEL CONCRETE MIX DESIGN

- 2. CONTRACTOR SHALL REVIEW AND STAMP SHOP DRAWINGS PRIOR TO SUBMITTING. CONTRACTOR'S REVIEW SHALL CHECK FOR COMPLETENESS/COMPLIANCE WITH CONTRACT DOCUMENTS.
- SHOP DRAWINGS ARE REVIEWED BY ENGINEER ONLY FOR GENERAL COMPLIANCE WITH THE STRUCTURAL DRAWINGS. RESPONSIBILITY FOR CORRECTNESS SHALL REST WITH THE CONTRACTOR. SHOP DRAWINGS DO NOT SUPERSEDE OR REPLACE THE CONTRACT DRAWINGS OR SPECIFICATIONS. CHANGES, SUBSTITUTIONS, OR DEVIATIONS FROM CONTRACT DRAWINGS AND/OR SPECIFICATIONS WILL NOT BE ACCEPTED VIA SHOP DRAWING REVIEW. ALL SUCH MODIFICATIONS SHALL BE SUBMITTED SEPARATELY FOR ENGINEER'S REVIEW
- PREFABRICATED COMPONENTS, SPECIALTY ITEMS, OR DESIGN-BUILD ELEMENTS NOTED ON THE STRUCTURAL DRAWINGS, BUT WHICH REQUIRE THE MFR. OR SUPPLIER TO PROVIDE THE DESIGN, SHALL BE SUBMITTED BY THE CONTRACTOR TO THE ARCHITECT AND/OR ENGINEER FOR REVIEW AS A DEFERRED SUBMITTAL. DEFERRED SUBMITTALS REO'D, BY THE STRUCTURAL ENGINEER OF RECORD SHALL INCLUDE. BUT NOT BE LIMITED TO. THE FOLLOWING:
- 5 DEFERRED SUBMITTALS SHALL INCLUDE CALCULATIONS AND DRAWINGS PREPARED AND STAMPED BY AN APPROPRIATELY LICENSED ENGINEER (SPECIALTY ENGINEER) SHOWING LOCATION AND MAGNITUDE OF LOADS, CONFIGURATION AND SIZE OF MEMBERS, AND COMPATIBILITY OF SUBMITTAL ITEM WITH THE PRIMARY STRUCTURAL SYSTEM.
- THE PURPOSE OF THE STRUCTURAL ENGINEER'S REVIEW OF DEFERRED SUBMITTALS SHALL BE LIMITED TO DETERMINING THAT THE DRAWINGS AND CALCULATIONS HAVE BEEN PROPERLY SEALED, THAT THE LOAD CRITERIA IS IN GENERAL CONFORMANCE WITH THE CONTRACT DOCUMENTS AND WITH THE REFERENCED BUILDING CODE. THAT CONNECTIONS TO THE PRIMARY STRUCTURE ARE COMPATIBLE WITH THE PRIMARY DESIGN, AND THAT THE PRIMARY STRUCTURE IS CAPABLE OF SUPPORTING THE IMPOSED LOADS.
- THE STRUCTURAL ENGINEER WILL RELY UPON THE SPECIALTY ENGINEER'S SEAL AS CERTIFICATION THAT THE ITEMS DESIGNED BY THE SPECIALTY ENGINEER COMPLY WITH THE CRITERIA SET FORTH IN THE CONTRACT DOCUMENTS AND APPLICABLE CODES AND STANDARDS. THE STRUCTURAL ENGINEER SHALL NOT BE RESPONSIBLE FOR THE ADEQUACY OF DESIGNS PROVIDED BY OTHERS.
- 8. FOR ALL SUBMITTALS, ANY CORRECTIONS NOTED WILL BE MARKED ON ONE (1) COPY SET ONLY AND RETURNED. ADDITIONAL COPIES OF ANY SUBMITTAL WILL BE RETURNED UNMARKED. CONTRACTOR SHALL BE RESPONSIBLE FOR REPRODUCING ENGINEER'S CORRECTIONS ON ADDITIONAL COPIES REQ'D. ONE COPY SET MAY BE RETAINED FOR THE ENGINEER'S RECORDS. ALLOW FIVE (5) TO TEN (10) WORKING DAYS FOR THE ENGINEER'S REVIEW.
- REFER TO APPLICABLE G.S.N. SECTIONS FOR FURTHER REQUIREMENTS SPECIFIC TO INDIVIDUAL



Know what's below.

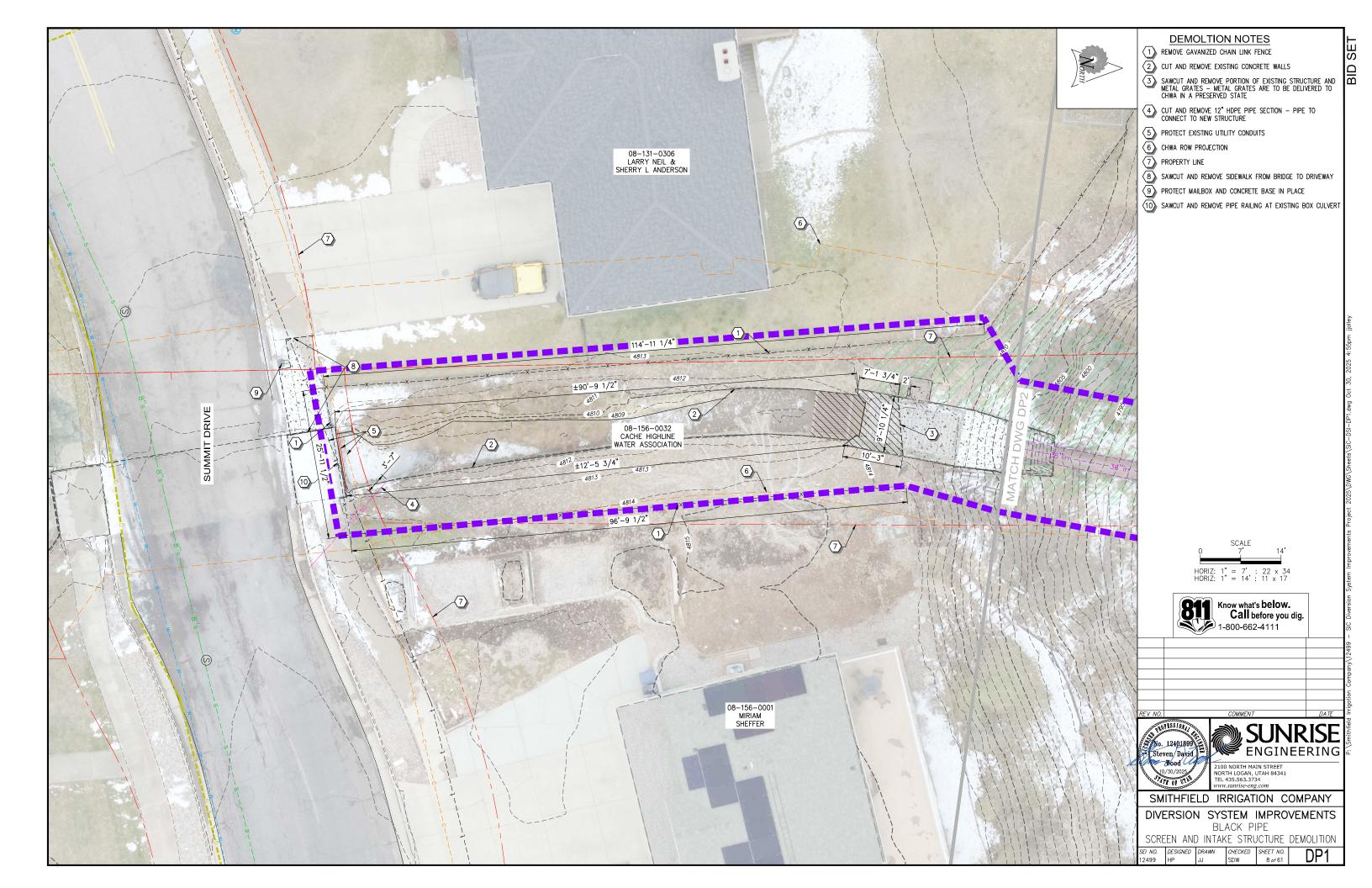


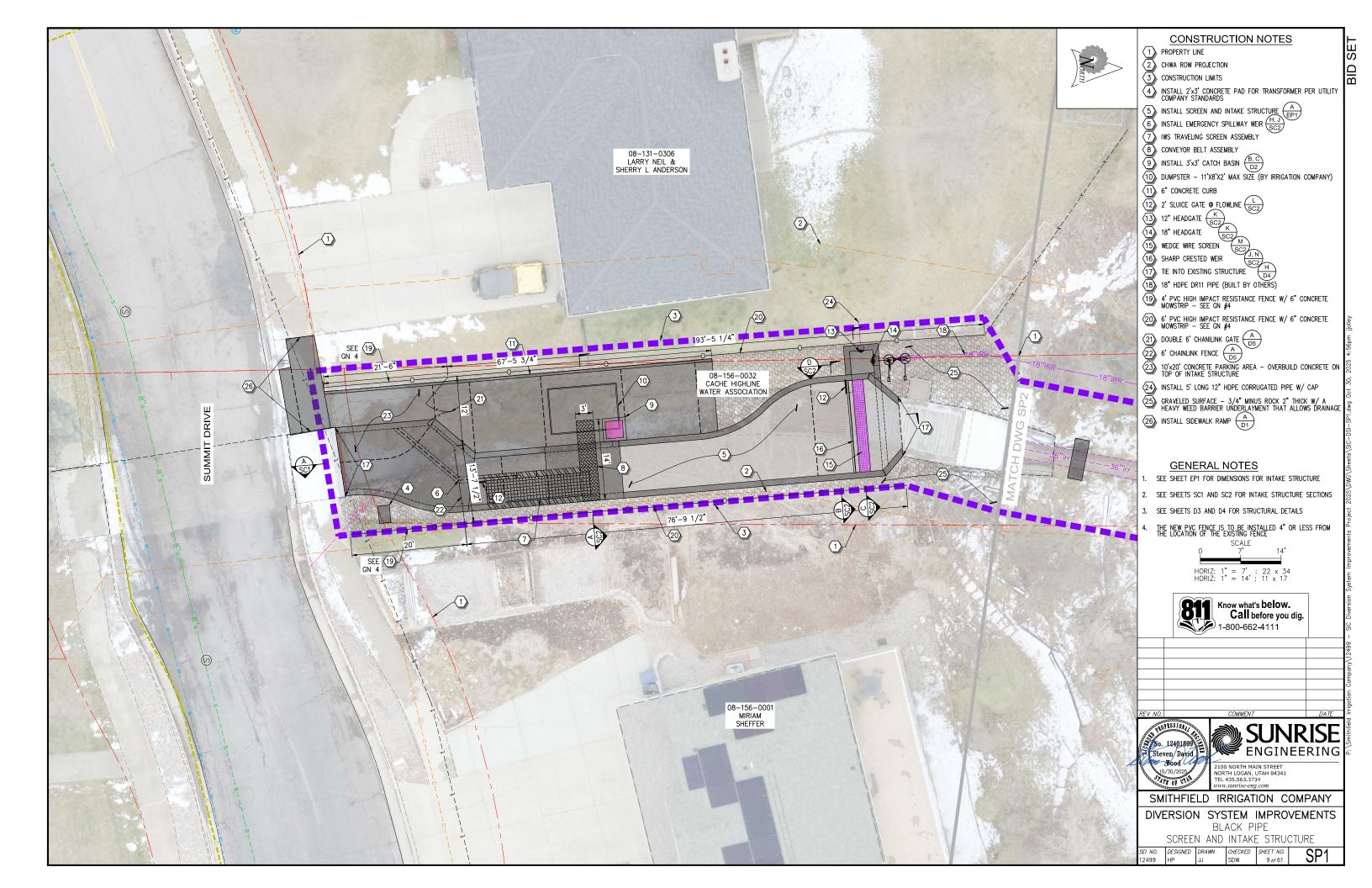
SMITHFIELD IRRIGATION COMPANY

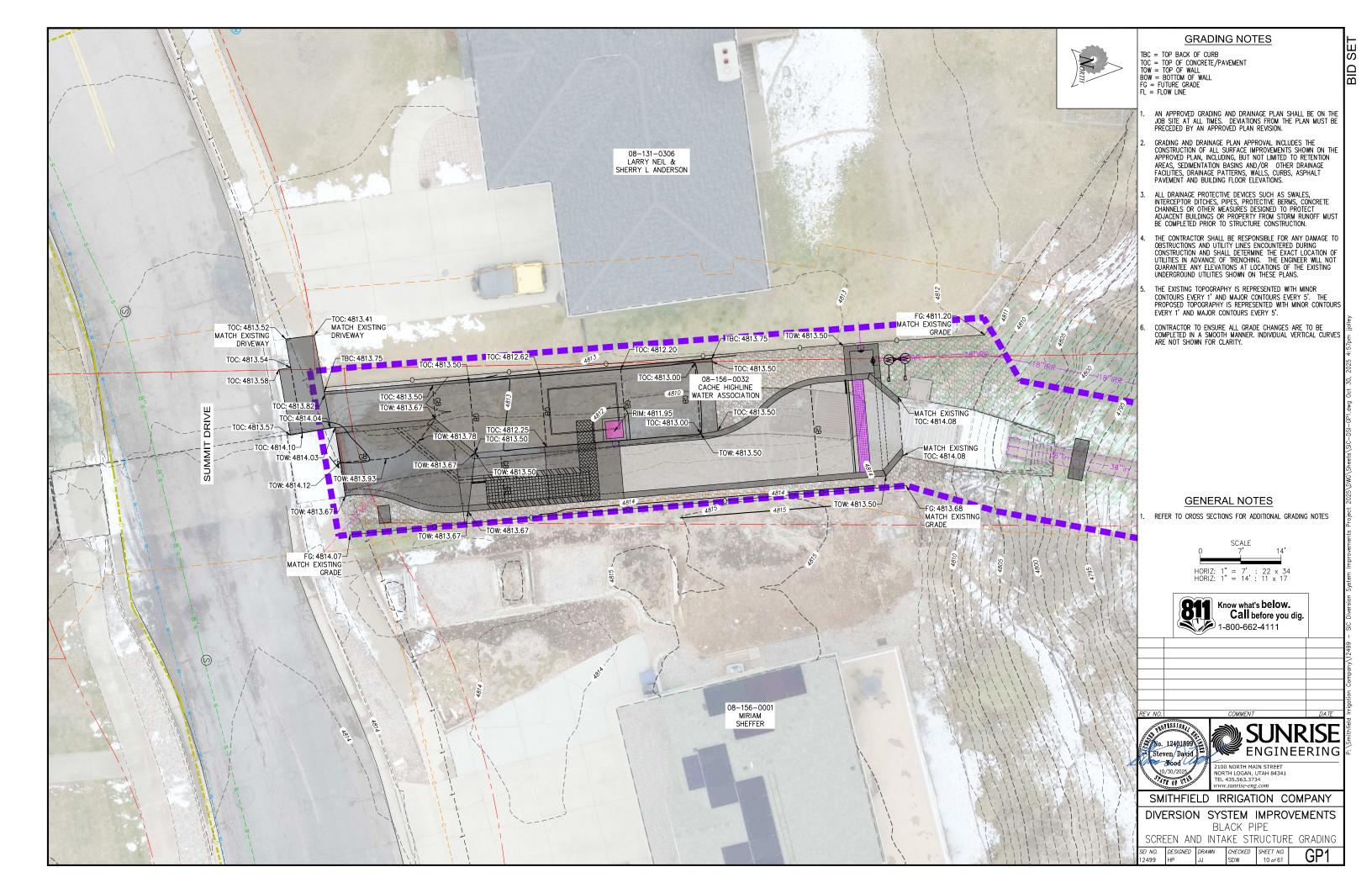
DIVERSION SYSTEM IMPROVEMENTS GENERAL

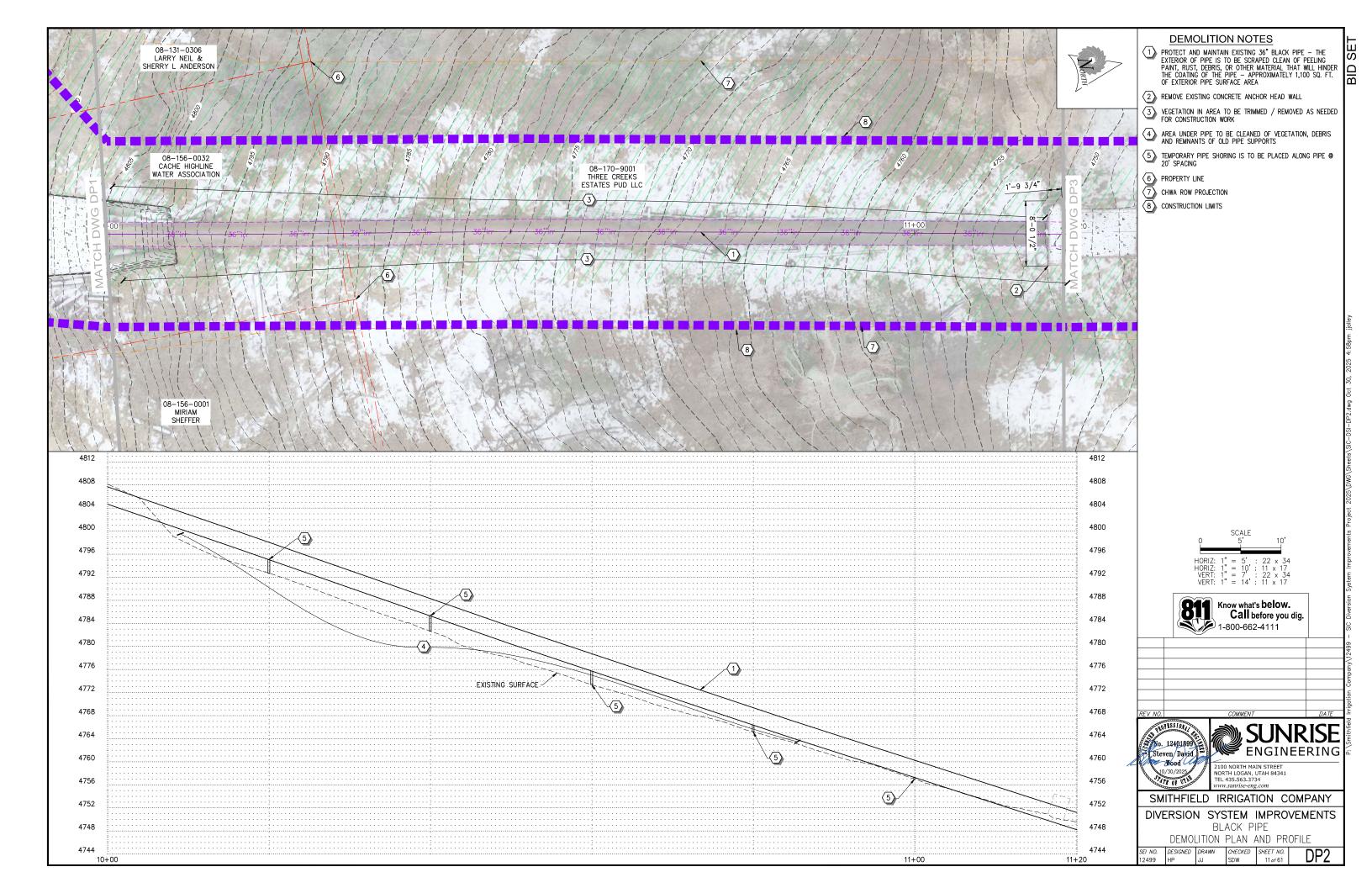
STRUCTURAL SPECIFICATIONS AND REQUIREMENTS

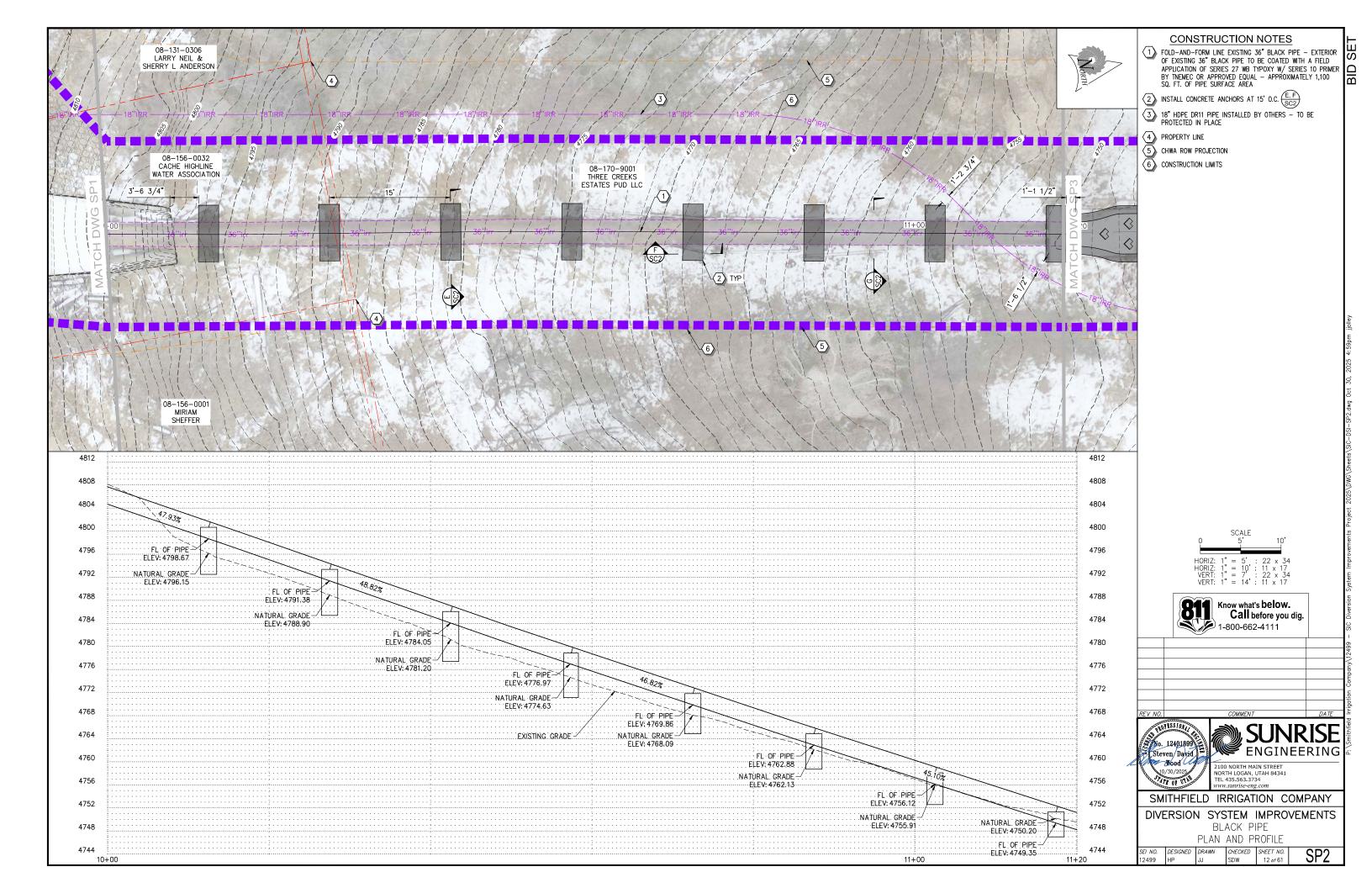
SEI NO. DESIGNED DRAWN CHECKED SHEET NO. SDW

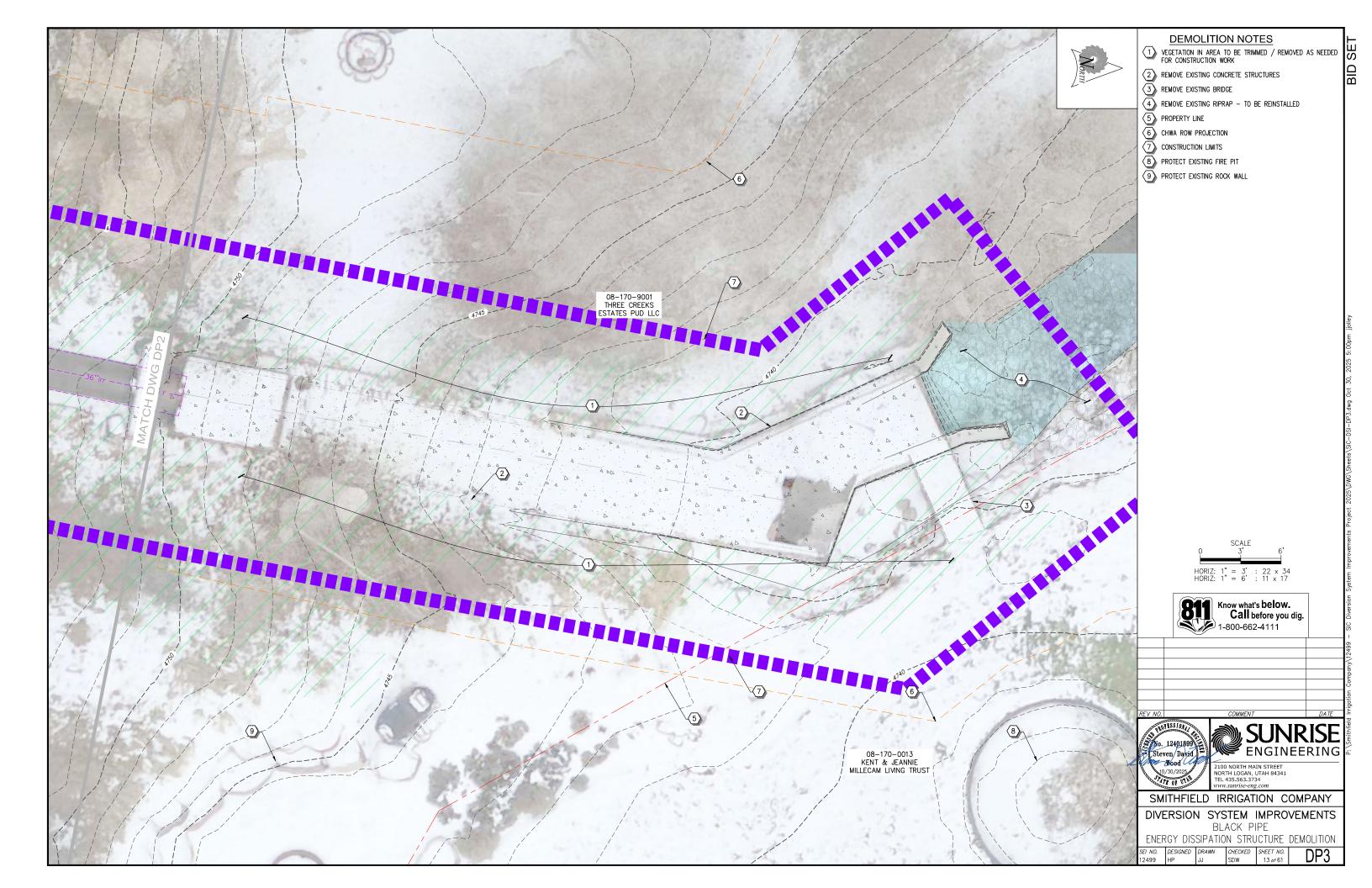


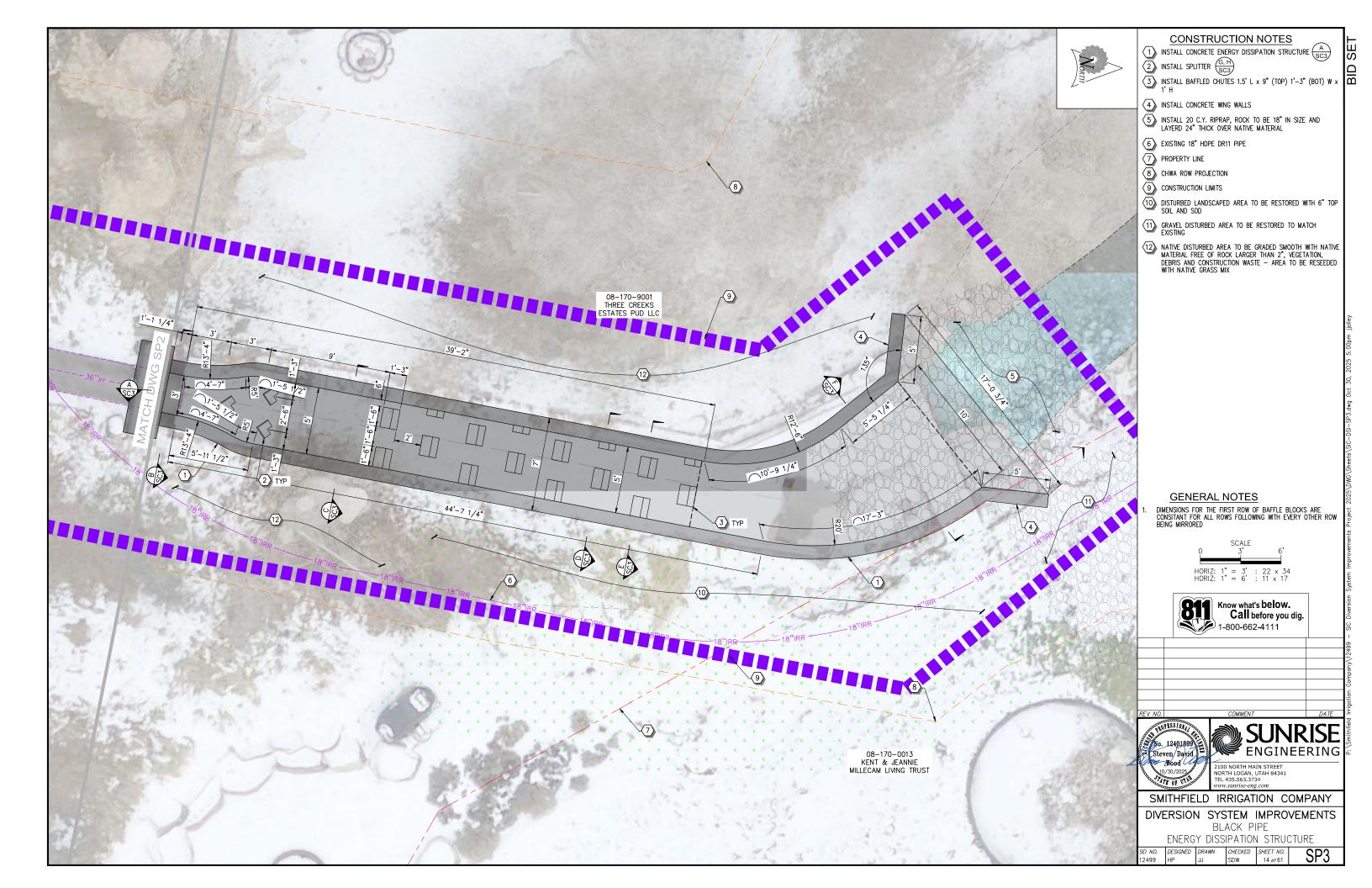


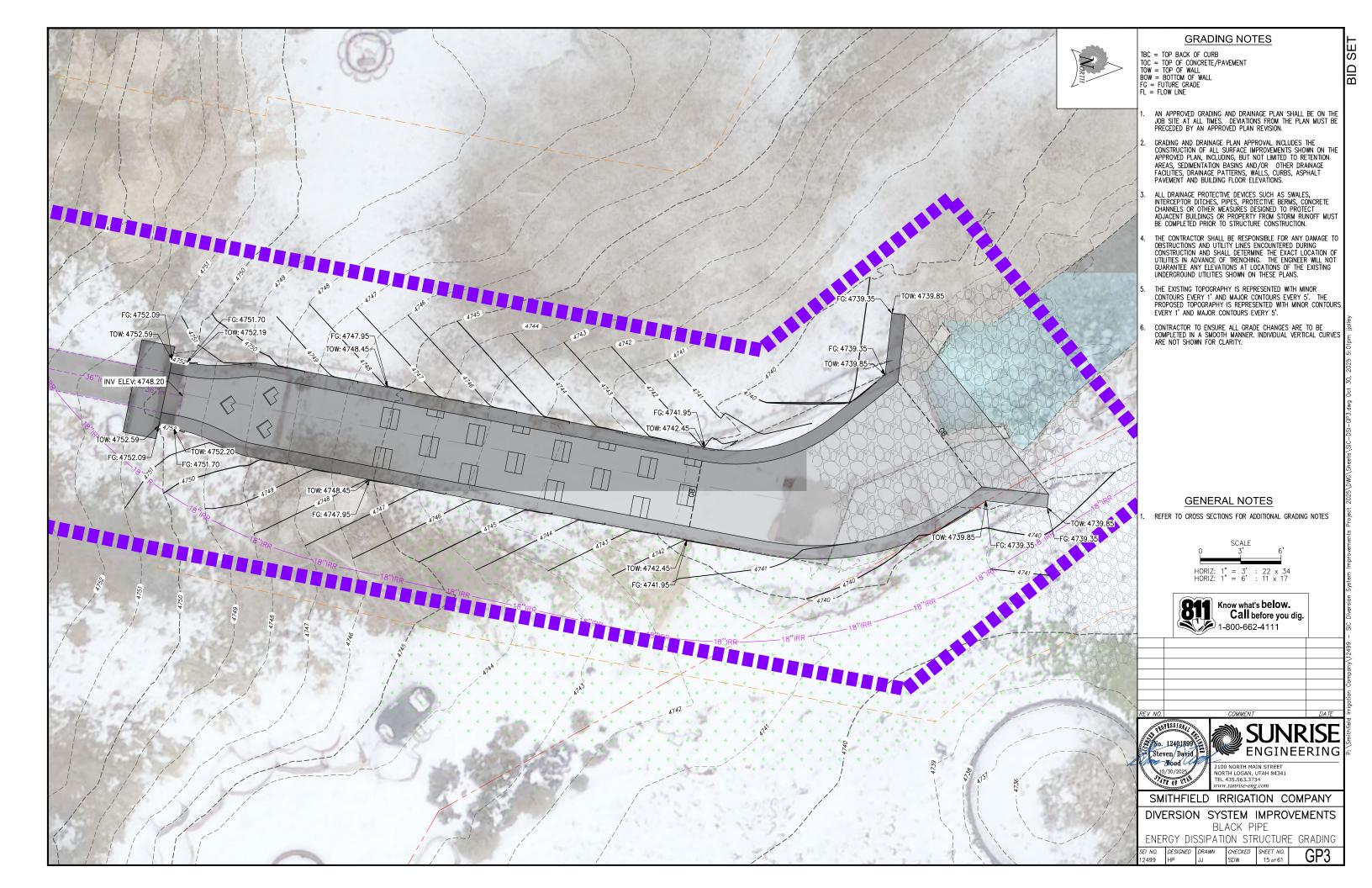


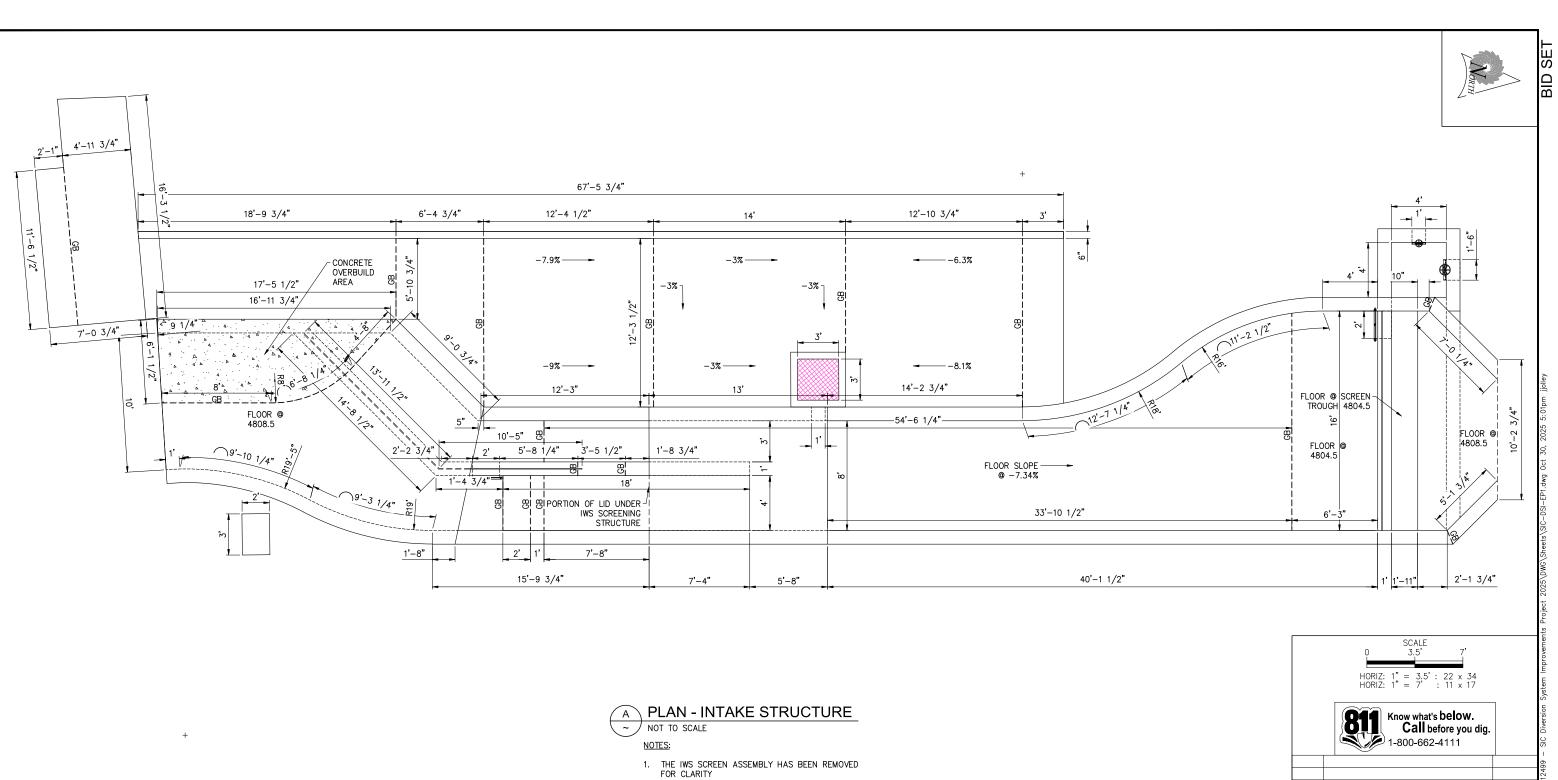




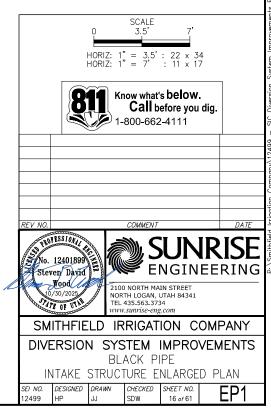


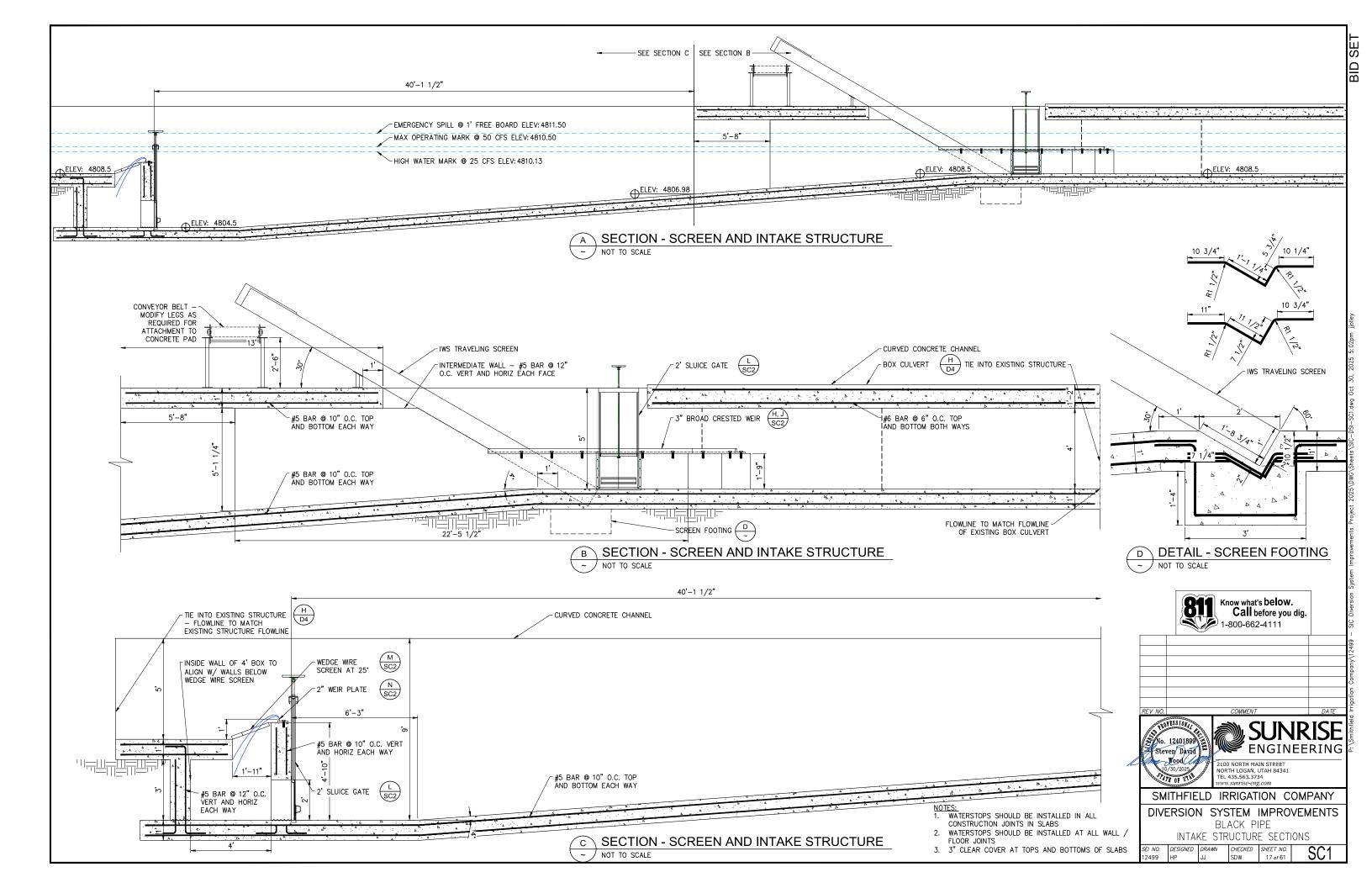


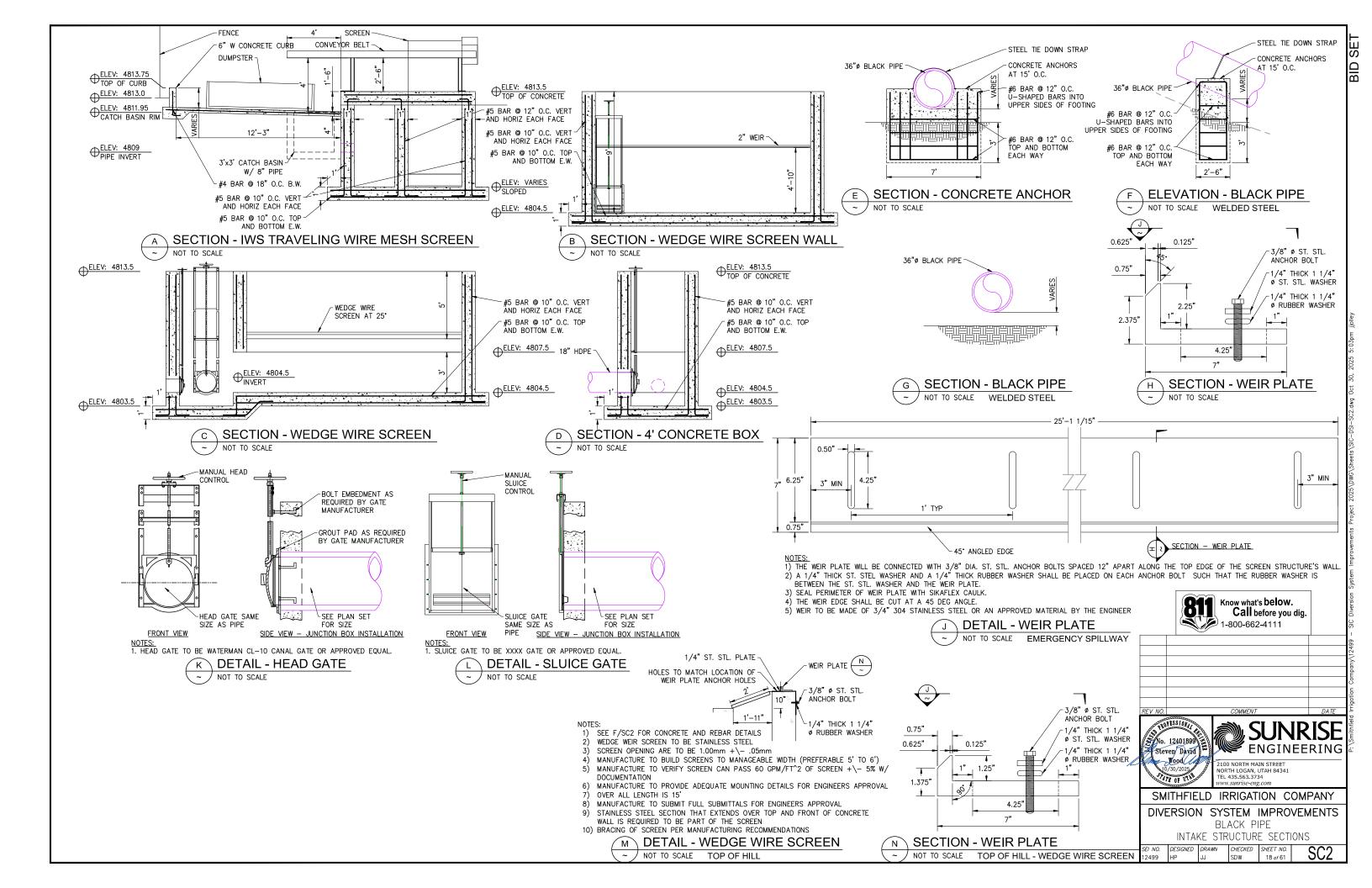


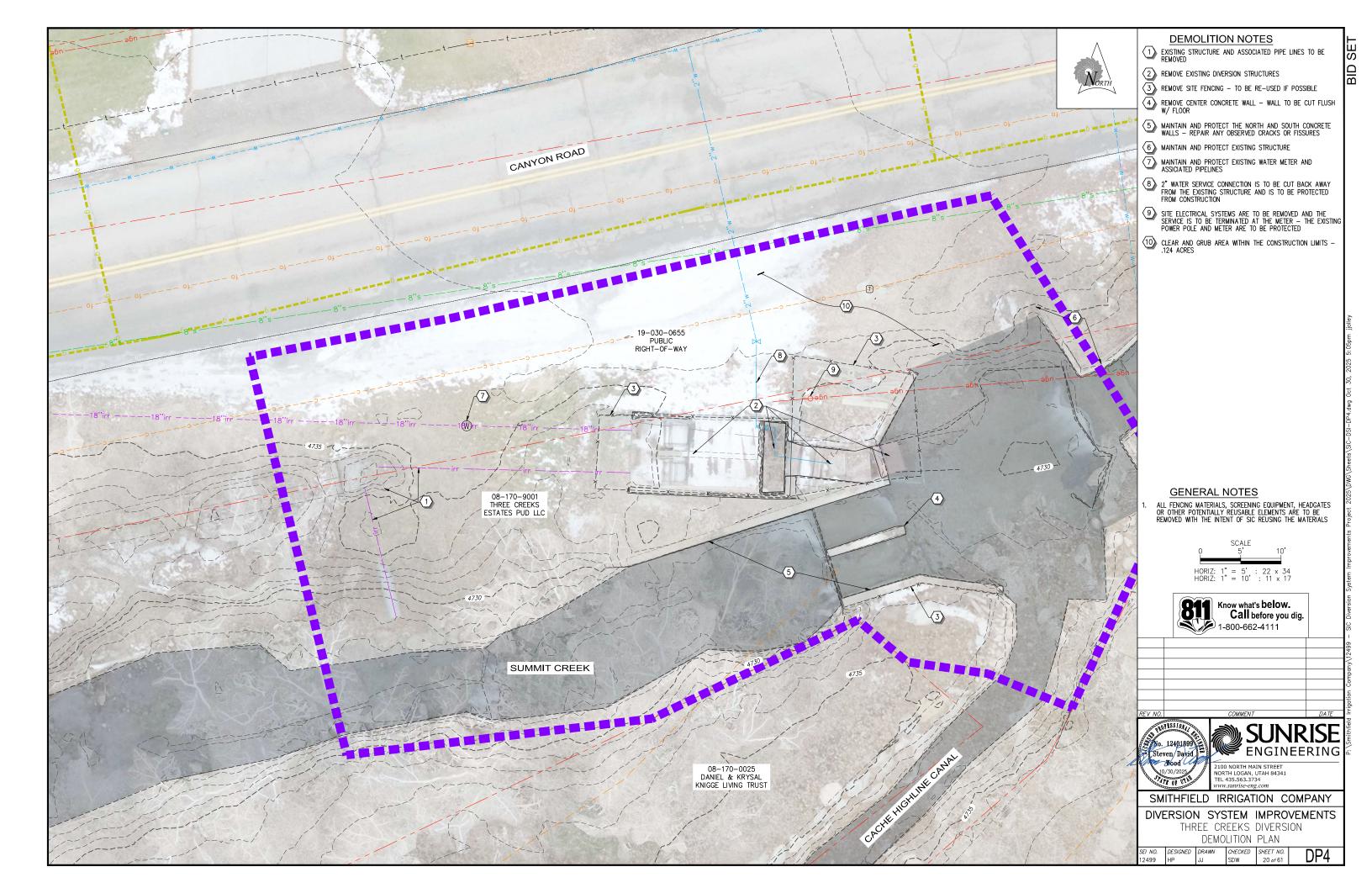


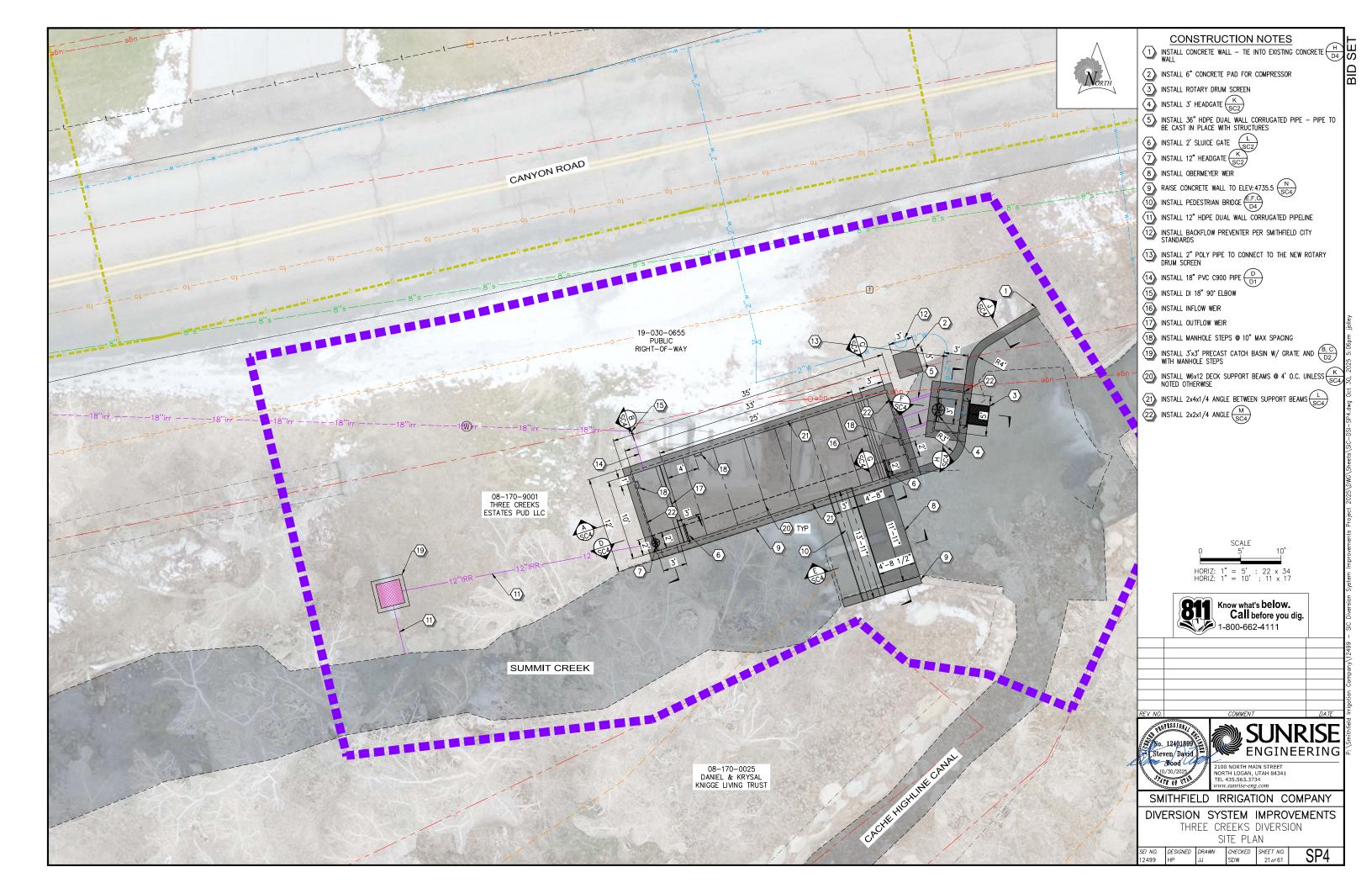
- 2. THE DUMPSTER LINE WORK HAS BEEN REMOVED FOR CLARITY
- 3. THE WEDGE WIRE SCREEN HATCH HAS BEEN REMOVED FOR CLARITY

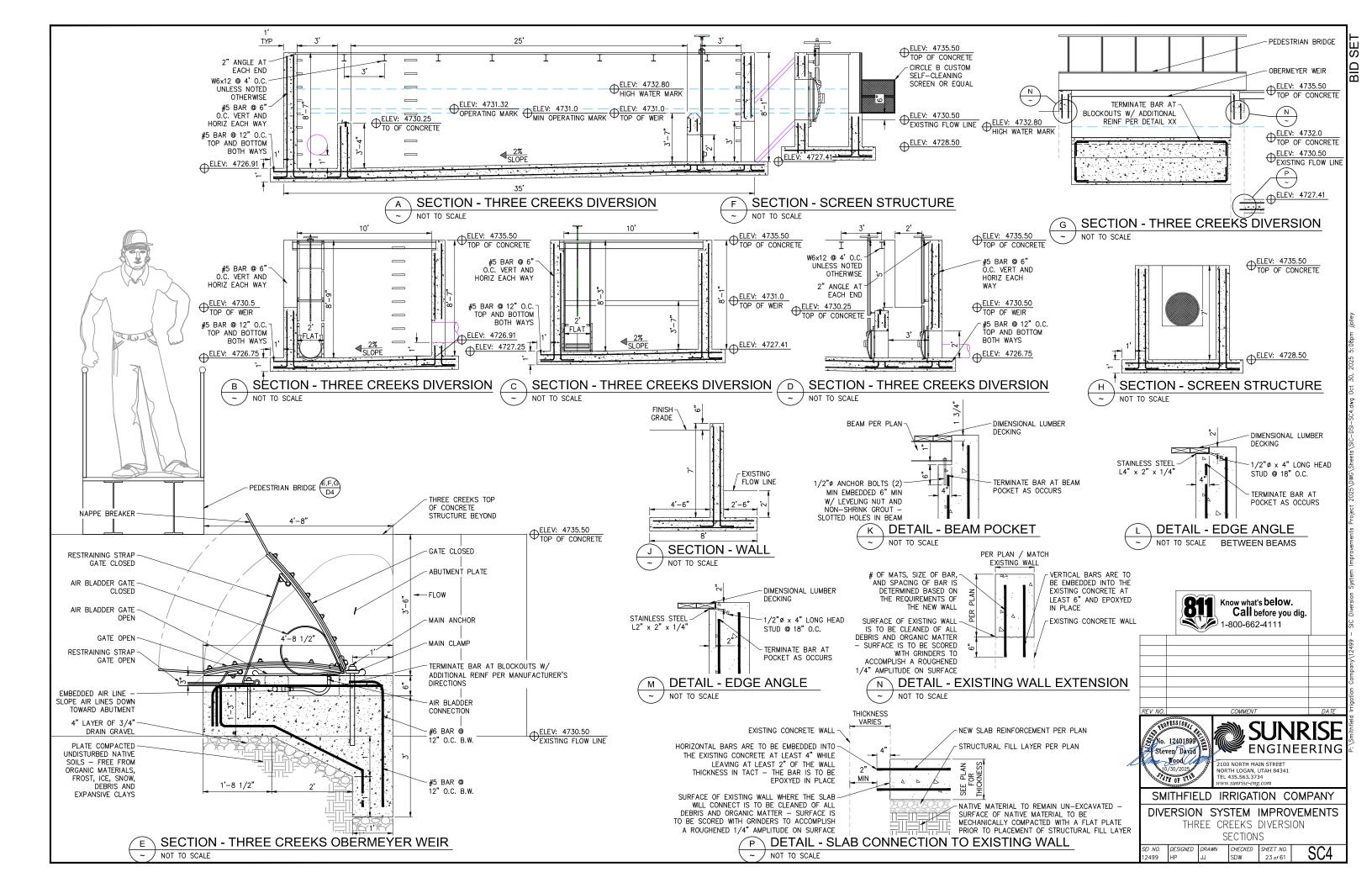






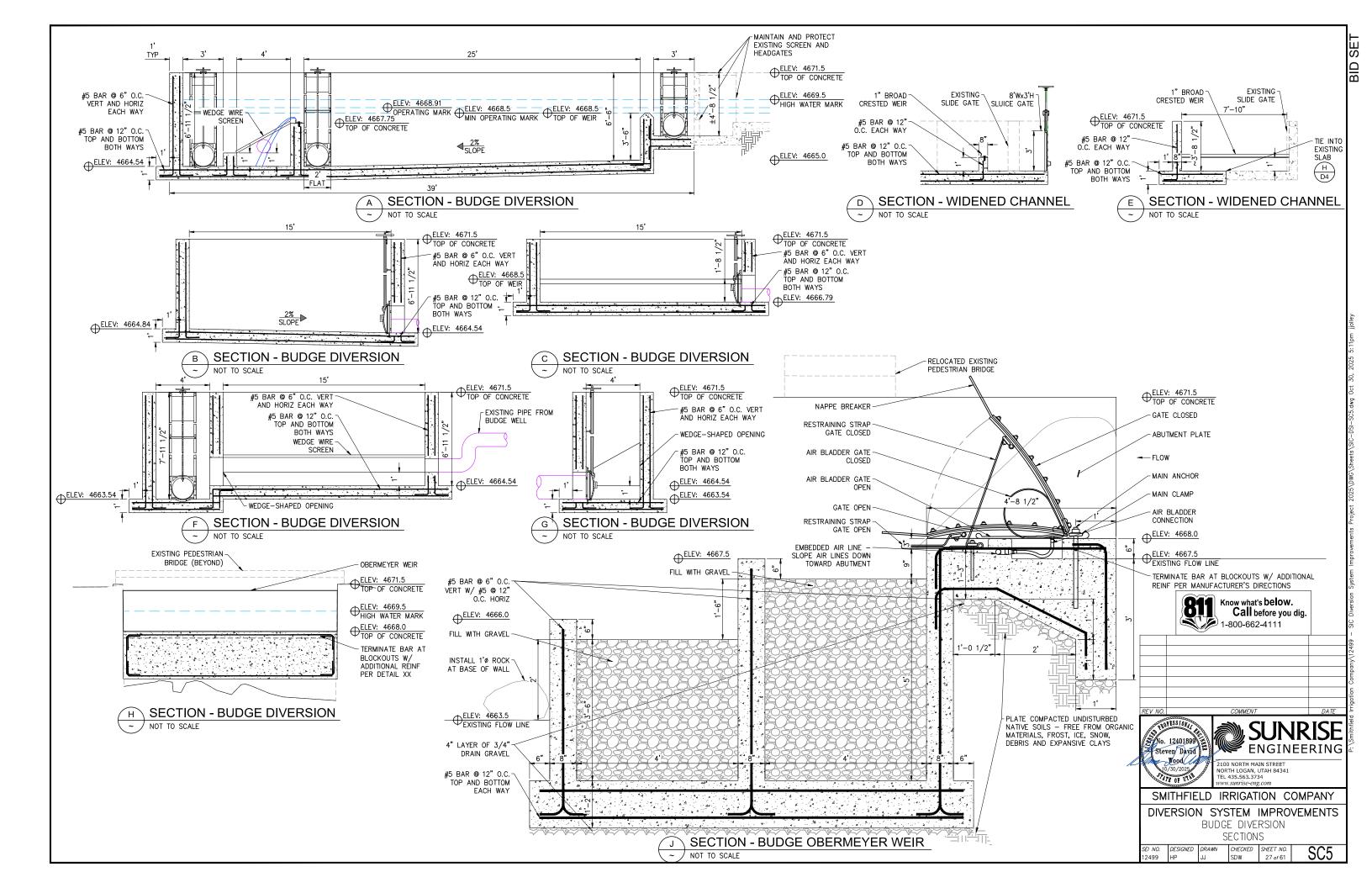


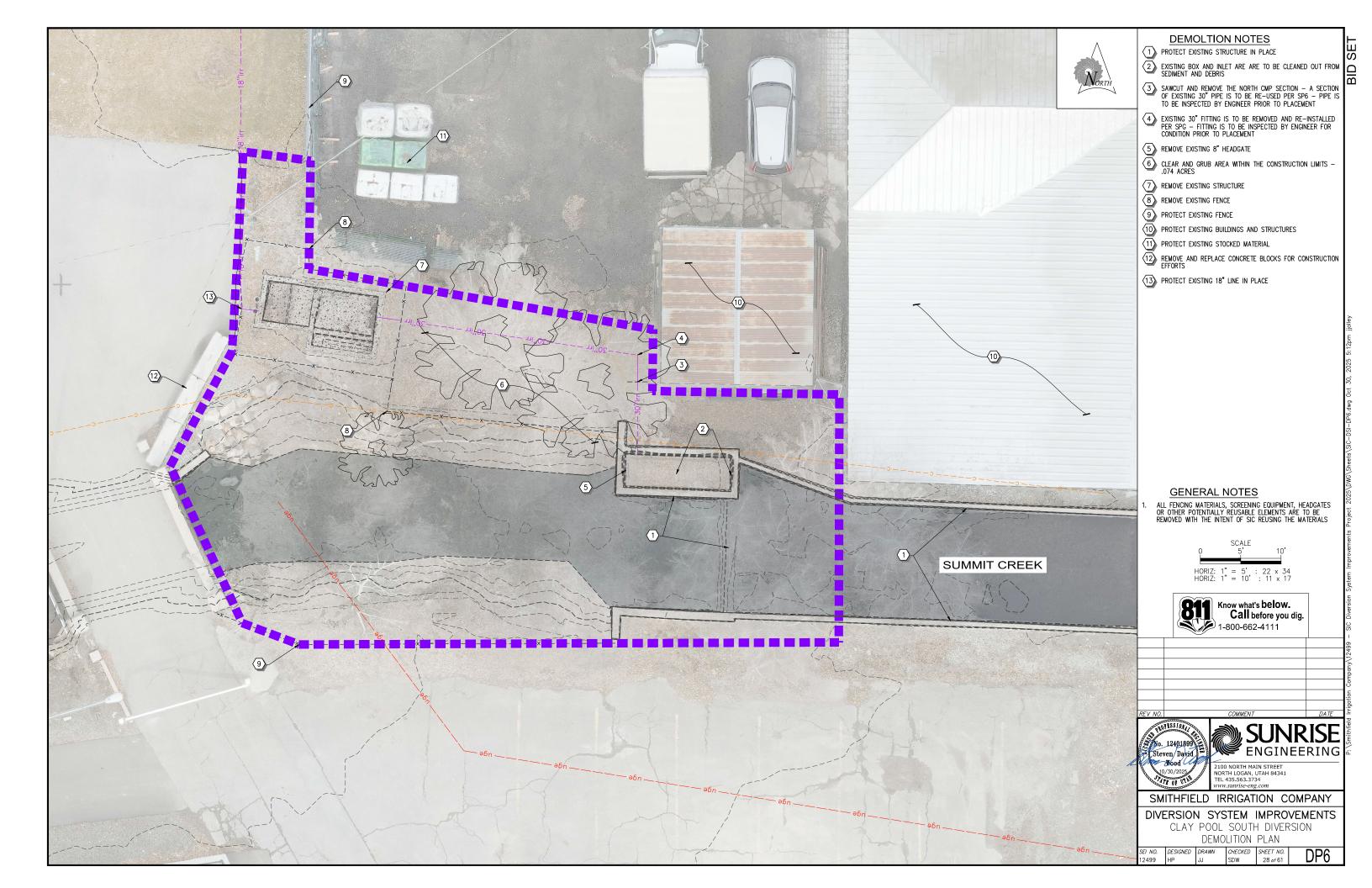


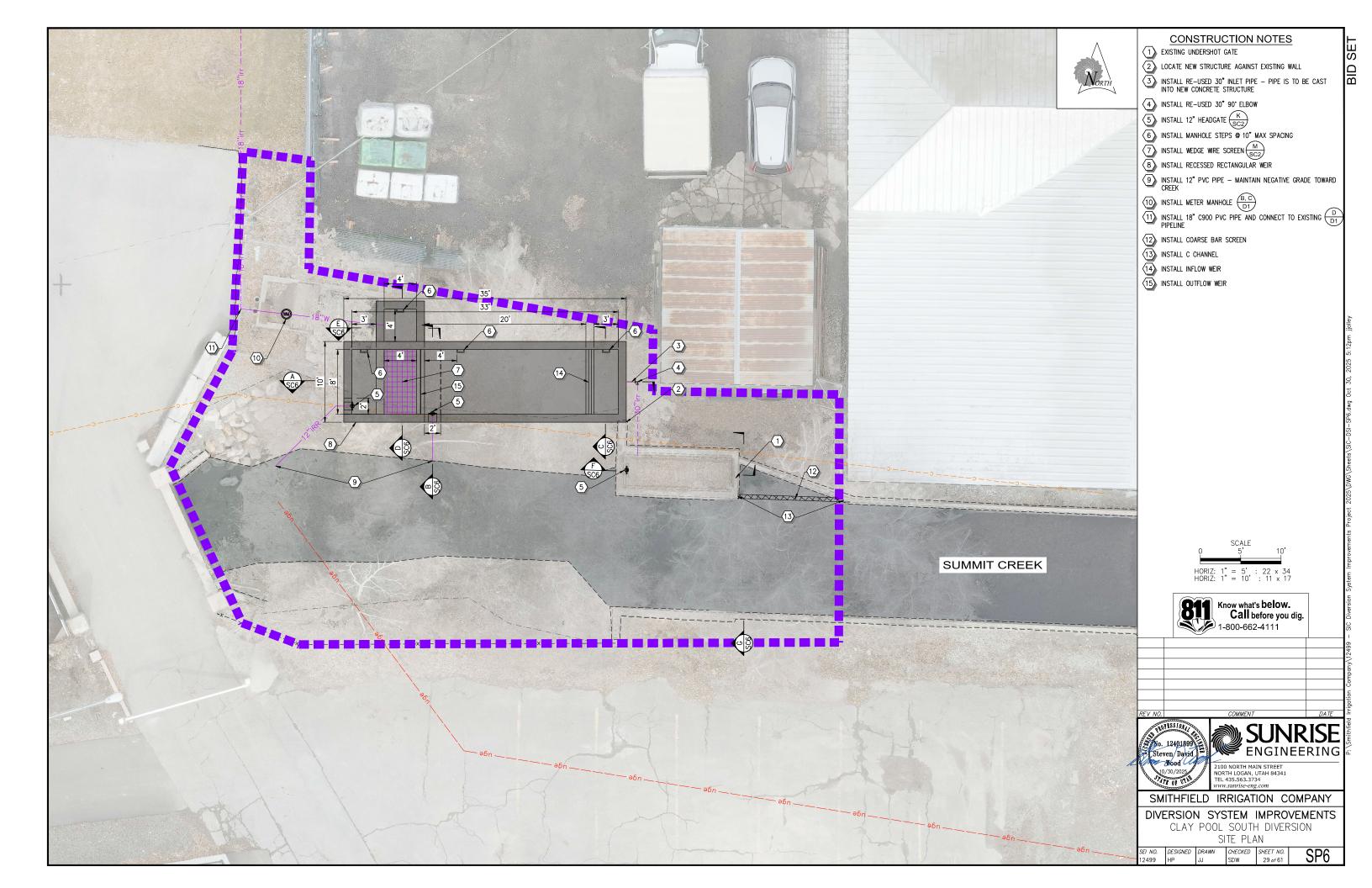


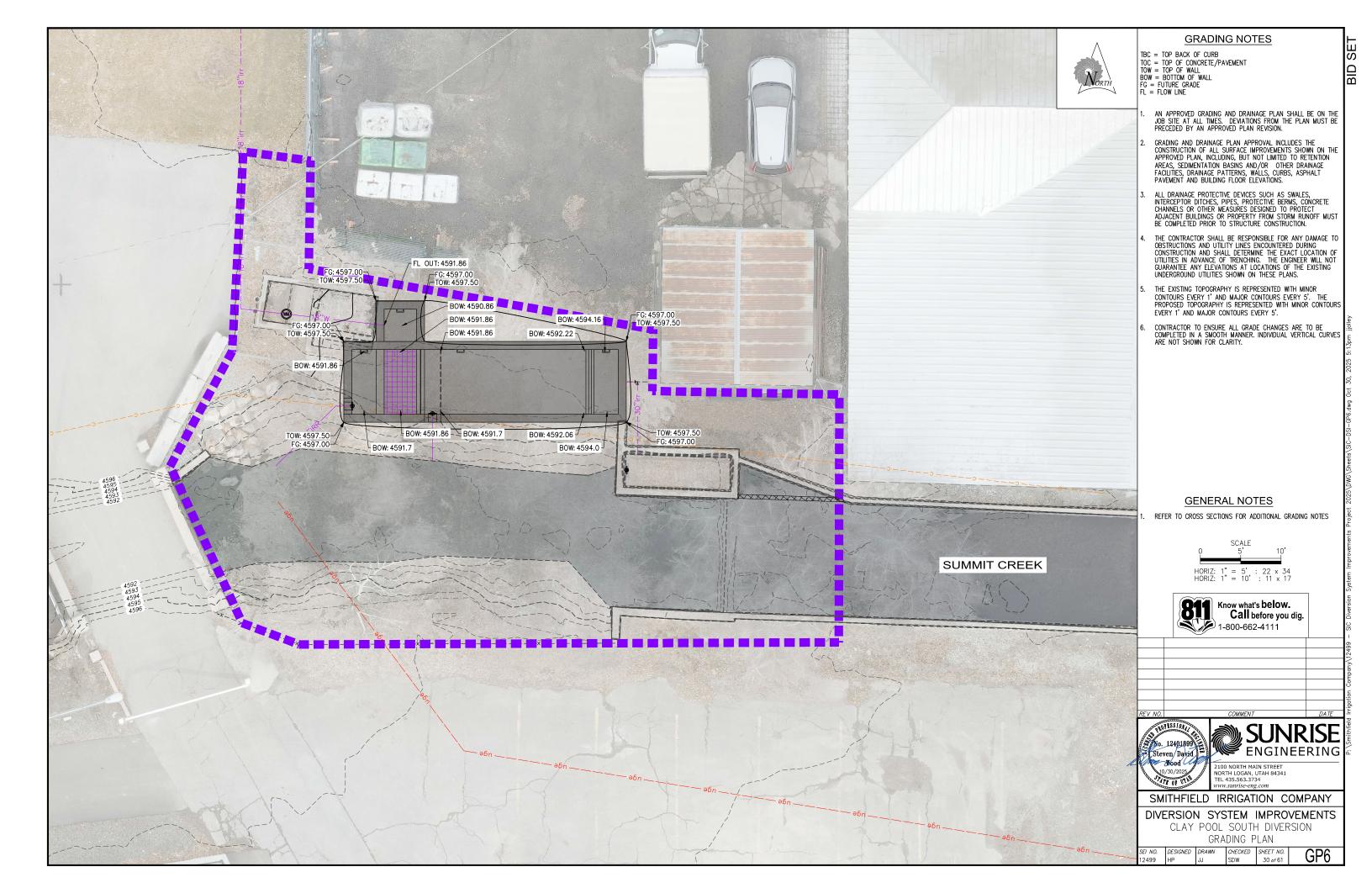


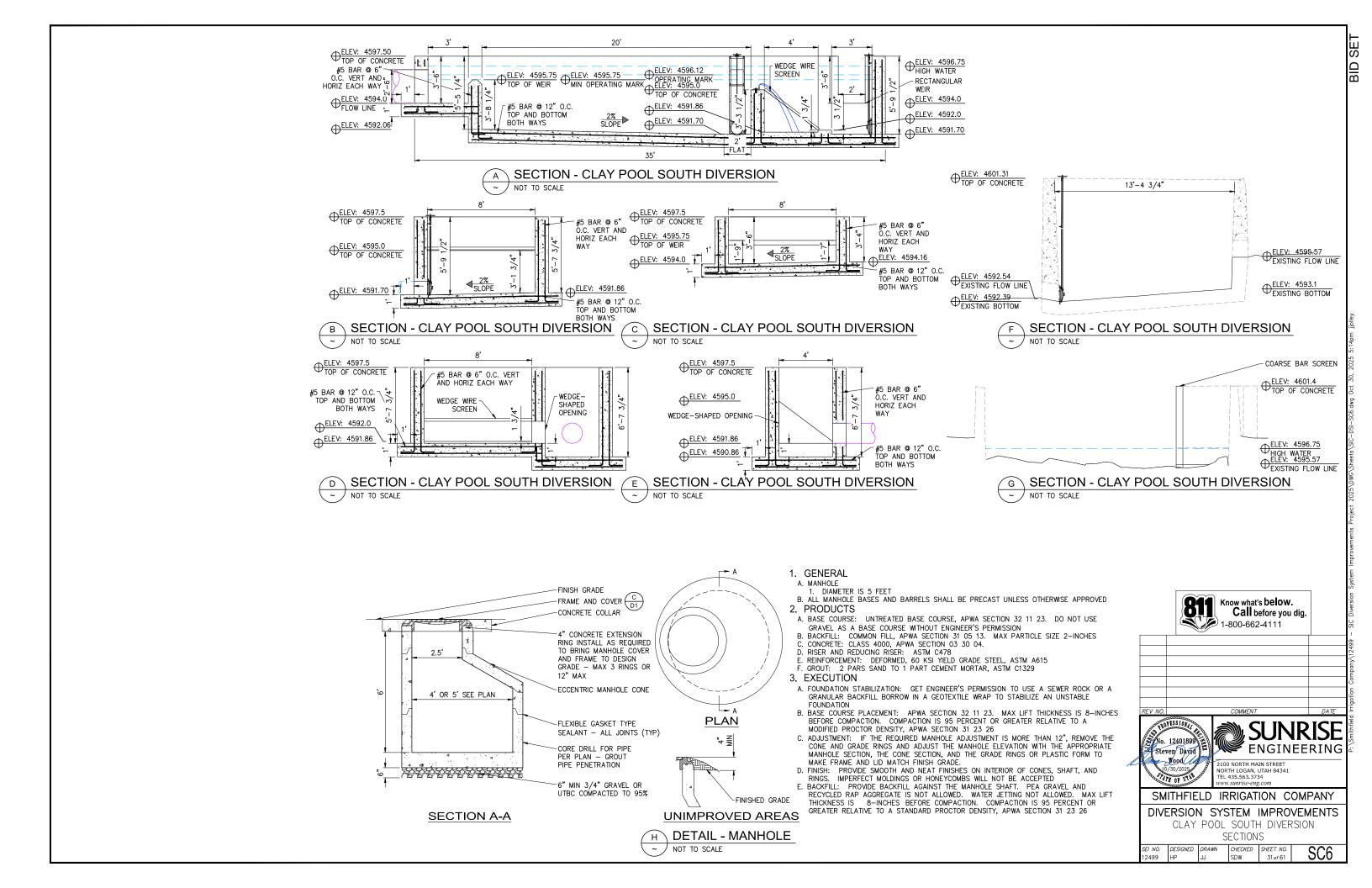




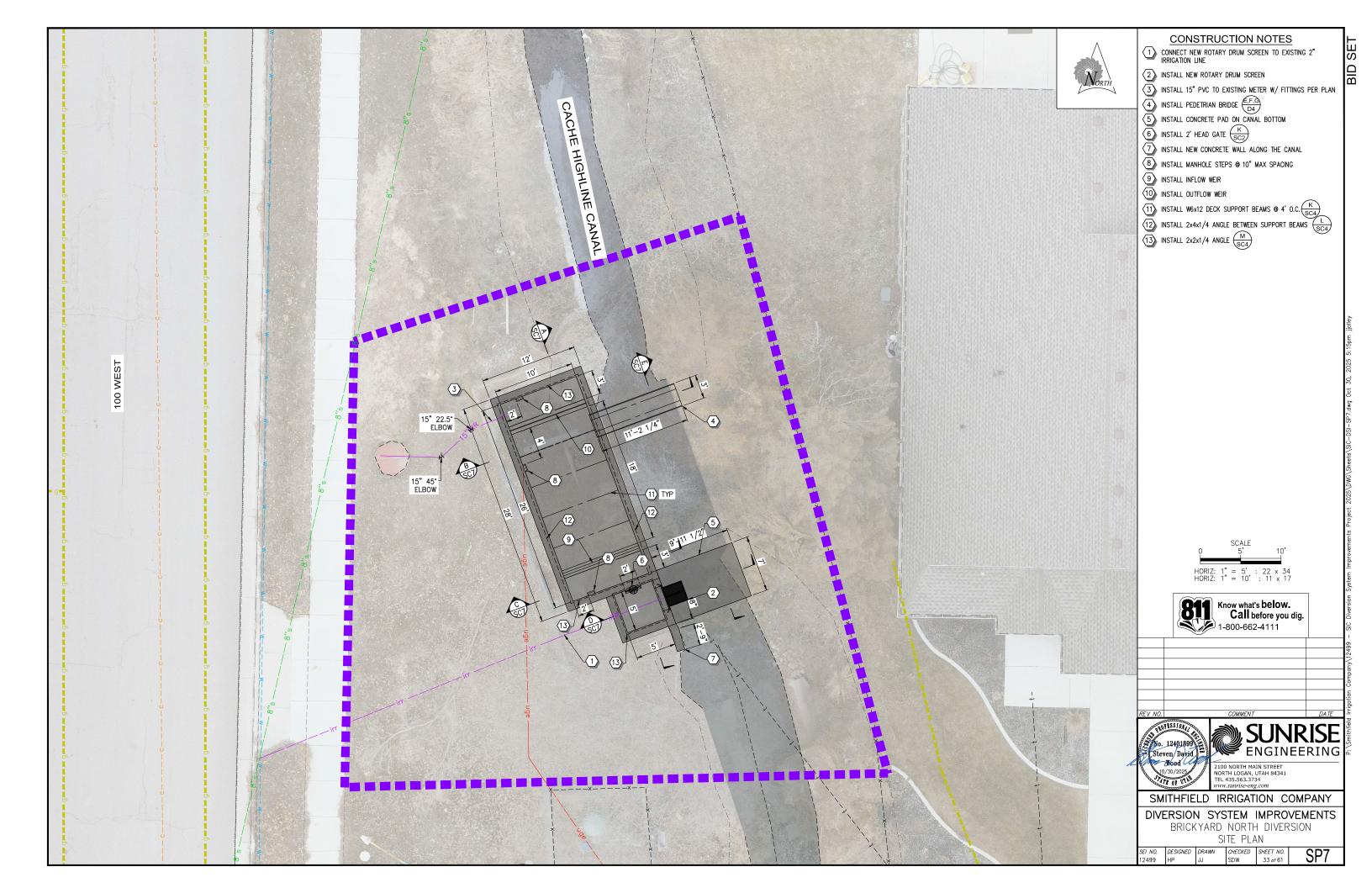


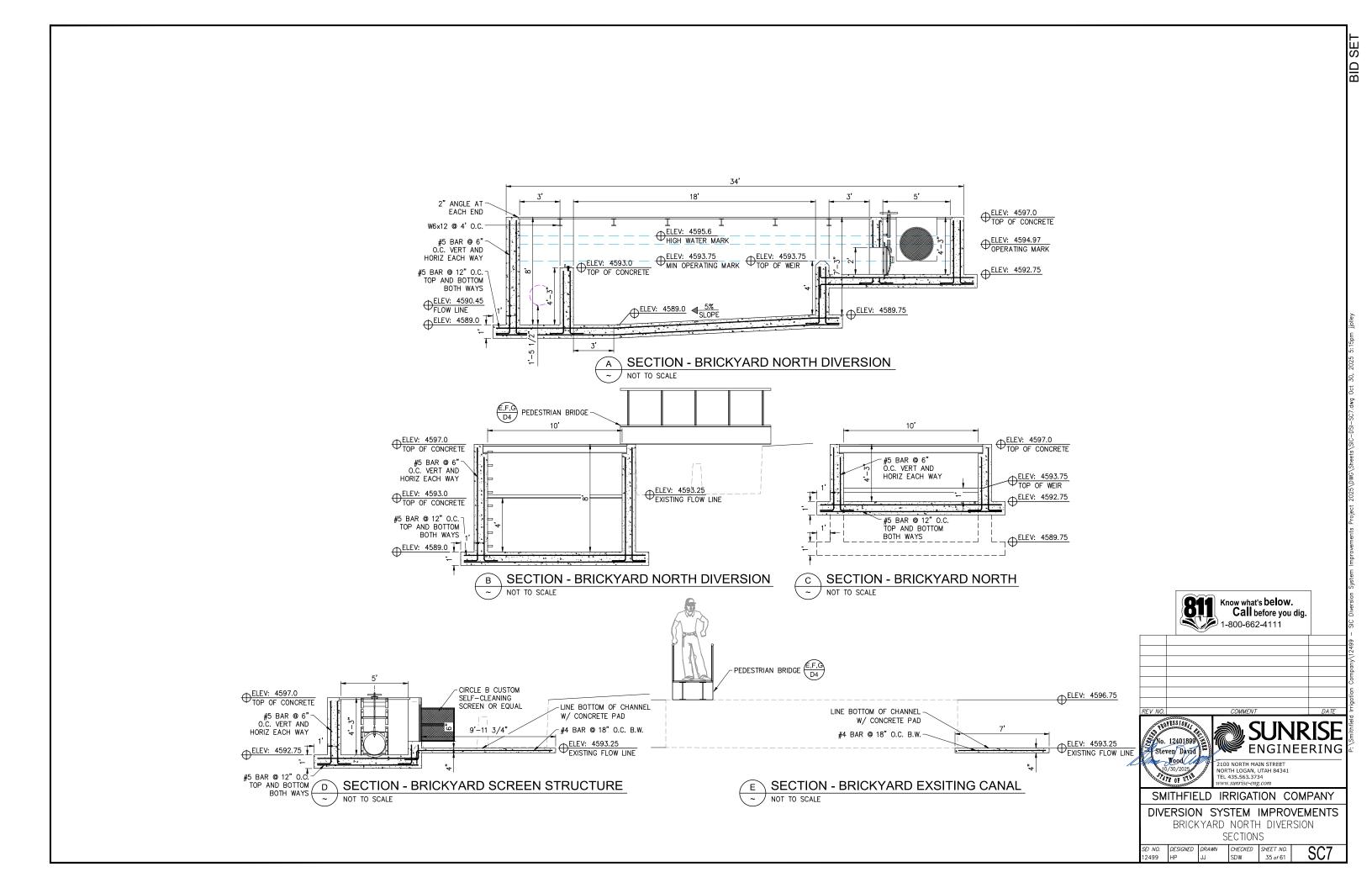


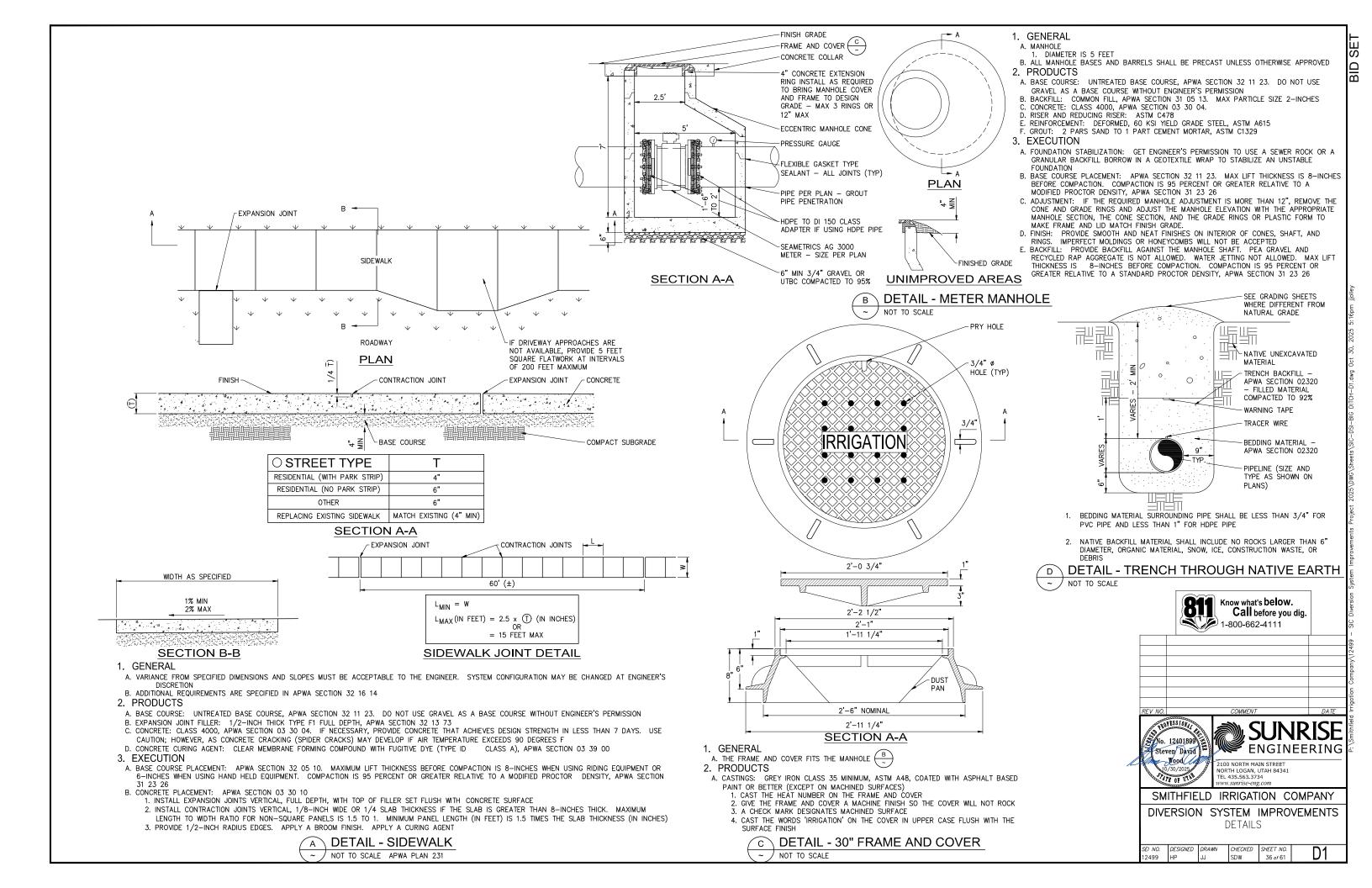










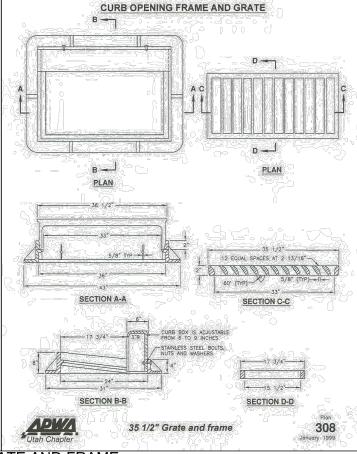


35 1/2" Grate and frame

1. GENERAL

A. The grate and frame fits concrete boxes in Plan 315.

- PRODUCTS
 A. Castings: Grey iron class 35 minimum per ASTM A48, coated with asphalt based paint or better.
- B. Bolts, Nuts, Washers, Accessories: Stainless steel, APWA Section 05 05 23.
- 3. EXECUTION (Not used)



Precast box

1. GENERAL

- A. The drawing shows typical pipe connections. Refer to construction drawings for connection locations or refer to field location of existing piping when engineering pipe connection to the box.

 This drawing is acceptable where the water table elevation is less than 3 feet above
- the floor of the box. If elevation of water table is higher, engineering calculations and drawings must be submitted to and approved by the ENGINEER.

 C. Submit bar design detail for ENGINEER's review.

2. PRODUCTS

- PRODUCTS
 A. Base Course: Untreated base course, APWA Section 32 11 23. Do not use gravel as a base course without ENGINEER's permission.
 B. Backfill: Common fill, APWA Section 31 05 13. Maximum particle size 2-inches.
 C. Precast Concrete: Class 4000 precast, APWA Section 03 40 00.
 D. Reinforcement: Deformed, 60 ksi yield grade steel, ASTM A615. Coated steel is not required for small drainage structures shown on this drawing.
 E. Frame and Cover (or Grate): Use the appropriate unit indicated in the Contract Documents.

- F. Joint Sealant: Rubber-based, compressible.

3. EXECUTION

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- Concrete Placement: Provide 2-inches of concrete cover over reinforcing steel. B. Lifting Points: Provide at least 2 lifting points per section that avoid interference with the reinforcing steel and that are designed according to PCI (Prestressed Concrete
- Institute) design handbook. Lift only from the engineered lifting points.

 C. Depth: Drainage boxes and riser combinations that exceed 8-feet from finished grade to the bottom of the box requires ENGINEER's approval. Submit design calculations and shop drawings.
- D. Core Holes:
- Provide core holes that are at least 4" larger than attaching outer pipe diameter.
 Cut core holes at the manufacturing plant unless ENGINEER permits field core
- Center core holes to leave 2" of concrete measured horizontally from inside wall of the box to core hole. Locate core hole vertically so bottom of core hole will be at or above floor elevation with at least 5-inches of concrete directly above the
- core hole to the top of the box.

 3) Deviations from core hole tolerances require shop drawings. Shop drawings will
- identify lifting point number and location.

 E. Precast Top: Design precast top for AASHTO HL-93 live loads and submit rebar detail and stamped design drawings to ENGINEER. Show connection detail for frame and grate or cover.

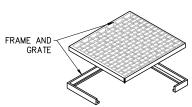
SEI BID OBLIQUE TABLE OF DIMENSIONS SECTION A-A NOTE: DTHER BOX SIZES (LESS THAN 48") WITH DIFFERENT & AND b DIMENSIONS ARE ACCEPTABLE REINFORCING STEEL LAYOUT PROVIDE 2" MIN. COVER ON ALL BARS SECTION B-B 332 Precast box

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NOT TO SCALE

B DETAIL - PRECAST BOX NOT TO SCALE

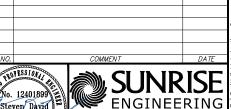


NOTES: 1. CATCH BASINS SHALL MEET ASTM C858 WITH

- AASHTO HS-20 LOADING
- OPENINGS MAY BE LOCATED AS REQUIRED
- FOR CATCH BASIN DETAILS SEE B

© DETAIL - FRAME AND GRATE NOT TO SCALE







SMITHFIELD IRRIGATION COMPANY

DIVERSION SYSTEM IMPROVEMENTS DETAILS

CATCH BASINS

SEI NO. DESIGNED DRAWN CHECKED SHEET NO. SDW

F'c = 4500 PSI								
	TENSION							
BAR SIZE (#)	LTE TOP ①	LTE OTHER	LTS TOP ①	LTS OTHER				
#3	15	12	20	15				
#4	20	16	26	20				
#5	25	19	32	25				
#6	30	23	39	30				
#7	54	42	71	54				
#8	62	48	81	62				

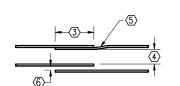
DETAIL - WATER STOP / COLD JOINT AT FOUNDATION WALL

6" FLAT STRIP WATER

STOP OF CONSTANT

THICKNESS

- A. ALL TABULATED VALUES ARE IN UNITS OF INCHES U.N.O.
- B. AT CONTRACTOR'S OPTION, MECHANICAL SPLICE COUPLERS PER G.S.N. MAY BE USED IN LIEU OF LAP SPLICES
- C. SEE G.S.N. FOR ACTUAL CONCRETE SPECIFICATIONS AND MIN. CLR. COVER / CLR. SPACING
- SCHEDULED VALUES ARE BASED ON CLASS "B" TENSION LAP SPLICES U.N.O., NORMAL WT. CONCRETE, AND UNCOATED GRADE 60 REINF. FOR OTHER CONDITIONS NOTED BELOW, MODIFY TABULATED VALUES AS INDICATED:
- E. E.1. FOR DEVELOPMENT LENGTH AND CLASS "A" LAP SPLICES, WHERE SPECIFICALLY NOTED ON PLANS OR DETAILS, DIVIDE TABULATED VALUES BY 1.3. CLASS "A" SPLICES SHALL BE LOCATED SUCH THAT NO MORE THAN 1/2 OF THE TOTAL REINF. IS LAPPED WITHIN THE REQUIRED LAP LENGTH
 - E.2. FOR LIGHTWEIGHT CONCRETE, MULTIPLY TABULATED VALUES BY 1.3 E.3. FOR EPOXY COATED REBAR, MULTIPLY TABULATED VALUES BY 1.5
- E.4. FOR GRADE 75 REINF., MULTIPLY TABULATED VALUES BY 1.25
- F. LCE = COMPRESSION EMBEDMENT LENGTH LCS = COMPRESSION LAP SPLICE LENGTH
 - LTE = TENSION EMBEDMENT LENGTH
- "TOP" BARS ARE HORIZONTAL BARS PLACED SUCH THAT MORE THAN 12 IN. OF FRESH CONCRETE IS CAST BELOW BAR. ALL BARS THAT ARE NOT "TOP" BARS ARE "OTHER" BARS UNLESS NOTED OTHERWISE ALL HOOKS SHALL EXTEND TO THE FAR FACE (LESS 2"



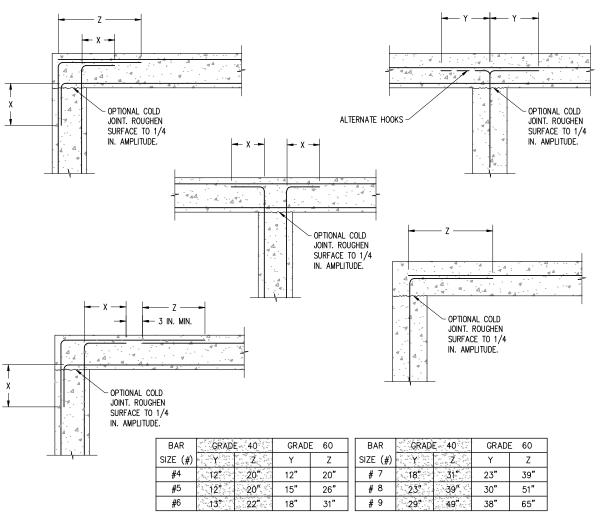
COLD JOINT - ROUGHEN SURFACE TO 1/4" AMPLITUDE

SURFACE FINISH PER

SPECIFICATIONS

CONCRETE LAP AND DEVELOPMENT NOTES

- TOP BARS ARE HORIZ. BARS PLACED SUCH THAT MORE THAN 12" OF FRESH CONCRETE IS CAST IN MEMBER BELOW SPLICE
- WHERE BARS OF UNEQUAL SIZE LAP ONE ANOTHER, USE TABULATED LAP LENGTH FOR SMALLER BAR U.N.O.
- 3 LAP SPLICE LENGTH PER SCHEDULE
- CLEAR DISTANCE BETWEEN ADJACENT BARS OR SPLICES TO BE USED IN DETERMINING APPLICABLE LAP LENGTH FROM SCHEDULE
- (5) OPTIONAL OFFSET. SEE STANDARD REBAR BEND DETAILS FOR OFFSET REQUIREMENTS
- (6) FOR NON-CONTACT LAP SPLICES, MIN. CLEAR DISTANCE BETWEEN SPLICED BARS SHALL BE PER GENERAL STRUCTURAL NOTES. MAX. CLEAR DISTANCE SHALL BE 1/5 THE TABULATED LAP LENGTH OR (6"- "DB"), WHICHEVER IS



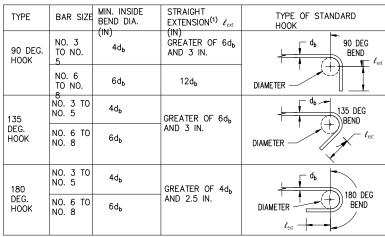
REFER TO STD. HOOK DETAIL FOR LENGTH OF DIMENSION "X"

DETAIL - HORIZONTAL REINFOREMENT AT WALL INTERSECTIONS

2. THIS DETAIL TO BE USED ONLY WHEN NO OHTER DETAIL IS INDICATED ON THE DRAWINGS.

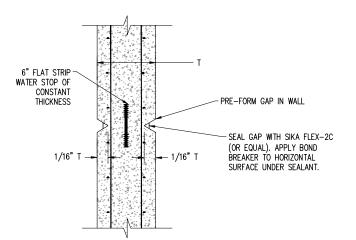
DETAIL - CONCRETE LAP / /DEVELOPMENT SCHEDULESS, WHERE "DB" = BAR DIA.

NOT TO SCALE

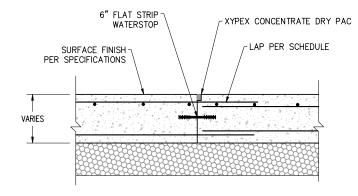


(1)— A STANDARD HOOK FOR STIRRUPS, TIES, AND HOOPS INCLUDES THE SPECIFIC INSIDE BEND DIAMETER AND STRAIGHT EXTENSION LENGTH. IT SHALL BE PERMITTED TO USE A LONGER STRAIGHT EXTENSION AT THE END OF A HOOK. A LONGER EXTENSION SHALL NOT BE CONSIDERED TO INCREASE THE ANCHORAGE CAPACITY OF THE HOOK.

DETAIL - MIN BENDS & HOOK GEOMETRY NOT TO SCALE STIRRUPS, TIES, AND HOOPS IN CONCRETE

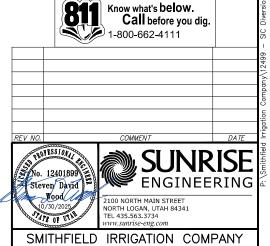






NOT TO SCALE

F DETAIL - TYP FLOOR CONTROL / COLD JOINT NOT TO SCALE

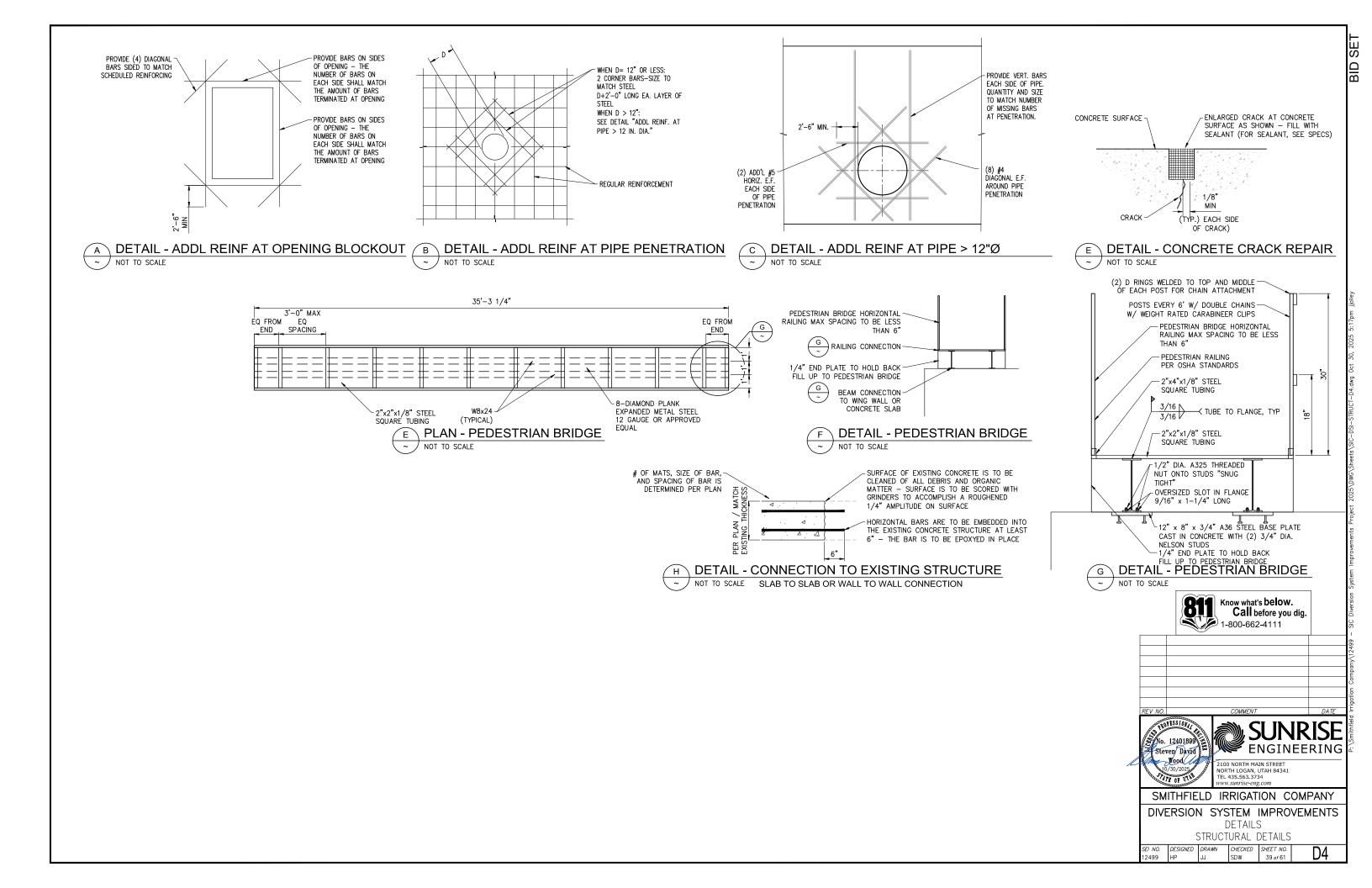


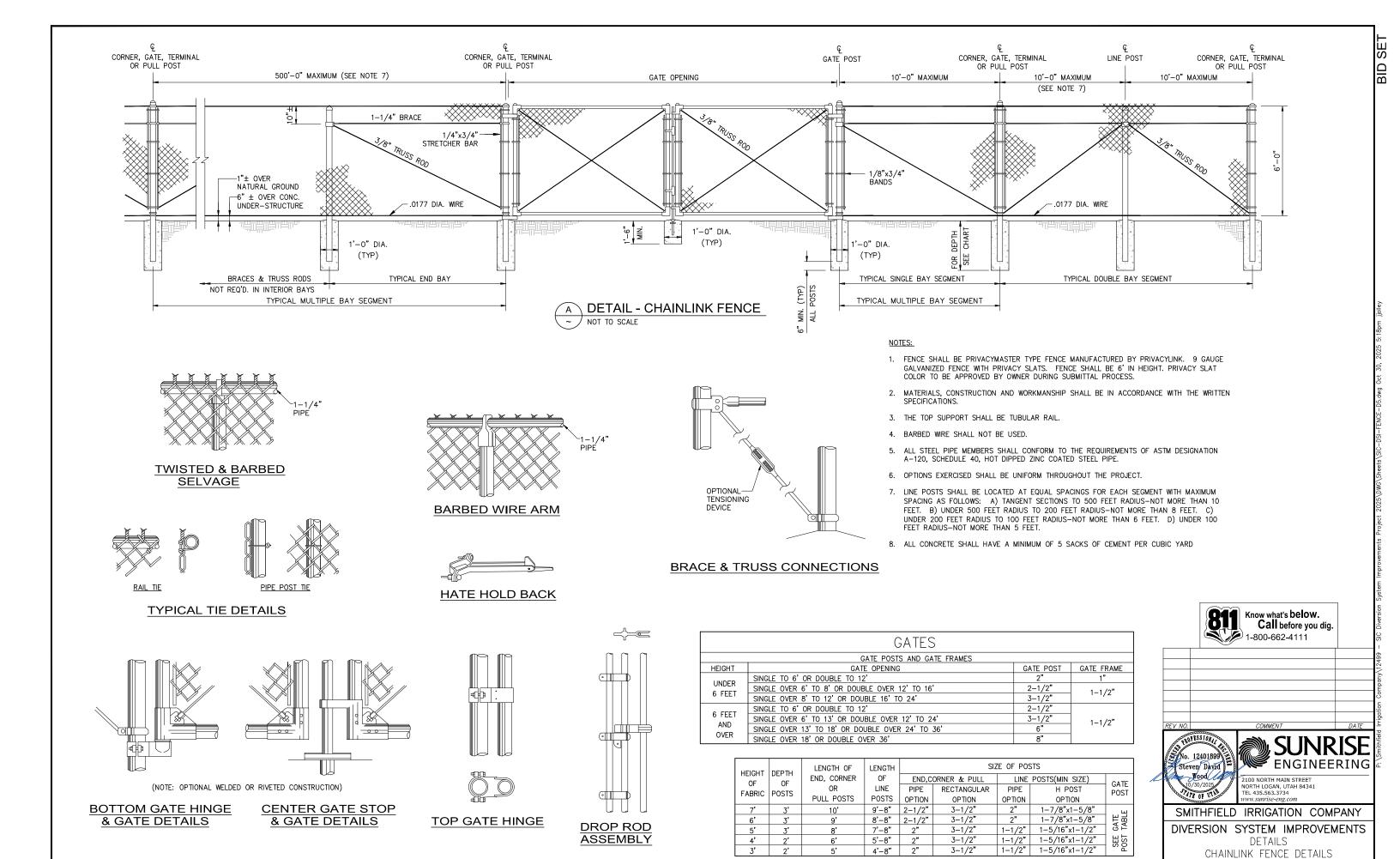
BID

DIVERSION SYSTEM IMPROVEMENTS DETAILS STRUCTURAL DETAILS SEI NO. DESIGNED DRAWN CHECKED SHEET NO.

SDW

NOT TO SCALE



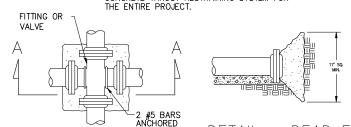


SEI NO. DESIGNED DRAWN

CHECKED SHEET NO.

CONCRETE THRUST BLOCK RESTRAINING SYSTEM DETAILS

CONTRACTOR SHALL USE EITHER MEGALUG OR CONCRETE THRUST RESTRAINING SYSTEM FOR







8.0

- 184

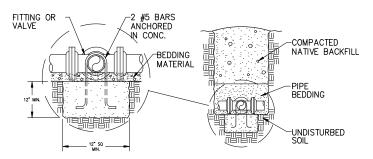
26.3

BEND

ANGLE

11.25

22.5



SECTION A-A

THF	RUST	BLC)CK	BEAF	RING	ARE	A IN	SQ.	FT.
PIPE SIZE	CROSS	TEE	WYE	DEAD END	VALVE	90° BEND	45° BEND	22.5° BEND	11.25° BEND
4	2.4	1.7	1.7	1.7	1.7	2.4	1.3	1.0	1.0
6	5.4	3.8	3.8	3.8	3.8	5.4	2.9	1.5	1.0
8	9.6	6.8	6.8	6.8	6.8	9.6	5.2	2.7	1.4
10	15.0	10.6	10.6	10.6	10.6	15.0	8.1	4.2	2.1
12	21.5	15.2	15.2	15.2	15.2	21.5	11.7	6.0	3.0
14	29.3	20.7	20.7	20.7	20.7	29.3	15.9	8.1	4.1
16	38.2	27.0	27.0	27.0	27.0	38.2	20.7	10.6	5.3
18	48.4	34.2	34.2	34.2	34.2	48.4	26.2	13.4	6.7
20	59.7	42.2	42.2	42.2	42.2	59.7	32.3	16.5	8.3
24	85.9	60.8	60.8	60.8	60.8	85.9	46.5	23.7	11.9
				RED	UCE	₹			
D2 P1	6	8	10	12	14	16	18	20	24
4	2.1	5.1	8.8	13.4	-	_	-	-	_
6	_	3.0	6.7	11.3	16.8	23.1	-	-	-
8	-	-	3.8	8.4	13.8	20.1	27.2	_	-
10	-	-	-	4.6	10.1	16.3	23.5	31.4	_
12	-	_	-	_	5.5	11.7	18.8	26.8	45.2
14	_	-	-	_	-	6.3	13.4	21.3	39.8
16	_	_	_	_	_	_	7.1	15.1	33.5

20 NOTES:

- 1. ALL THRUST BLOCK BEARING FACES SHALL BE PLACED AGAINST UNDIS-TURBED SOIL OR APPROVED COMPACTED BACKFILL.
- 2. SQUARE ALL EDGES TO RECEIVE CONCRETE.
- 3. CONCRETE SHALL BE CLASS C.
- 4. CALCULATED ON 200 PSI TEST PRESSURE AND ALLOWABLE BEARING PRESSURE OF 1500 LBS. PER SQ. FT.
- 5. IN POORER SOILS SPECIAL DESIGN IS REQUIRED.
- 6. ALL FITTINGS AND VALVES SHALL BE ANCHORED WITH TWO #5 REBAR. ONLY ONE REQ'D FOR REDUCER
- 7. CENTER ALL FITTINGS OVER THRUST BLOCKS.

DETAIL - THRUST BLOCK NOT TO SCALE

MEGALUG

16

18

CONTRACTOR SHALL USE EITHER MEGALUG OR CONCRETE THRUST RESTRAINING SYSTEM FOR THE ENTIRE PROJECT. THRUST RESTRAINING SYSTEM DETAILS

* CALCULATIONS DERIVED FROM EBAA IRON SALES

BASED ON: TEST PRESSURE 200 PSI SOIL TYPE: GM - SILTY GRAVEL, GRAVEL-SAND-SILT MIXTURE BURIAL DEPTH: 4 FT.

TRENCH TYPE: 5 - PIPE BEDDED IN COMPACTED GRANULAR

IN FT.

112/66 | 188/91

47/37 103/67

22/19 72/53

45/37

76/52 141/80

24

EXAMPLE.

20

158/77

18

79/52

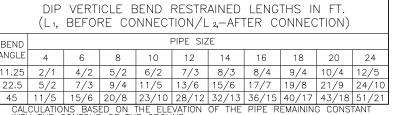
48/37

22/19

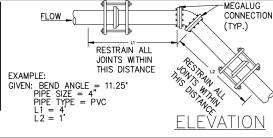
MATERIAL TO THE CENTER LINE OF PIPE, 4" MIN. UNDER PIPE. COMPACTED GRANULAR OR SELECT MATERIAL TO TOP OF PIPE. (APPROX. 90% STANDARD PROCTOR, AASHTO T-99) SAFETY FACTOR: 1.5

SE.

BID



CALCULATIONS BASED ON THE ELEVATION OF WITH THE CONTOUR OF THE GROUND.



PVC	HORI	ZONT.	al Be	END F	RESTR	RAINED	LEN	IGTHS	L,	IN	FT.
BEND					PIPE SIZ	Œ.					
ANGLE	4	6	8	10	12	14	16	18	20		24
11.25	1	2	2	3	3	4	4	5	5		6
22.5	3	4	5	6	7	8	9	9	10		12
45	5	8	10	12	14	16	18	20	21		25
90	13	18	24	29	34	38	43	47	52		60
1 Δ	ALOIL II	TS WITH	IN THE	"I" DIS	TANCE S	SHALL BE	REST	PAINED			

PVC VERTICLE BEND RESTRAINED LENGTHS IN FT.

(L1,-BEFORE CONNECTION/L2, AFTER CONNECTION)

PIPE SIZE

45 | 16/5 | 23/8 | 30/10 | 36/12 | 43/14 | 49/16 | 56/18 | 62/20 | 68/21 | 79/25

CALCULATIONS BASED ON THE ELEVATION OF THE PIPE REMAINING CONSTANT THE CONTOUR OF THE GROUND. FOR TWO WAY FLOW, SUCH AS FOUND IN DISTRIBUTION SYSTEMS, USE L, ON BOTH SIDES OF FITTING.

12 | 14

8/3 | 11/4 | 15/5 | 17/6 | 21/7 | 24/8 | 27/9 | 30/9 | 33/10 | 38/12

5/2 7/2 9/3 10/3 12/4 13/4 15/5 16/5 19/6

10

HOM	ZONTA	4F DE	NU K	(E21K	AINED	LEIM	91H2	L, IIV	l Fl.
BEND PIPE SIZE									
4	6	8	10	12	14	16	18	20	24
1	2	2	2	3	3	4	4	4	5
2	3	4	5	6	6	7	8	9	10
5	6	8	10	12	13	15	17	18	21
11	15	20	24	28	32	36	40	44	51
	1 2 5	4 6 1 2 2 3 5 6	4 6 8 1 2 2 2 3 4 5 6 8	FINAL DEND IX 4 6 8 10 1 2 2 2 2 3 4 5 5 6 8 10	PIPE SIZ 4 6 8 10 12 1 2 2 2 3 2 3 4 5 6 5 6 8 10 12	PIPE SIZE 4 6 8 10 12 14 1 2 2 2 3 3 3 2 3 4 5 6 6 5 6 8 10 12 13	PIPE SIZE 4 6 8 10 12 14 16 1 2 2 2 3 3 4 5 6 6 7 5 6 8 10 12 13 15	PIPE SIZE 4 6 8 10 12 14 16 18 1 2 2 2 3 3 4 4 2 3 4 5 6 6 7 8 5 6 8 10 12 13 15 17	PIPE SIZE 4 6 8 10 12 14 16 18 20 1 2 2 2 3 3 4 4 4 4 2 3 4 5 6 6 7 8 9 5 6 8 10 12 13 15 17 18

	EXAMPLE: GIVEN: BEND ANGLE = 45' PIPE SIZE = 6" PIPE TYPE = PVC L = 8' RESTRAIN ALL JOINTS WITHIN THIS DISTANCE	MEGALUG CONNECTION (TYP.)
_	<u>PLAN</u>	•

PVC TEE RESTRAINED LENGTHS L, IN FT.													
BRANCH SIZE		RUN SIZE DIAMETER											
DIA.	4	6	8	10	12	14	16	18	20	24			
4	*	*	*	*	*	*	*	*	*	*			
6	_	*	*	*	*	*	*	*	*	*			
8	_	_	*	*	*	*	*	*	*	*			
10	_	_	_	*	*	*	*	*	*	*			
12	-	_	-	-	*	*	*	*	*	*			
14	_	_	-	-	_	*	*	*	*	*			
16	-	-	_	_	_	_	7	*	*	*			
18	-	_	-	-	_	_	-	20	4	*			
20	_	_	_	_	_	_	_	-	34	3			
24	_	_	_	_	_	-	_	_	_	60			
* =	= FOR ⁻	THIS CO	NDITION	NFFD (NIY RF	STRAIN	THE OU	TLETS O	F TFF				

- NOTES:
- 1. RESTRAIN THE THREE MECHANICAL JOINTS ON THE TEE.
 2. ALL JOINTS WITHIN THE "L" DISTANCE ON THE BRANCH SIDE OF TEE SHALL BE RESTRAINED AND ALL JOINTS WITHIN 18' ON THE RUN SIDE OF THE TEE

DH	p te	EE F	REST	RAIN	1ED	LEN	GTH:	SL,	IN	FT.			
BRANCH SIZE	RUN SIZE DIAMETER												
DIA.	4	6	8	10	12	14	16	18	20	24			
4	*	*	*	*	*	*	*	*	*	*			
6	-	*	*	*	*	*	*	*	*	*			
8	_	_	*	*	*	*	*	*	*	*			
10	-	_	-	*	*	*	*	*	*	*			
12	-	_	-	_	*	*	*	*	*	*			
14	_	_	_	_	_	*	*	*	*	*			
16	_	_	_	_	_	_	12	2	*	*			
18	_	_	_	_	_	ı	_	21	12	*			
20	_	_	_	_	-	_	_	_	30	12			
24	_	_	_	_	_	ı	_	_	_	47	EXAM		
* =	= FOR ⁻	THIS CO	NDITION	NEED (ONLY RE	STRAIN	THE OU	TLETS O	F TEE	-	PI		

DIP REDUCER RESTRAINED LENGTHS

- 26/20 57/35 95/48 140/61 192/73

24/19 53/35

1. ALL JOINTS WITHIN THE "L" DISTANCE SHALL BE RESTRAINED

12

23/20

10

27/18 | 65/34 | 110/46 | 166/58

RESTRAIN THE THREE MECHANICAL JOINTS ON THE TEE.
 ALL JOINTS WITHIN THE "L" DISTANCE ON THE BRANCH SIDE OF TEE SHALL BE RESTRAINED AND ALL JOINTS WITHIN 18' ON THE RUN SIDE OF THE TEE.

SMALL SIDE/LARGE SIDE

14

87/50 127/63

23/20 49/37

51/36 82/51 118/64

22/20

		<u> </u>	
-	18' MIN. (DIP) 20' MIN. (PVC)	18' MIN. (DIP) 20' MIN. (PVC)	.1
	RESTRAIN ALL JOINTS WITHIN THIS DISTANCE	RESTRAIN ALL JOINTS WITHIN THIS DISTANCE	
	RUN		
COP AMPLE: PIPE TYPE = [DIP	RESTRAIN ALL JOINTS WITHIN THIS DISTANCE	
PIPE SIZE = 10 L = 12'	6"		
	<u>PL</u>	<u>_AN</u>	

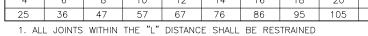
PV	PVC REDUCER RESTRAINED LENGTHS L, IN FT.													
	(SMALL SIDE/LARGE SIDE)													
D2 D1	6	8	10	12	14	16	18	20	24					
4	42/29	100/52	171/71	258/90	_	_	_	_	_					
6	_	40/31	88/54	147/75	217/95	297/113	_	_	_					
8	_	_	37/29	82/55	135/77	197/98	266/117	_	_					
10	-	-	-	36/30	79/56	128/80	183/100	244/120	-					
12	_	-	-	-	36/30	77/57	123/80	174/102	293/142					
14	_	_	_	_	_	35/30	74/57	118/81	219/124					
16	_	_	ı	_	_	_	34/30	73/57	161/104					
18	_	_	-	_	_	_	_	34/30	111/82					
20	_	_	ı	_	_	_	_	_	70/57					
1 A	TAIOL LL	MIHTIW 2	THE "I'	, DISTVII	OF SHALL	BE BES	TRAINED							

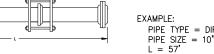
(SMALL SIDE/LARGE SIDE)													
D2 D1	6	8	10	12	14	16	18	20	24				
4	42/29	100/52	171/71	258/90	_	_	_	_	_				
6	_	40/31	88/54	147/75	217/95	297/113	_	_	_				
8	_	_	37/29	82/55	135/77	197/98	266/117	_	_				
10	-	-	-	36/30	79/56	128/80	183/100	244/120	-				
12	-	-	_	_	36/30	77/57	123/80	174/102	293/142				
14	_	_	_	_	_	35/30	74/57	118/81	219/124				
16	_	_	-	_	_	-	34/30	73/57	161/104				
18	_	_	_	_	_	_	_	34/30	111/82				
20	_	_	_	_	_	_	_	_	70/57				
1 A	TAIOL IL	S WITHIN	THE "L'	' DISTANO	CE SHALL	BE RES	STRAINED						

	PVC	DEAL) EN	D RE	STRAI	NED	LENG	THS	L, IN	FT.			
ı	PIPE SIZE												
	4	6	8	10	12	14	16	18	20	24			
I	39	55	73	88	104	119	134	149	163	192			

1. ALL JOINTS WITHIN THE "L" DISTANCE SHALL BE RESTRAINED

DIP	DEAD	EN[) RES	STRAII	NED	THS	L, IN	FT.			
PIPE SIZE											
4	6	8	10	12	14	16	18	20	24		
25	36	47	57	67	76	86	95	105	123		





PIPE TYPE = DIP



10

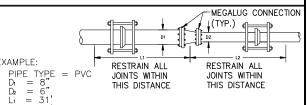
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B DETAIL - MECHANICAL THRUST RESTRAINING SYSTEM DETAILS FOR PRESSURE SYSTEMS ~ / NOT TO SCALE









SMITHFIELD IRRIGATION COMPANY

MILL RACE PRESSURIZED PIPELINE DETAILS

THRUST BLOCK DETAILS SEI NO. DESIGNED DRAWN CHECKED SHEET NO.

SDW