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ADDITIONALLY, DRAWINGS MAY NOT BE RE-SCALED WHEN PRINTED, WRITTEN DIMENSIONS SHALL HAVE PRECEDENCE, AND LARGER SCALE DRAWINGS SHALL HAVE PRECEDENCE OVER SMALLER SCALE

ANY DEVIATION FROM OR CONFLICT WITHIN THE DRAWINGS AND/OR SPECIFICATIONS. MUST BE SUBMITTED VIA REQUEST FOR INFORMATION (RFI) AND RESPONDED TO BY THE ARCHITECT PRIOR TO BID OR BEFORE CONTINUING THAT PORTION OF WORK.

BID ALTERNATES

(REFER TO SPECIFICATIONS 01 2300-ALTERNATES FOR FULL DESCRIPTION OF ALTERNATES)

BID ALTERNATE 1
• REMOVAL OF CLASSROOMS

BID ALTERNATE 2 ROOFING MEMBRANE

DEFFERED SUBMITTALS

 FIRE ALARMS 2. FIRE PROTECTION AND SEISMIC DESIGNS 3. OPEN WEB JOISTS AND RELATED COMPONENTS

4. COLD FORM FRAMING 5. SEISMIC BRACING OF ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL ITEMS STEEL STAIRS

BID PACKAGE NOTES

SHEET REVISIONS WILL BE ISSUED ACCORDING TO THEIR BID PACKAGE. (EX: ASI-1/BP-1)

<u>notes:</u> Bid package 1

CIVIL / UNDERGROUND MEP / FOUNDATION (BC.....ISSUED FOR BIDDING AND CONSTRUCTION) (RO.....ISSUED FOR REFERENCE ONLY)

BID PACKAGE 2

LANDSCAPE / STRUCTURAL / ARCHITECTURAL / MECH., PLUMBING, & FIRE / ELECTRICAL / KITCHEN (BC.....ISSUED FOR BIDDING AND CONSTRUCTION) (RR/WR.....RE-RELEASED w/ REVISIONS)

SHEET INDEX

VO	LUME 1	124 - BID PACKAGE - bid package 2			124 - BID PACKAGE - BID PACKAGE 2			024 - BID PACKAGI 1 - BID PACKAGE 2	VO	LUME 2)24 - BID PACKAGE
SHEET #	SHEET CONTENTS	03.07.20	SHEET #	SHEET CONTENTS	03.07.20	SHEET #	SHEET CONTENTS	03.07.2	SHEET #	SHEET CONTENTS	03.07.20
GENERAL:						A-413	ENLARGED VIEWS	BC	MECHANIC		
G-001	COVER SHEET	BC	ARCHITECT		DO DD	A-414 A-415	ENLARGED VIEWS ENLARGED VIEWS	BC BC	M-000	MECHANICAL TITLE SHEET	
G-002 G-003	PROJECT INFORMATION PLAN - SITE CODE	BC RO BC	A-101 A-102	PLAN - OVERALL LEVEL 1 PLAN - OVERALL LEVEL 2	BC RR BC RR	A-415 A-416	ENLARGED VIEWS ENLARGED VIEWS	BC	M-001 M-101	MECHANICAL GENERAL NOTES MECHANICAL HVAC PLAN - LEVEL 01 - OVERALL	
G-003	CODE ANALYSIS	RC BC	A-102 A-111.0	PLAN - LEVEL 01 - AREA A - SLAB & FOUNDATION	BC RR	A-417	ENLARGED VIEWS	BC	M-102	MECHANICAL HVAC PLAN - LEVEL 01 - OVERALL MECHANICAL HVAC PLAN - LEVEL 02 - OVERALL	
G-005	PLAN - CODE	BC	A-111.1	PLAN - LEVEL 01 - AREA A - DIMENSION	BC	A-418	ENLARGED VIEWS	BC	M-103	MECHANICAL HVAC PLAN - ROOF - OVERALL	
G-006	PLAN - CODE	BC	A-111.2	PLAN - LEVEL 01 - AREA A - ANNOTATION	BC	A-419	ENLARGED VIEWS	ВС	M-111.A	MECHANICAL HVAC PLAN - LEVEL 01 - AREA A	
G-007	PLAN - CODE	BC	A-111.3	PLAN - LEVEL 01 - AREA A - FINISH	BC	A-420	ENLARGED VIEWS	BC BC	M-111.B	MECHANICAL HVAC PLAN - LEVEL 01 - AREA B	
011/11 -			A-111.4	PLAN - LEVEL 01 - AREA A - REFLECTED CEILING	BC BD	A-421 A-422	ENLARGED VIEWS ENLARGED VIEWS	BC BC	M-111.C	MECHANICAL HVAC PLAN - LEVEL 01 - AREA C	
CIVIL: C-001	CIVIL GENERAL NOTES	BC	A-112.0 A-112.1	PLAN - LEVEL 01 - AREA B - SLAB & FOUNDATION PLAN - LEVEL 01 - AREA B - DIMENSION	BC RR	A-423	ENLARGED VIEWS	BC	M-111.D M-111.E	MECHANICAL HVAC PLAN - LEVEL 01 - AREA D MECHANICAL HVAC PLAN - LEVEL 01 - AREA E	
C-101	CIVIL - DEMOLITION PLAN	BC	A-112.2	PLAN - LEVEL 01 - AREA B - ANNOTATION	BC	A-424	ENLARGED VIEWS	ВС	M-111.F	MECHANICAL HVAC PLAN - LEVEL 01 - AREA F	
C-201	CIVIL - OVERALL SITE PLAN	RO	A-112.3	PLAN - LEVEL 01 - AREA B - FINISH	BC	A-425	INTERIOR ELEVATIONS	BC	M-112.A	MECHANICAL HVAC PLAN - LEVEL 02 - AREA A	
C-202	CIVIL - SITE PLAN NORTHWEST	R0	A-112.4	PLAN - LEVEL 01 - AREA B - REFLECTED CEILING	ВС	A-426	INTERIOR ELEVATIONS	BC	M-112.B	MECHANICAL HVAC PLAN - LEVEL 02 - AREA B	
C-203	CIVIL - SITE PLAN NORTHEAST	RO	A-113.0	PLAN - LEVEL 01 - AREA C - SLAB & FOUNDATION	BC RR	A-427 A-428	INTERIOR ELEVATIONS INTERIOR ELEVATIONS	BC BC	M-112.C	MECHANICAL HVAC PLAN - LEVEL 02 - AREA C	
C-204 C-205	CIVIL - SITE PLAN EAST CIVIL - SITE PLAN SOUTHEAST	R0 R0	A-113.1	PLAN - LEVEL 01 - AREA C - DIMENSION PLAN - LEVEL 01 - AREA C - ANNOTATION	BC	A-420 A-431	ENLARGED VIEWS - STAIR/ELEVATOR/ LADDER	BC	M-112.E M-112.F	MECHANICAL HVAC PLAN - ROOF - AREA E MECHANICAL HVAC PLAN - LEVEL 02 - AREA F	
C-205 C-301	CIVIL UTILITY PLAN	BC	A-113.2 A-113.3	PLAN - LEVEL 01 - AREA C - ANNOTATION PLAN - LEVEL 01 - AREA C - FINISH	RC	A-432	ENLARGED VIEWS - STAIR/ELEVATOR/ LADDER	BC	M-113.A	MECHANICAL HVAC PLAN - LEVEL 02 - AREA F	
C-302	CIVIL STORMWATER PLAN	RO	A-113.4	PLAN - LEVEL 01 - AREA C - REFLECTED CEILING	BC	A-433	ENLARGED VIEWS - STAIR/ELEVATOR/ LADDER	ВС	M-113.B	MECHANICAL HVAC PLAN - ROOF - AREA B	
C-401	CIVIL - OVERALL GRADING PLAN	ВС	A-114.0	PLAN - LEVEL 01 - AREA D - SLAB & FOUNDATION	BC RR	A-434	ENLARGED VIEWS - ELEVATOR	ВС	M-113.C	MECHANICAL HVAC PLAN - ROOF - AREA C	
C-402	CIVIL - GRADING PLAN NORTHWEST	BC	A-114.1	PLAN - LEVEL 01 - AREA D - DIMENSION	ВС	A-511	DETAILS - METAL STUD TYPICALS	BC	M-113.D	MECHANICAL HVAC PLAN - ROOF - AREA D	
C-403	CIVIL - GRADING PLAN NORTHEAST	BC	A-114.2	PLAN - LEVEL 01 - AREA D - ANNOTATION	BC	A-512 A-531	DETAILS - ADA STANDARDS DETAILS - SLAB & FOUNDATION	BC RC	M-113.F	MECHANICAL ISOMETRIC VIEWS	
C-404 C-405	CIVIL - GRADING PLAN EAST CIVIL - GRADING PLAN SOUTHEAST	BC BC	A-114.3 A-114.4	PLAN - LEVEL 01 - AREA D - FINISH PLAN - LEVEL 01 - AREA D - REFLECTED CEILING	BC BC	A-532	DETAILS - SLAD & TOOKDATION DETAILS - BUILDING	BC	M-301 M-302	MECHANICAL ISOMETRIC VIEWS MECHANICAL ISOMETRIC VIEWS	
C-501	CIVIL UTILITY DETAILS	BC	A-115.0	PLAN - LEVEL 01 - AREA B - NET ELEVIED OCICINA PLAN - LEVEL 01 - AREA E - SLAB & FOUNDATION	BC RR	A-533	DETAILS - BUILDING	BC	M-303	MECHANICAL ISOMETRIC VIEWS MECHANICAL ISOMETRIC VIEWS	
C-502	CIVIL UTILITY DETAILS	BC	A-115.1	PLAN - LEVEL 01 - AREA E - DIMENSION	BC	A-534	DETAILS - BUILDING	ВС	M-304	MECHANICAL ISOMETRIC VIEWS	
C-601	CIVIL - ROADWAY PLAN & PROFILE 3020 S / 920 W	BC	A-115.2	PLAN - LEVEL 01 - AREA E - ANNOTATION	BC	A-541	DETAILS - ROOF	BC	M-401	MECHANICAL ENLARGED VIEWS	
C-602	CIVIL - ROADWAY PLAN & PROFILE 3200 SOUTH	R0	A-115.3	PLAN - LEVEL 01 - AREA E - FINISH	BC	A-542	DETAILS - ROOF	BC BC	M-402	MECHANICAL ENLARGED VIEWS	
LANDCCADE			A-115.4	PLAN - LEVEL 01 - AREA E - REFLECTED CEILING	BC BD	A-543 A-544	DETAILS - CANOPY DETAILS - CANOPY	BC BC	M-403	MECHANICAL ENLARGED VIEWS	
LANDSCAPE L-100	:: OVERALL PLANTING PLAN	BC.	A-116.0 A-116.1	PLAN - LEVEL 01 -AREA F - SLAB & FOUNDATION PLAN - LEVEL 01 - AREA F - DIMENSION	BC RR	A-545	DETAILS - CANOPY	BC	M-404 M-501	MECHANICAL ENLARGED VIEWS MECHANICAL DETAILS	
L-101	PLANTING PLAN AREA A	BC	A-116.2	PLAN - LEVEL 01 - AREA F - ANNOTATION	BC	A-551	DETAILS - STAIRS	BC	M-502	MECHANICAL DETAILS	
L-102	PLANTING PLAN AREA B	BC	A-116.3	PLAN - LEVEL 01 - AREA F - FINISH	BC	A-561	SCHEDULE - DOOR & TYPE NOTES	BC	M-503	MECHANICAL DETAILS	
L-103	PLANTING PLAN AREA C	BC	A-116.4	PLAN - LEVEL 01 - AREA F - REFLECTED CEILING	BC	A-571	SCHEDULE - FRAME TYPES	BC	M-504	MECHANICAL DETAILS	
L-104	PLANTING PLAN AREA D	BC	A-121.0	PLAN - LEVEL 02 - AREA A - SLAB	BC RR	A-572 A-573	SCHEDULE - FRAME TYPES DETAILS - WINDOWS	BC BC	M-505	MECHANICAL DETAILS	
L-501	LANDSCAPE DETAILS	BC	A-121.1	PLAN - LEVEL 02 - AREA A - DIMENSION	BC	A-573 A-574	DETAILS - WINDOWS DETAILS - WINDOWS	BC.	M-506	MECHANICAL DETAILS MECHANICAL SCHEDULES	
STRUCTURA	14		A-121.2 A-121.3	PLAN - LEVEL 02 - AREA A - ANNOTATION PLAN - LEVEL 02 - AREA A - FINISH	BC:	A-576	DETAILS - DOORS	BC	M-601 M-602	MECHANICAL SCHEDULES MECHANICAL SCHEDULES	
S-001	STRUCTURAL NOTES	BC BC	A-121.4	PLAN - LEVEL 02 - AREA A - REFLECTED CEILING	BC	A-577	DETAILS - DOORS	BC	M-603	MECHANICAL SCHEDULES	
S-002	STRUCTURAL NOTES	BC BC	A-122.0	PLAN - LEVEL 02 - AREA B - SLAB	BC RR	A-581	DETAILS - CEILING	BC	M-604	MECHANICAL SCHEDULES	
S-010	SCHEDULES	BC BC	A-122.1	PLAN - LEVEL 02 - AREA B - DIMENSION	BC	A-591	DETAILS - MILLWORK	BC BC	M-701	MECHANICAL SCHEMATICS	
S-011	SCHEDULES	BC BC	A-122.2	PLAN - LEVEL 02 - AREA B - ANNOTATION	BC	A-592 A-593	DETAILS - MILLWORK DETAILS - MILLWORK	BC	M-702	MECHANICAL SCHEMATICS	
S-012 S-013	SCHEDULES SCHEDULES	BC BC BC	A-122.3 A-122.4	PLAN - LEVEL 02 - AREA B - FINISH PLAN - LEVEL 02 - AREA B - REFLECTED CEILING	BC BC	A-594	DETAILS - INTERIOR	BC	M-703 M-704	MECHANICAL SCHEMATICS MECHANICAL SCHEMATICS	
S-110	FOOTING AND FOUNDATION PLAN - OVERALL	BC BC	A-123.0	PLAN - LEVEL 02 - AREA C - SLAB	BC RR	A-611	ASSEMBLIES - EXTERIOR WALLS, FLOORS, ROOFS	BC	M-705	MECHANICAL SCHEMATICS MECHANICAL SCHEMATICS	
S-111	FOOTING AND FOUNDATION PLAN - AREA A	BC BC	A-123.1	PLAN - LEVEL 02 - AREA C - DIMENSION	BC	A-612	ASSEMBLIES - INTERIOR WALLS	BC	M-706	MECHANICAL SCHEMATICS	
S-112	FOOTING AND FOUNDATION PLAN - AREA B	BC BC	A-123.2	PLAN - LEVEL 02 - AREA C - ANNOTATION	BC	A-613	ASSEMBLIES - INTERIOR WALLS	BC	M-707	MECHANICAL SCHEMATICS	
S-113	FOOTING AND FOUNDATION PLAN - AREA C	BC BC	A-123.3	PLAN - LEVEL 02 - AREA C - FINISH	BC	A-681 A-691 - H	SCHEDULE - SIGNAGE SCHEDULE - FINISH	BC RC	MP-101	MECHANICAL PIPING PLAN - LEVEL 01 - OVERALL	
S-114 S-115	FOOTING AND FOUNDATION PLAN - AREA D FOOTING AND FOUNDATION PLAN - AREA E	BC BC	A-123.4 A-124.1	PLAN - LEVEL 02 - AREA C - REFLECTED CEILING PLAN - LEVEL 02 - AREA F - DIM/ANNO	BC BC	A-691 - N	SCHEDULE - FINISH	BC	MP-102 MP-103	MECHANICAL PIPING PLAN - LEVEL 02 - OVERALL MECHANICAL PIPING PLAN - ROOF - OVERALL	
S-116	FOOTING AND FOUNDATION PLAN - AREA F	BC BC	A-124.1 A-151	PLAN - ROOF - OVERALL	BC	A-692 - H	SCHEDULE - FINISH	BC	MP-111.A	MECHANICAL PIPING PLAN - ROOF - OVERALL MECHANICAL PIPING PLAN - LEVEL 01 - AREA A	
S-120	FLOOR FRAMING PLAN - OVERALL	RO BC	A-161	STORAGE SHED	BC RR	A-692 - N	SCHEDULE - FINISH	BC	MP-111.B	MECHANICAL PIPING PLAN - LEVEL 01 - AREA B	
S-121	FLOOR FRAMING PLAN - AREA A	RO BC	A-201	ELEVATIONS - EXTERIOR	ВС	A-811.0	BID ALT 1 - PLAN - LEVEL 01 - AREA A - SLAB	BC	MP-111.C	MECHANICAL PIPING PLAN - LEVEL 01 - AREA C	_
S-122	FLOOR FRAMING PLAN - AREA B	RO BC	A-202	ELEVATIONS - EXTERIOR	BC	A-811.1	BID ALT 1 - PLAN - LEVEL 01 - AREA A - DIMENSION	BC BC	MP-111.D	MECHANICAL PIPING PLAN - LEVEL 01 - AREA D	
S-123	FLOOR FRAMING PLAN - AREA C	RO BC	A-203 A-204	ELEVATIONS - EXTERIOR ELEVATIONS - EXTERIOR	BC	A-821.0 A-821.1	BID ALT 1 - PLAN - LEVEL 02 - AREA A - SLAB BID ALT 1 - PLAN - LEVEL 02 - AREA A - DIMENSION	RC.	MP-111.E	MECHANICAL PIPING PLAN - LEVEL 01 - AREA E MECHANICAL PIPING PLAN - LEVEL 01 - AREA F	
S-124 S-130	FLOOR FRAMING PLAN - AREA F ROOF FRAMING PLAN - OVERALL	RO BC RO BC	A-204 A-205	ELEVATIONS - EXTERIOR ELEVATIONS - EXTERIOR	BC BC	- 7.021.1	DID TIET I TEME ELVEL OF THEFT DIMENSION		MP-111.F MP-112.A	MECHANICAL PIPING PLAN - LEVEL 01 - AREA F MECHANICAL PIPING PLAN - LEVEL 02 - AREA A	
S-131	ROOF FRAMING PLAN - AREA A	RO BC	A-206	ELEVATIONS - EXTERIOR	BC	_			MP-112.B	MECHANICAL PIPING PLAN - LEVEL 02 - AREA B	
S-132	ROOF FRAMING PLAN - AREA B	RO BC	A-331	SECTIONS - BUILDING	BC	_			MP-112.C	MECHANICAL PIPING PLAN - LEVEL 02 - AREA C	
S-133	ROOF FRAMING PLAN - AREA C	RO BC	A-332	SECTIONS - BUILDING	ВС	_			MP-112.E	MECHANICAL PIPING PLAN - ROOF - AREA E	
S-134	ROOF FRAMING PLAN - AREA D	RO BC	A-333	SECTIONS - BUILDING	BC	_			MP-112.F	MECHANICAL PIPING PLAN - LEVEL 02 - AREA F	
S-135 S-136	ROOF FRAMING PLAN - AREA E ROOF FRAMING PLAN - AREA F	RO BC RO BC	A-334 A-335	SECTIONS - BUILDING SECTIONS - BUILDING	RC RC	_			MP-113.A MP-113.B	MECHANICAL PIPING PLAN - ROOF - AREA A MECHANICAL PIPING PLAN - ROOF - AREA B	
S-136 S-140	SPORTS STORAGE FOOTING, FDN, AND ROOF FRAMING PLAN	BC BC	A-335 A-341	SECTIONS - WALL	RC.	-			MP-113.B MP-113.C	MECHANICAL PIPING PLAN - ROUF - AREA B MECHANICAL PIPING PLAN - ROOF - AREA C	
S-201	TYPICAL DETAILS	BC BC	A-341	SECTIONS - WALL	BC	_			MP-113.D	MECHANICAL PIPING PLAN - ROOF - AREA D	
S-202	TYPICAL DETAILS	BC BC	A-343	SECTIONS - WALL	ВС	_			MP-113.F	MECHANICAL PIPING PLAN - ROOF - AREA F	
S-210	FOOTING & FOUNDATION DETAILS	BC BC	A-344	SECTIONS - WALL	ВС	_			MP-401	MECHANICAL PIPING ENLARGED VIEWS	
S-211	FOOTING & FOUNDATION DETAILS	BC BC	A-345	SECTIONS - WALL	BC	_			MP-402	MECHANICAL PIPING ENLARGED VIEWS	
S-220 S-230	FLOOR FRAMING DETAILS ROOF FRAMING DETAILS	RO BC RO BC	A-346 A-347	SECTIONS - WALL SECTIONS - WALL	BC BC	_			MP-403 MP-404	MECHANICAL PIPING ENLARGED VIEWS MECHANICAL PIPING ENLARGED VIEWS	
S-230 S-231	ROOF FRAMING DETAILS	RO BC	A-347 A-348	SECTIONS - WALL	BC BC	_			MP-404 MP-701	MECHANICAL PIPING ENLARGED VIEWS MECHANICAL PIPING SCHEMATICS	
S-301	ELEVATIONS STATES	RO BC	A-349	SECTIONS - WALL	BC.	_			MP-702	MECHANICAL PIPING SCHEMATICS	

MECHANICAL			P-111.D	PLUMBING PLAN - LEVEL 01 - AREA D	BC	E-413	LEVEL 1 - AREA C - AUDIO VIUSAL
VI-000	MECHANICAL TITLE SHEET	BC	P-111.E	PLUMBING PLAN - LEVEL 01 - AREA E	BC	E-414	LEVEL 1 - AREA D - AUDIO VIUSAL
VI-001	MECHANICAL GENERAL NOTES	ВС	P-111.F	PLUMBING PLAN - LEVEL 01 - AREA F	BC	E-415	LEVEL 1 - AREA E - AUDIO VIUSAL
VI-101	MECHANICAL HVAC PLAN - LEVEL 01 - OVERALL	BC	P-112.A	PLUMBING PLAN - LEVEL 02 - AREA A	BC	E-416	LEVEL 1 - AREA F - AUDIO VIUSAL
VI-102	MECHANICAL HVAC PLAN - LEVEL 02 - OVERALL	BC	P-112.B	PLUMBING PLAN - LEVEL 02 - AREA B	BC	E-421	LEVEL 2 - AREA A - AUDIO VIUSAL
VI-103	MECHANICAL HVAC PLAN - ROOF - OVERALL	BC	P-112.C	PLUMBING PLAN - LEVEL 02 - AREA C	BC	E-422	LEVEL 2 - AREA B - AUDIO VIUSAL
VI-111.A	MECHANICAL HVAC PLAN - LEVEL 01 - AREA A	BC	P-401	PLUMBING ENLARGED VIEWS	BC	E-423	LEVEL 2 - AREA C - AUDIO VIUSAL
M-111.B	MECHANICAL HVAC PLAN - LEVEL 01 - AREA B	BC	P-402	PLUMBING ENLARGED VIEWS	BC	E-431	AUDIO VISUAL SECTIONS AND ELEVATIONS
M-111.C	MECHANICAL HVAC PLAN - LEVEL 01 - AREA C	BC	P-403	PLUMBING ENLARGED VIEWS	BC	E-432	AUDIO VISUAL SECTIONS AND ELEVATIONS
M-111.D	MECHANICAL HVAC PLAN - LEVEL 01 - AREA D	BC	P-404	PLUMBING ENLARGED VIEWS	BC	E-441	AUDIO VISUAL ENLARGED PLANS - TYPICAL CLASSRI
VI-111.E	MECHANICAL HVAC PLAN - LEVEL 01 - AREA E	BC	P-405	PLUMBING ENLARGED VIEWS	BC	E-442	AUDIO VISUAL ENLARGED PLANS Copy 1
VI-111.F	MECHANICAL HVAC PLAN - LEVEL 01 - AREA F	BC	P-406	PLUMBING ENLARGED VIEWS	BC	E-451	AUDIO VISUAL DETAILS
				·			
W-112.A	MECHANICAL HVAC PLAN - LEVEL 02 - AREA A	BC	P-407	PLUMBING ENLARGED VIEWS	BC	E-471	AUDIO VISUAL RISER AND EQUIPMENT LIST
VI-112.B	MECHANICAL HVAC PLAN - LEVEL 02 - AREA B	BC	P-501	PLUMBING DETAILS	BC	E-472	AV RISER & EQUIPMENT LIST
VI-112.C	MECHANICAL HVAC PLAN - LEVEL 02 - AREA C	BC	P-502	PLUMBING DETAILS	BC	E-473	AV RISER & EQUIPMENT LIST
VI-112.E	MECHANICAL HVAC PLAN - ROOF - AREA E	BC	P-601	PLUMBING SCHEDULES	BC	E-474	AV RISER & EQUIPMENT LIST
M-112.F	MECHANICAL HVAC PLAN - LEVEL 02 - AREA F	BC	P-810	PLUMBING UNDERFLOOR PLAN - LEVEL 01 - BID ALTERNATE	BC	E-475	AV RISER & EQUIPMENT LIST
M-113.A	MECHANICAL HVAC PLAN - ROOF - AREA A	ВС	P-811	PLUMBING PLAN - LEVEL 01 - BID ALTERNATE	BC	E-476	AV RISER & EQUIPMENT LIST
M-113.B	MECHANICAL HVAC PLAN - ROOF - AREA B	ВС	P-812	PLUMBING PLAN - LEVEL 02 - BID ALTERNATE	BC	E-511	LEVEL 1 - AREA A - INTERCOM
M-113.C	MECHANICAL HVAC PLAN - ROOF - AREA C	BC	P-813	PLUMBING PLAN - ROOF - BID ALTERNATE	BC	E-512	LEVEL 1 - AREA B - INTERCOM
M-113.D	MECHANICAL HVAC PLAN - ROOF - AREA D	BC	PS-101	PLUMBING SITE PLAN	BC	E-513	LEVEL 1 - AREA C - INTERCOM
VI-113.F	MECHANICAL HVAC PLAN - ROOF - AREA F	BC				E-514	LEVEL 1 - AREA D - INTERCOM
VI-301	MECHANICAL ISOMETRIC VIEWS	BC	ELECTRICAL			E-515	LEVEL 1 - AREA E - INTERCOM
				CENEDAL NOTES AND SYMPOLS LISTS			
M-302	MECHANICAL ISOMETRIC VIEWS	BC	E-001	GENERAL NOTES AND SYMBOLS LISTS		E-516	LEVEL 1 - AREA F - INTERCOM
M-303	MECHANICAL ISOMETRIC VIEWS	BC	E-002	SYMBOL LISTS		E-521	LEVEL 2 - AREA A - INTERCOM
VI-304	MECHANICAL ISOMETRIC VIEWS	BC	E-101	SITE PLAN - NIBLEY - ELECTRICAL		E-522	LEVEL 2 - AREA B - INTERCOM
VI-401	MECHANICAL ENLARGED VIEWS	BC	E-102	SITE PLAN - HYDE PARK - ELECTRICAL		E-523	LEVEL 2 - AREA C - INTERCOM
VI-402	MECHANICAL ENLARGED VIEWS	ВС	E-103	SITE DETAILS		E-551	INTERCOM DETAILS
VI-403	MECHANICAL ENLARGED VIEWS	BC	E-111	LEVEL 1 - AREA A - LIGHTING		E-571	INTERCOM - RISER & EQUIP LIST.
VI-404	MECHANICAL ENLARGED VIEWS	ВС	E-112	LEVEL 1 - AREA B - LIGHTING		FIRE PROTEC	CTION:
VI-501	MECHANICAL DETAILS	BC	E-113	LEVEL 1 - AREA C - LIGHTING		F-001	FIRE PROTECTION TITLE SHEET
VI-502	MECHANICAL DETAILS	BC	E-114	LEVEL 1 - AREA D - LIGHTING		F-101	FIRE PROTECTION PLAN - LEVEL 01 - OVERALL
M-502	MECHANICAL DETAILS	BC	E-115	LEVEL 1 - AREA E - LIGHTING		F-102	FIRE PROTECTION PLAN - LEVEL 02 - OVERALL
W-504	MECHANICAL DETAILS	BC	E-116	LEVEL 1 - AREA F - LIGHTING		F-501	FIRE PROTECTION DETAILS
VI-505	MECHANICAL DETAILS	BC	E-121	LEVEL 2 - AREA A - LIGHTING			
VI-506	MECHANICAL DETAILS	BC	E-122	LEVEL 2 - AREA B - LIGHTING		KITCHEN	
M-601	MECHANICAL SCHEDULES	BC	E-123	LEVEL 2 - AREA C - LIGHTING		K-101	KITCHEN EQUIPMENT LAYOUT
M-602	MECHANICAL SCHEDULES	BC	E-131	LEVEL 1 - ENLARGED LIGHTING PLANS		K-102	KITCHEN EQUIPMENT SCHEDULES
M-603	MECHANICAL SCHEDULES	BC	E-141	THEATRICAL LIGHTING RISER		K-201	KITCHEN ELECTRICAL ROUGH-INS
VI-604	MECHANICAL SCHEDULES	BC	E-151	LIGHTING DETAILS		K-301	KITCHEN PLUMBING ROUGH-INS
VI-701	MECHANICAL SCHEMATICS	BC	E-152	LIGHTING NOTES		K-401	KITCHEN ELEVATIONS
VI-702	MECHANICAL SCHEMATICS	BC	E-161	LIGHT FIXTURE SCHEDULE		K-402	KITCHEN ELEVATIONS
VI-702 VI-703	MECHANICAL SCHEMATICS	BC		LEVEL 1 - AREA A - POWER			SECTIONS AND DETAILS
			E-211			K-403	
W-704	MECHANICAL SCHEMATICS	BC	E-212	LEVEL 1 - AREA B - POWER		K-501	KITCHEN EXHAUST HOOD DETAILS
VI-705	MECHANICAL SCHEMATICS	BC	E-213	LEVEL 1 - AREA C - POWER		K-502	KITCHEN EXHAUST HOOD DETAILS
VI-706	MECHANICAL SCHEMATICS	BC	E-214	LEVEL 1 - AREA D - POWER		K-503	KITCHEN EXHAUST HOOD DETAILS
VI-707	MECHANICAL SCHEMATICS	BC	E-215	LEVEL 1 - AREA E - POWER		K-504	KITCHEN EXHAUST HOOD DETAILS
MP-101	MECHANICAL PIPING PLAN - LEVEL 01 - OVERALL	BC	E-216	LEVEL 1 - AREA F - POWER		K-505	KITCHEN UDS WALL DETAILS
MP-102	MECHANICAL PIPING PLAN - LEVEL 02 - OVERALL	BC	E-221	LEVEL 2 - AREA A - POWER		K-601	BUILDING CONDITIONS - SLAB DEPRESSION PLAN
MP-103	MECHANICAL PIPING PLAN - ROOF - OVERALL	BC	E-222	LEVEL 2 - AREA B - POWER			
MP-111.A	MECHANICAL PIPING PLAN - LEVEL 01 - AREA A	BC	E-223	LEVEL 2 - AREA C - POWER			
MP-111.B	MECHANICAL PIPING PLAN - LEVEL 01 - AREA B	ВС	E-224	LEVEL 2 - ROOF PLAN - OVERALL - POWER			
MP-111.C	MECHANICAL PIPING PLAN - LEVEL 01 - AREA C	BC	E-231	LEVEL 1 - ENLARGED POWER PLANS			
MP-111.D	MECHANICAL PIPING PLAN - LEVEL 01 - AREA D	BC	E-232	ENLARGED KITCHEN PLAN			
MP-111.E	MECHANICAL PIPING PLAN - LEVEL 01 - AREA E	BC	E-241	POWER DETAILS			
MP-111.F	MECHANICAL PIPING PLAN - LEVEL 01 - AREA F	BC	E-242	DATA RISERS			
MP-112.A	MECHANICAL PIPING PLAN - LEVEL 02 - AREA A	BC	E-251	ONE-LINE DIAGRAM			
MP-112.B	MECHANICAL PIPING PLAN - LEVEL 02 - AREA B	BC	E-252	ONE-LINE DIAGRAM - POWER			
MP-112.C	MECHANICAL PIPING PLAN - LEVEL 02 - AREA C	BC	E-261	PANEL SCHEDULES			
MP-112.E	MECHANICAL PIPING PLAN - ROOF - AREA E	BC	E-262	PANEL SCHEDULES			
MP-112.F	MECHANICAL PIPING PLAN - LEVEL 02 - AREA F	ВС	E-263	PANEL SCHEDULES			
MP-113.A	MECHANICAL PIPING PLAN - ROOF - AREA A	BC	E-264	PANEL SCHEDULES			
WP-113.B	MECHANICAL PIPING PLAN - ROOF - AREA B	BC	E-265	PANEL SCHEDULES			
MP-113.C	MECHANICAL PIPING PLAN - ROOF - AREA C	BC	E-266	PANEL SCHEDULES PANEL SCHEDULES			
				PANEL SCHEDULES PANEL SCHEDULES			
MP-113.D	MECHANICAL PIPING PLAN - ROOF - AREA D	BC	E-267				
MP-113.F	MECHANICAL PIPING PLAN - ROOF - AREA F	BC	E-311	LEVEL 1 - AREA A - SYSTEMS			
MP-401	MECHANICAL PIPING ENLARGED VIEWS	BC	E-312	LEVEL 1 - AREA B - SYSTEMS			
MP-402	MECHANICAL PIPING ENLARGED VIEWS	ВС	E-313	LEVEL 1 - AREA C - SYSTEMS			
MP-403	MECHANICAL PIPING ENLARGED VIEWS	BC	E-314	LEVEL 1 - AREA D - SYSTEMS			
MP-404	MECHANICAL PIPING ENLARGED VIEWS	ВС	E-315	LEVEL 1 - AREA E - SYSTEMS	-		
MP-701	MECHANICAL PIPING SCHEMATICS	BC	E-316	LEVEL 1 - AREA F - SYSTEMS			
MP-702	MECHANICAL PIPING SCHEMATICS	BC	E-321	LEVEL 1 - AILEA I - OTSTEMS LEVEL 2 - AREA A - SYSTEMS			
vii -1 UZ	WILDHAMIDAL I II IIVU JUHLIMATIUJ	DU					
			E-322	LEVEL 2 - AREA B - SYSTEMS			
PLUMBING:			E-323	LEVEL 2 - AREA C - SYSTEMS			
P-000	PLUMBING TITLE SHEET	BC	E-341	CABLE TRAY DETAILS			
P-101	PLUMBING PLAN - LEVEL 01 - OVERALL	BC	E-342	SECURITY DETAIL & RISERS			
P-102	PLUMBING PLAN - LEVEL 02 - OVERALL	ВС	E-343	ACCESS CONTROL DOOR ROUGH-IN DETAILS			
	PLUMBING PLAN - ROOF - OVERALL	BC	E-351	FIRE ALARM RISER DIAGRAM			
P-103	PLUMBING PLAN - LEVEL 01 - AREA A	BC	E-352	FIRE RISER DIAGRAM HORN ALARM SYSTEM			
P-103 P-111 A		DU	L 00L	THE HOLD BUILDING HOUR ALAHIM OTOTLIN			
P-111.A		DC	F_//11	ΙΕΛΕΙ 1 - ΔΒΕΔ Δ - ΔΙΙΝΙΟ ΜΙΙΟΛΙ			
	PLUMBING PLAN - LEVEL 01 - AREA B PLUMBING PLAN - LEVEL 01 - AREA C	BC BC	E-411 E-412	LEVEL 1 - AREA A - AUDIO VIUSAL LEVEL 1 - AREA B - AUDIO VIUSAL			

P-111.D PLUMBING PLAN - LEVEL 01 - AREA D

E-413 LEVEL 1 - AREA C - AUDIO VIUSAL

ABBREVIATIONS

EQUAL EQUIPMENT

ELECTRIC WATER COOLER

EQUIP

<u>DESCRIPTION</u> **DESCRIPTION DESCRIPTION** EXIST PART BD PARTICLE BOARD ANCHOR BOLT EXISTING PART'N P-LAM PLYWD PREFAB ACRYLONITRILE-BUTADIENE PARTITION EXPANSION PLASTIC LAMINATE PLATE EXTERIOR ACOUSTIC, ACOUSTICAL FLOOR DRAIN PLYW00D ACCESSIBLE STATION FOUNDATION PREFABRICATED FIRE EXTINGUISHER CABINET PROJECTION ADDENDUM ADJUSTABLE FINISH PRESERVATIVE TREATED FL00R POLYVINYL CHLORIDE ABOVE FINISH FLOOR ALTERNATE FOOTING QUARRY TILE ALUMINUM GAUGE ROUND ARCHITECT SUPPLEMENTAL GALV GALVANIZED RADIUS INSTRUCTION GALVANIZED IRON **ROOF DRAIN** GYP BD REFRIGERATOR ASPHALT GYPSUM BOARD HDWD REINF BASKETBALL HARDW00D REINFORCE REVISION BOARD HOLLOW METAL BUILDING HORIZ HORIZONTAL REQUEST FOR INFORMATION BLOCKING HEIGHT ROUGH OPENING SCHED SHT INSIDE DIAMETER SCHEDULE BENCH MARK BOTTOM OF INSULATION SHEET BEARING INTERIOR SIMILAR BASEMENT SPECIFICATION KNOCK DOWN SQUARE BUILT UP ROOF KNOCK OUT STAINLESS STEEL CHANNEL CHALKBOARD STD STANDARD ANGLE STL STOR STRUCT SUSP CENTER LINE LONG LEG VERTICAL STEEL STORAGE CEILING MAXIMUM CONCRETE MASONRY UNIT MARKER BOARD STRUCTURAL CLEAN OUT MECHANICAL SUSPENDED, SUSPENSION MANUFACTURER SYSTEM COLUMN CONCRETE MANHOLE TOP AND BOTTOM CONNECTION MINIMUM TACKBOARD CONTINUOUS MISCELLANEOUS TEMPORARY CONTR CONTRACTOR MASONRY OPENING TELEPHONE CONTROL JOINT THRESHOLD MOUNT CERAMIC TILE METAL TUBE STEEL PENNY NEW TOP OF NOT IN CONTRACT TOILET DIMENSION TELEVISION TYPICAL VERTICAL DOWNSPOU' DRAWING EXISTING NOT TO SCALE ON CENTER OUTSIDE DIAMETER TYP VERT U.N.O. EACH OVERHEAD UNLESS NOTED OTHERWISE EXTERIOR INSULATION OWNER FURNISHED / WIDE FLANGE FINISH SYSTEM CONTRACTOR INSTALLED WITH ELECTRICAL ELEVATION WATER CLOSET OWNER FURNISHED / ELEV OWNER INSTALLED WOOD

OPPOSITE

OPEN TO STRUCTURE

OPNG OPP O.T.S.

S-301

AS-103

AS-104

ARCHITECTURAL SITE:

AS-501 SITE DETAILS

ELEVATIONS

SCHEMATIC REFERENCE

AS-100 OVERALL ARCHITECTURAL SITE PLAN

AS-101 ARCHITECTURAL SITE PLAN AREA A

AS-102 ARCHITECTURAL SITE PLAN AREA B

ARCHITECTURAL SITE PLAN AREA C

ARCHITECTURAL SITE PLAN AREA D

WM W/O WWF

WATER METER WITHOUT

WELDED WIRE FABRIC

SYMBOLS LEGEND

A-349

A-350

A-351

A-352

A-353

A-354

A-355

A-356

A-411

A-412

SECTIONS - WALL

SECTIONS - WALL

SECTIONS - WALL

WALL PROFILES

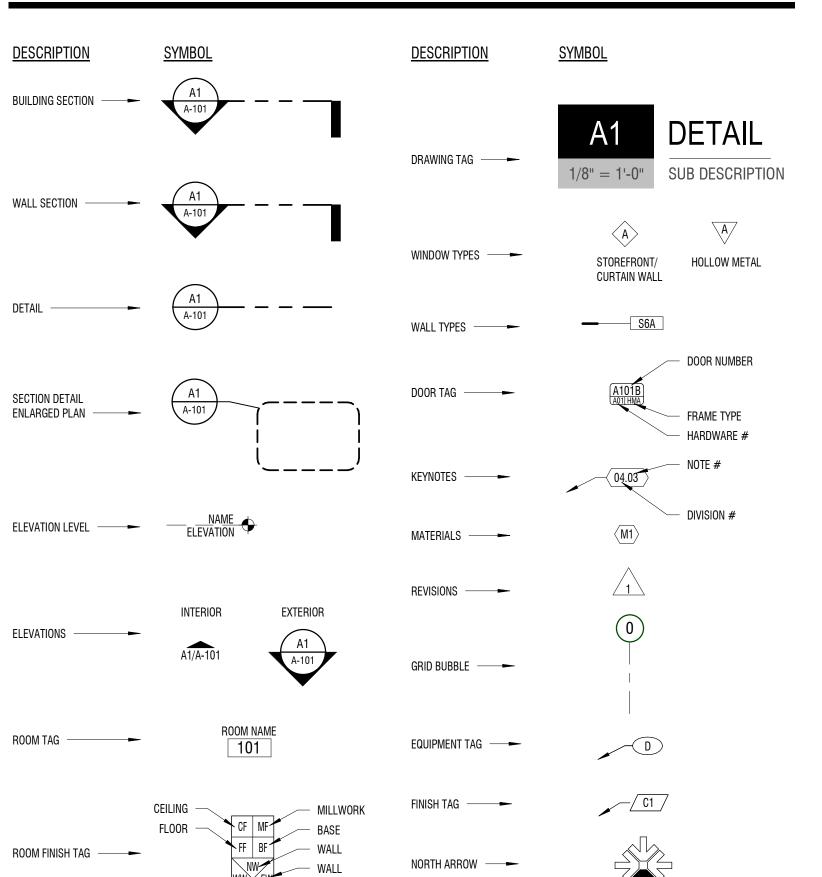
WALL PROFILES

WALL PROFILES WALL PROFILES

WALL PROFILES

ENLARGED VIEWS

ENLARGED VIEWS



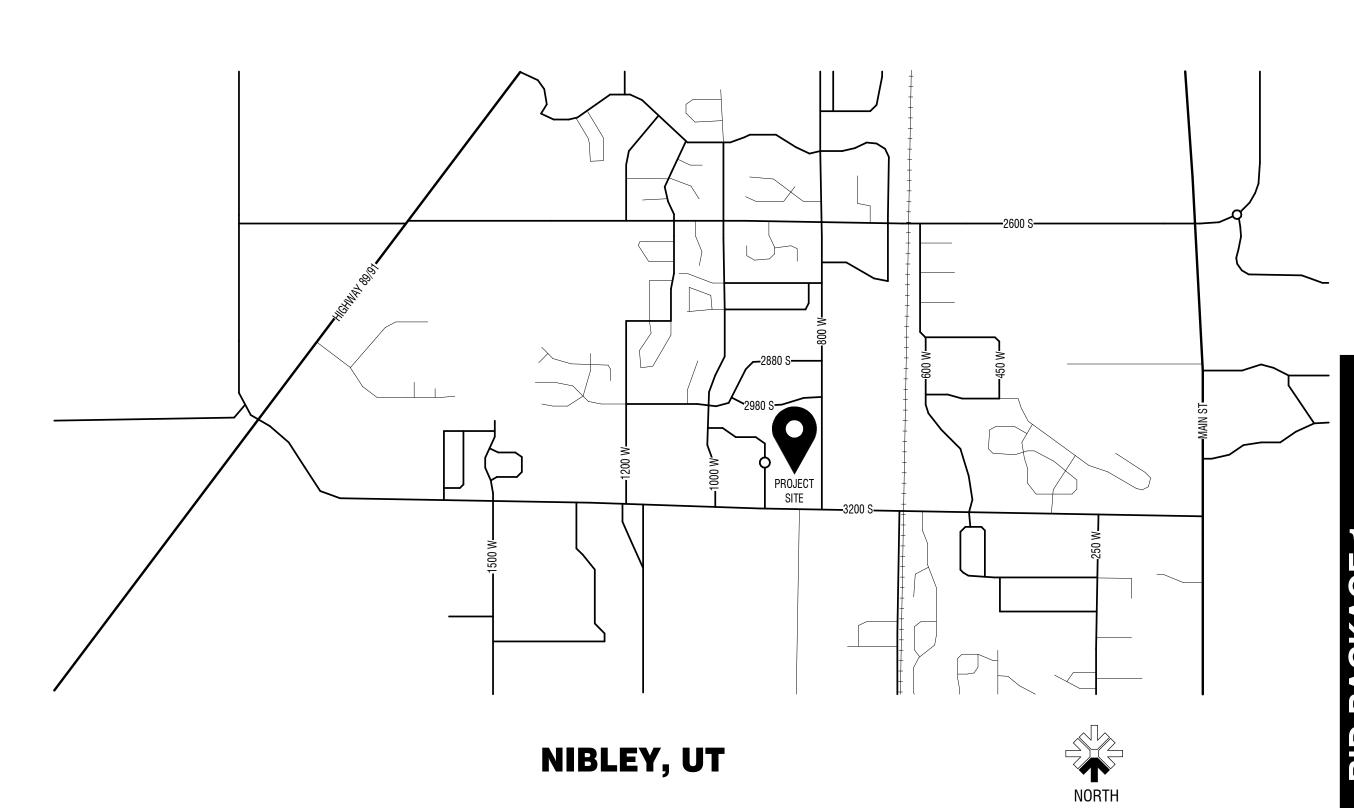
MATERIALS LEGEND

<u>MATERIAL</u> <u>SYMBOL</u> ASPHALT PAVING COMPACTED GRANULAR FILL 4 4 4 4 CONCRETE CONCRETE MASONRY UNITS CONCRETE MASONRY UNITS STEEL CONTINUOUS WOOD WOOD BLOCKING PLYWOOD / OSB PARTICLE BOARD INSULATION RIGID INSULATION GYPSUM BOARD GLU-LAMINATE BEAM GLASS FINISH WOOD ALUMINUM

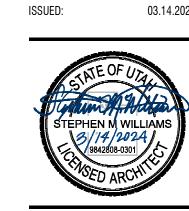
WOOD STUD WALL

NORTH DIRECTION IS INDICATED BY THE FILLED ARROW

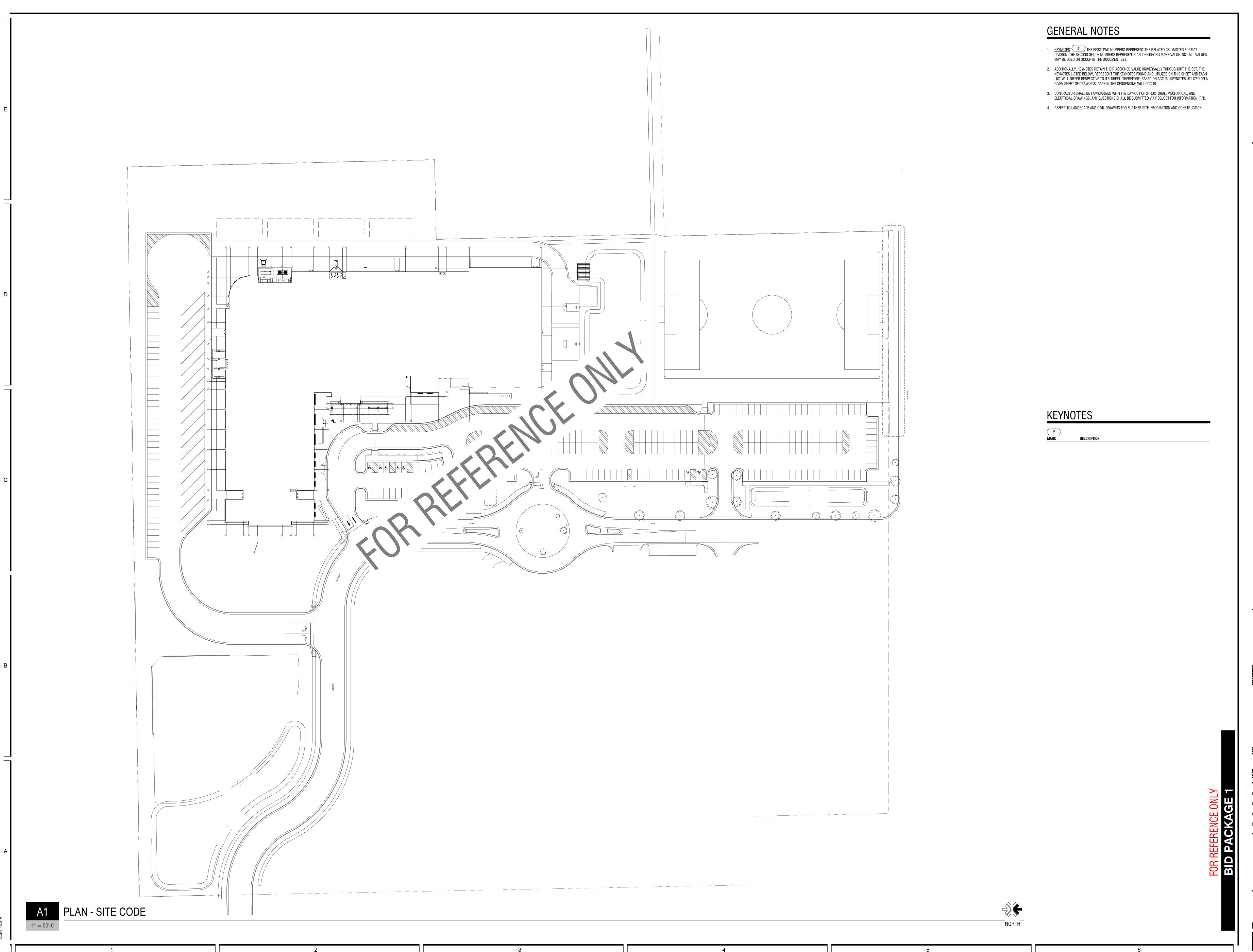
VICINITY MAP



BID PACKAG



architects



architects
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CIVIL GENERAL NOTES

BID PACKAGE NOTES:

BID PACKAGE #1 TO INCLUDE:

1. ROUGH GRADING, ON-SITE.

2. ROUGH GRADING, 3200 SOUTH/920 WEST STREET.

3. INSTALLATION OF WATER AND SEWER UTILITIES.

4. STOCKPILING OF TOP SOIL.

BID PACKAGE #2 TO INCLUDE:

1. INSTALLATION OF STORMDRAIN, GAS, POWER, AND COMMUNICATION UTILITIES.

2. FINISH GRADING, ROADWAY AND ON-SITE, INCLUDING: -ASPHALT PAVING. -CONCRETE CURBING AND FLATWORK.

3. WIDEN EXISTING ROADWAY (3200 SOUTH STREET).

-TOP SOIL IN LANDSCAPE AREAS.

GENERAL SITE NOTES

1. NO WORK IS TO BEGIN UNTIL NECESSARY PERMITS HAVE BEEN OBTAINED.

2. REQUIREMENTS SHOWN ON SITE PLAN SHALL GOVERN. GENERAL CONTRACTOR TO POINT OUT ANY DISCREPANCIES PRIOR TO CONSTRUCTION.

3. ENTIRE INSTALLATION SHALL MEET ALL APPLICABLE CODES.

4. VERIFY ALL CONDITIONS AND DIMENSIONS ON SITE.

5. GENERAL CONTRACTOR RESPONSIBLE TO OBTAIN AND PAY FOR ALL NECESSARY PERMITS.

6. GENERAL CONTRACTOR TO PROVIDE ALL EQUIPMENT, PERSONNEL, AND CONSTRUCTION STAKING REQUIRED FOR FINAL CHECKOUT OF ALL FACILITIES BY OWNER'S REPRESENTATIVE.

7. GENERAL CONTRACTOR TO PERFORM GENERAL YARD AND BUILDING CLEAN-UP AT COMPLETION OF

8. ALL PUBLIC IMPROVEMENTS SHALL BE IN ACCORDANCE WITH THE NIBLEY CITY STANDARDS AND SPECIFICATIONS FOR THE DESIGN AND CONSTRUCTION OF PUBLIC IMPROVEMENTS, LATEST REVISION IHEREOF. II IS RECOMMENDED IHAI IHE CONTRACTOR OBTAIN A COPY OF THIS MANUAL FROM

9. ALL ASPHALT CUTS FOR UTILITIES AND PAVEMENT WITHIN PUBLIC RIGHTS OF WAY SHALL BE IN ACCORDANCE WITH THE NIBLEY CITY STANDARDS AND SPECIFICATIONS. "WORK IN R/W" PERMITS ARE REQUIRED.

10. GENERAL CONTRACTOR SHALL PROVIDE TRAFFIC CONTROL PLAN WHICH SHALL BE SUBMITTED TO AND APPROVED BY THE CITY OF NORTH LOGAN PRIOR TO ANY WORK IN THE PUBLIC R/W. CONTRACTOR IS RESPONSIBLE FOR SAFETY TO THE PUBLIC BY MINIMIZING THE INTERRUPTION OF THE USE OF ROADS AND PROVIDING SIGNS, FLARES, BARRICADES, ETC. AS NECESSARY. TRAFFIC CONTROL TO BE COMPLIANT WITH CURRENT MUTCD. WORK SHALL COMPLY WITH "WORK IN R/W"

11. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO VERIFY THE ACTUAL LOCATION AND ELEVATION OF EXISTING UTILITIES WHICH MAY BE IN CONFLICT WITH THE PROPOSED CONSTRUCTION. IF A CONFLICT DOES EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER PRIOR TO CONSTRUCTION SO THAT ADJUSTMENTS CAN BE MADE.

12. IT WILL BE THE RESPONSIBILITY OF THE CONTRACTOR TO NOTIFY ALL UTILITIES WHEN CONSTRUCTION WORK BEGINS IN THE VICINITY OF ANY UTILITY LINES AND TO ARRANGE FOR A REPRESENTATIVE OF THE UTILITY TO BE PRESENT IF THE CONTRACTOR'S OPERATIONS ARE IN CLOSE PROXIMITY TO ANY LINES IN THEIR EXISTING OR RELOCATED POSITION WHICH COULD CREATE A HAZARDOUS CONDITION.

13. WHERE THERE IS A CONFLICT BETWEEN THESE PLANS AND THE SPECIFICATIONS, OR ANY APPLICABLE STANDARDS, THE HIGHER QUALITY STANDARD SHALL APPLY.

14. THE CONTRACTOR SHALL BE RESPONSIBLE FOR KEEPING ROADWAYS FREE AND CLEAR OF ALL CONSTRUCTION DEBRIS AND DIRT TRACKED FROM THE SITE.

15. DIMENSIONS FOR LAYOUT AND CONSTRUCTION ARE NOT TO BE SCALED FROM ANY DRAWING. IF PERTINENT DIMENSIONS ARE NOT SHOWN, CONTACT THE CONSULTING ENGINEER FOR CLARIFICATION, AND ANNOTATE THE DIMENSION ON THE AS-BUILT RECORD DRAWINGS.

16. OWNER/CONTRACTOR IS RESPONSIBLE TO OBTAIN A UPDES STORMWATER DISCHARGE PERMIT AND IS RESPONSIBLE FOR DEVELOPING AND IMPLEMENTING A STORMWATER POLLUTION PREVENTION PLAN (SWPPP) AS PER THE REQUIREMENTS OF THE UPDES STORMWATER CONSTRUCTION PERMIT (NOI PERMIT # _____)

UTILITY NOTES

1. ALL UTILITY LOCATIONS SHOWN ARE BASED ON MAPS PROVIDED BY THE APPROPRIATE UTILITY COMPANY AND FIELD SURFACE EVIDENCE AT THE TIME OF SURVEY AND IS TO BE CONSIDERED AN APPROXIMATE LOCATION ONLY. IT IS THE CONTRACTOR'S RESPONSIBILITY TO FIELD VERIFY THE ACTUAL LOCATION OF ALL UTILITIES. PUBLIC OR PRIVATE. WHETHER SHOWN ON THE PLANS OR NOT, PRIOR TO CONSTRUCTION. REPORT ANY DISCREPANCIES TO THE ENGINEER PRIOR TO CONSTRUCTION.

2. WHERE A PROPOSED UTILITY CROSSES AN EXISTING UTILITY, IT IS THE CONTRACTOR'S RESPONSIBILITY TO FIELD VERIFY THE HORIZONTAL AND VERTICAL LOCATION OF SUCH EXISTING UTILITY, EITHER THROUGH POTHOLING OR ALTERNATIVE METHOD. REPORT INFORMATION TO THE ENGINEER PRIOR TO CONSTRUCTION.

3. VERIFY ALL CONDITIONS AND DIMENSIONS ON SITE.

4. GENERAL CONTRACTOR TO COORDINATE ALL UTILITY WORK WITH THE APPROPRIATE UTILITY PROVIDER. GENERAL CONTRACTOR TO VERIFY AND FOLLOW ALL UTILITY PROVIDER REQUIREMENTS, PROCEDURES, STANDARDS AND SPECIFICATIONS.

5. GENERAL CONTRACTOR TO PROVIDE ALL EQUIPMENT, PERSONNEL, AND CONSTRUCTION STAKING REQUIRED FOR FINAL CHECKOUT OF ALL FACILITIES BY OWNER'S REPRESENTATIVE.

6. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO VERIFY THE ACTUAL LOCATION AND ELEVATION OF EXISTING UTILITIES WHICH MAY BE IN CONFLICT WITH THE PROPOSED CONSTRUCTION. IF A CONFLICT DOES EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER

PRIOR TO CONSTRUCTION SO THAT ADJUSTMENTS CAN BE MADE. 7. ALL WATER LINES TO HAVE A MINIMUM COVER OF 5-FEET.

8. ALL WATER MAINS SHALL BE C900 PVC AND AWWA STANDARD EQUIVALENT.

9. SEPARATE WATER AND SEWER MAIN LINES AND SERVICES HORIZONTALLY BY 10-FEET MINIMUM FROM OUTSIDE OF PIPE TO OUTSIDE OF PIPE.

<u>GRADING NOTES</u>

1. FINAL GRADES ARE SUBJECT TO MINOR CHANGES AS APPROVED BY OWNER. NO GRADE CHANGES IN EXCESS OF 0.1 FEET WITHOUT ENGINEERS APPROVAL.

2. ALL ROUGH GRADES TO BE ESTABLISHED AS FOLLOWS:

A. BUILDING PADS, ROADWAYS, PARKING LOTS, FIRE AND TRUCK ACCESS WITHIN 0.10 FT ON TOP OF PIT RUN

ROUGH GRADE FOR:

BUILDING PADS: 8" BELOW FG

ROADWAYS: 23" BELOW FG

 PARKING LOTS: 9" BELOW FG FIRE AND TRUCK ACCESS (HEAVY-DUTY CONCRETE): 16" BELOW FG

BUS PARKING: 10" BELOW FG

B. ATHLETIC PERFORMANCE AREAS- SOFTBALL AND SOCCER PLAY FIELDS WITHIN 0.1 FT OF SUBGRADE

ROUGH GRADE FOR:

 SOCCER PLAY FIELD: 8" BELOW FG SOFTBALL PLAY FIELD: 8" BELOW FG

C. SITE CONCRETE (FLATWORK) AND SIDEWALKS WITHIN 0.10 FT OF SUBGRADE

ROUGH GRADE FOR: SITE CONCRETE: 10" BELOW FG

D. LANDSCAPE AREAS, PLANTER AREAS, SLOPED AREAS WITHIN 0.2 FT OF SUBGRADE

ROUGH GRADE FOR:

LANDSCAPE AREAS: 8" BELOW FG

PLANTER AREAS: 18" BELOW FG

3. ALL FILL MATERIAL REQUIRED, SHALL BE CLEAN FILL SOIL APPROVED BY ENGINEER.

4. GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR MINIMIZING DEPOSITION OF ONSITE SEDIMENTS ONTO SURROUNDING PUBLIC STREETS DURING CONSTRUCTION. SEE EROSION CONTROL PLAN.

5. GENERAL CONTRACTOR TO PROVIDE BARRICADE PROTECTION WITH FLASHING LIGHTS AROUND ALL EXCAVATIONS

6. UNLESS OTHERWISE SPECIFIED, ALL CONSTRUCTION LAYOUT AND STAKING SHALL BE PERFORMED UNDER THE RESPONSIBLE CHARGE OF A LAND SURVEYOR LICENSED IN THE STATE OF UTAH AND BY PARTY CHIEF OR ENGINEERING TECHNICIAN EXPERIENCED IN CONSTRUCTION LAYOUT AND STAKING TECHNIQUES AS REQUIRED BY THE SPECIFIC TYPE OF WORK BEING PERFORMED.

7. THE CROSS SLOPE ON NEW SIDEWALKS SHALL NOT EXCEED 2.0%.

8. ALL AREAS NOT LOCATED WITHIN A RIGHT—OF—WAY OR UTILITY EASEMENT ARE TO REMAIN UNDISTURBED TO THE MAXIMUM EXTENT POSSIBLE. THESE AREAS SHALL BE AVOIDED AND MAINTAINED BY THE CONTRACTOR IN THEIR PRE-CONSTRUCTION STATE.

9. ACCESS AND HAULAGE ROADS SHALL BE MAINTAINED IN A DUST-FREE CONDITION BY SURFACING OR OTHER TREATMENT AS APPROVED BY THE NORTH LOGAN CITY ENGINEER AND FUGITIVE DUST SHALL BE CONTROLLED IN ALL OTHER OPERATIONAL AREAS OF THE EXCAVATION SITE. USE WATER TRUCKS OR OTHER MEANS TO CONTROL DUST AS

SEWER LAYOUT GENERAL NOTES:

1. SEE PLAN AND PROFILE SHEETS FOR MORE DETAILED SEWER INFORMATION INCLUDING RIM &

2. ALL SEWER MANHOLES SHALL BE INSTALLED PER NIBLEY CITY STANDARDS UNLESS OTHERWISE INDICATED ON DRAWINGS.

2. ALL EXISTING UTILITY LOCATIONS SHOWN ARE BASED UPON MAPS PROVIDED BY THE APPROPRIATE UTILITY COMPANY AND FIELD SURFACE EVIDENCE AT THE TIME OF SURVEY. LOCATIONS ARE TO BE CONSIDERED APPROXIMATE AND ARE PROVIDED FOR REFERENCE AND COORDINATION ONLY. IT IS THE CONTRACTOR'S RESPONSIBILITY TO FIELD VERIFY THE ACTUAL LOCATION OF ALL UTILITIES, PUBLIC OR PRIVATE, WHETHER SHOWN ON THESE PLANS OR NOT, PRIOR TO CONSTRUCTION. REPORT ANY DISCREPANCIES TO THE CIVIL ENGINEER IMMEDIATELY AND PRIOR TO ANY CONSTRUCTION ACTIVITY.

3. WHERE A PROPOSED UTILITY CROSSES AN EXISTING UTILITY, IT IS THE CONTRACTOR'S RESPONSIBILITY TO FIELD VERIFY THE HORIZONTAL AND VERTICAL LOCATION OF THE EXISTING UTILITY, EITHER THROUGH POT—HOLING OR ANOTHER ALTERNATIVE METHOD. CONTRACTOR TO REPORT FINDINGS TO THE CIVIL ENGINEER PRIOR TO CONSTRUCTION.

4. THE GENERAL CONTRACTOR IS TO ENSURE THE ENTIRE PROJECT INSTALLATION MEETS ALL APPLICABLE FEDERAL, STATE AND LOCAL CODES.

5. CONTRACTOR TO FIELD VERIFY ALL CONDITIONS AND DIMENSIONS ON-SITE.

6. THE GENERAL CONTRACTOR SHALL COORDINATE ALL UTILITY WORK WITH THE APPROPRIATE UTILITY PROVIDER. THE GENERAL CONTRACTOR SHALL VERIFY AND FOLLOW ALL UTILITY PROVIDER REQUIREMENTS, PROCEDURES, STANDARDS AND SPECIFICATIONS.

7. SEPARATE WATER AND SEWER MAIN LINES AND SERVICES HORIZONTALLY BY A MINIMUM OF 10-FEET AS MEASURED FROM OUTSIDE OF PIPE TO OUTSIDE OF PIPE.

GENERAL WATER NOTES

1. ALL UTILITY LOCATIONS SHOWN ARE BASED ON MAPS PROVIDED BY THE APPROPRIATE UTILITY COMPANY AND FIELD SURFACE EVIDENCE AT THE TIME OF SURVEY AND IS TO BE CONSIDERED AN APPROXIMATE LOCATION ONLY. IT IS THE CONTRACTOR'S RESPONSIBILITY TO FIELD VERIFY THE ACTUAL LOCATION OF ALL UTILITIES, PUBLIC OR PRIVATE, WHETHER SHOWN ON THE PLANS OR NOT, PRIOR TO CONSTRUCTION. REPORT ANY DISCREPANCIES TO THE ENGINEER PRIOR TO CONSTRUCTION.

2. WHERE A PROPOSED UTILITY CROSSES AN EXISTING UTILITY, IT IS THE CONTRACTOR'S RESPONSIBILITY TO FIELD VERIFY THE HORIZONTAL AND VERTICAL LOCATION OF SUCH EXISTING UTILITY, EITHER THROUGH POTHOLING OR ALTERNATIVE METHOD. REPORT INFORMATION TO THE ENGINEER PRIOR TO CONSTRUCTION.

3. VERIFY ALL CONDITIONS AND DIMENSIONS ON SITE.

4. GENERAL CONTRACTOR TO COORDINATE ALL UTILITY WORK WITH THE APPROPRIATE UTILITY PROVIDER. GENERAL CONTRACTOR TO VERIFY AND FOLLOW ALL UTILITY PROVIDER REQUIREMENTS, PROCEDURES, STANDARDS AND SPECIFICATIONS.

5. GENERAL CONTRACTOR TO PROVIDE ALL EQUIPMENT, PERSONNEL, AND CONSTRUCTION STAKING REQUIRED FOR FINAL CHECKOUT OF ALL FACILITIES BY OWNER'S REPRESENTATIVE.

6. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO VERIFY THE ACTUAL LOCATION AND ELEVATION OF EXISTING UTILITIES WHICH MAY BE IN CONFLICT WITH THE PROPOSED CONSTRUCTION. IF A CONFLICT DOES EXIST. THE CONTRACTOR SHALL NOTIFY THE ENGINEER PRIOR TO CONSTRUCTION SO THAT ADJUSTMENTS CAN BE MADE.

7. ALL WATER LINES TO HAVE A MINIMUM COVER OF 5-FEET.

8. ALL WATER MAINS SHALL BE C900 PVC AND AWWA STANDARD EQUIVALENT.

9. SEPARATE WATER AND SEWER MAIN LINES AND SERVICES HORIZONTALLY BY 10-FEET MINIMUM FROM OUTSIDE OF PIPE TO OUTSIDE OF PIPE.

10. ALL CONSTRUCTION TO MEET NIBLEY CITY STANDARDS.

TREE PROTECTION NOTES:

1. ALL EXISTING TREES TO REMAIN SHALL BE PROPERLY PROTECTED DURING CONSTRUCTION. PLACE FOUR-FOOT TALL CONSTRUCTION FENCE AROUND THE CRITICAL ROOT ZONE OF THE TREE. REMOVE FENCE ONLY AS NECESSARY FOR DAILY CONSTRUCTION, FENCE SHALL REMAIN IN PLACE DURING CONSTRUCTION TO PREVENT UNINTENDED IMPACTS.

2. THE TREE'S CRITICAL ROOT ZONE SHALL BE AT TREE'S CANOPY DRIP LINE OR A RADIUS OF TWELVE—TIMES THE DIAMETER OF THE TRUNK AT 4.5—FOOT DBH (DIAMETER AT BREAST HEIGHT) WHICHEVER IS LARGER.

3. IN THE CRITICAL ROOT ZONE:

A. DO NOT ALTER OR DISTURB EXISTING GRADE. B. DO NOT STORE ANY CONSTRUCTION MATERIALS, EQUIPMENT, SOIL OR

C. DO NOT DISPOSE OF ANY LIQUIDS E.G. CONCRETE, GAS, OIL, PAINT

D. DO NOT PERMIT VEHICLES, EQUIPMENT, OR FOOT TRAFFIC . AVOID TRENCHING. F. AVOID CONSTRUCTION ACTIVITY THAT WILL COMPACT THE SOIL.

4. IF CONSTRUCTION WORK DOES ENCROACH INTO THE CRITICAL ROOT ZONE THEN LIMIT ENCROACHMENT TO LESS THAN TWENTY-FIVE PERCENT OF THE TOTAL AREA, AND NO CLOSER TO THE TRUNK THAT ONE-HALF THE RADIUS OF THE CRITICAL ROOT ZONE. PROVIDE FIVE INCHES OF MULCH AND A PROTECTIVE MAT OVER THE IMPACTED ROOT AREA.

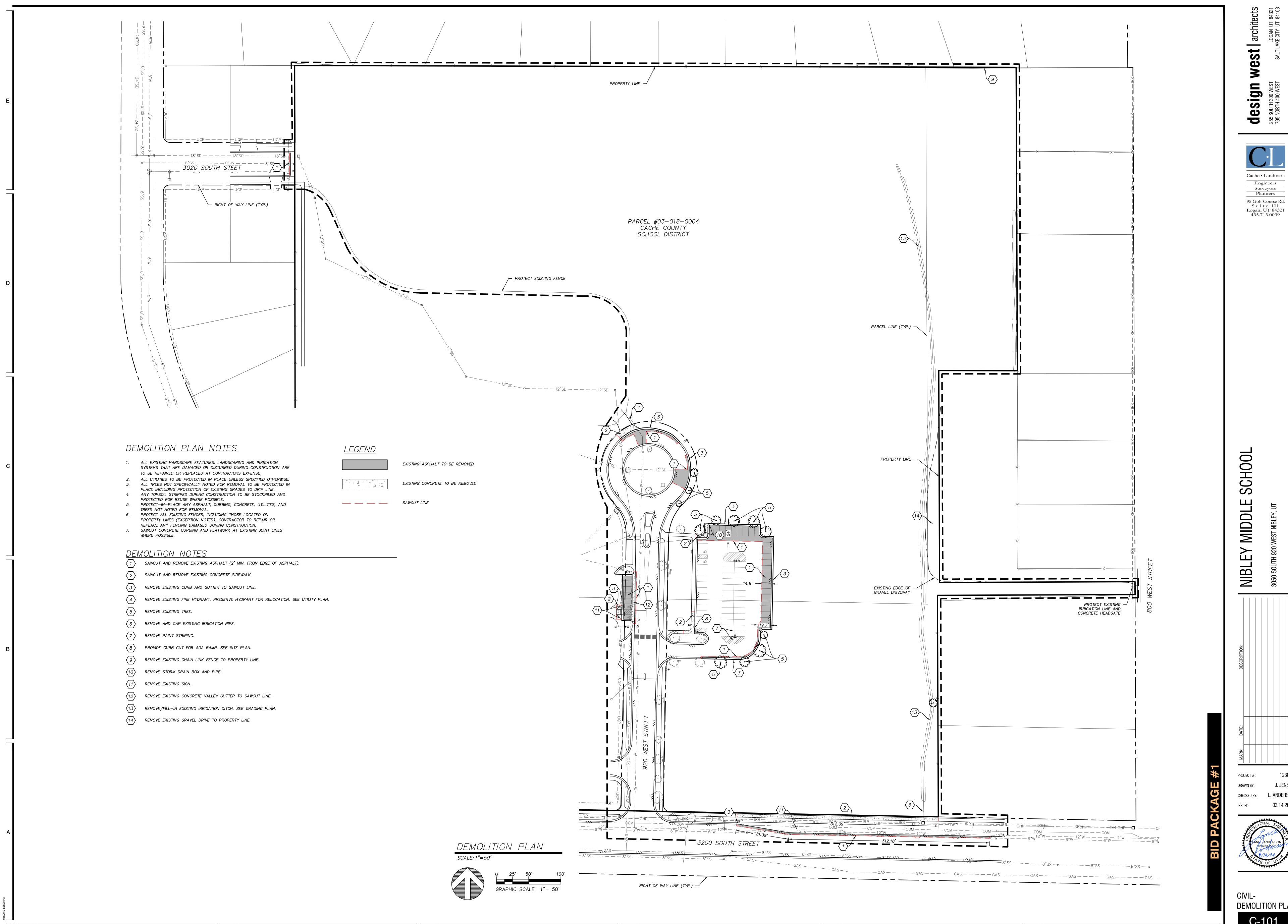
5. IF TRENCHING IS REQUIRED IN THE ROOT AREA, THEN BORE UNDER THE ROOTING AREA AT A MINIMUM DEPTH OF THIRTY-INCHES. IF A TRENCH FOR AN IRRIGATION HEAD IS NEEDED IN THE ROOT ZONE AREA, TRENCH IN A DIRECT LINE TOWARDS THE TRUNK TO MINIMIZE ROOT DAMAGE.

6. PROVIDE WATER TO THE TREE(S) DURING CONSTRUCTION TO MAINTAIN TREE

7. REPAIR OR REPLACE TREES AND VEGETATION INDICATED TO REMAIN THAT ARE DAMAGED BY CONSTRUCTION OPERATIONS, IN A MANNER APPROVED BY LANDSCAPE ARCHITECT. A. SUBMIT DETAILS OF PROPOSED REPAIRS TO DAMAGED TREES AND

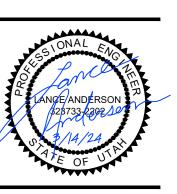
B. REPLACE TREES THAT CANNOT BE REPAIRED AND RESTORED TO FULL-GROWTH STATUS, AS DETERMINED BY A QUALIFIED ARBORIST.

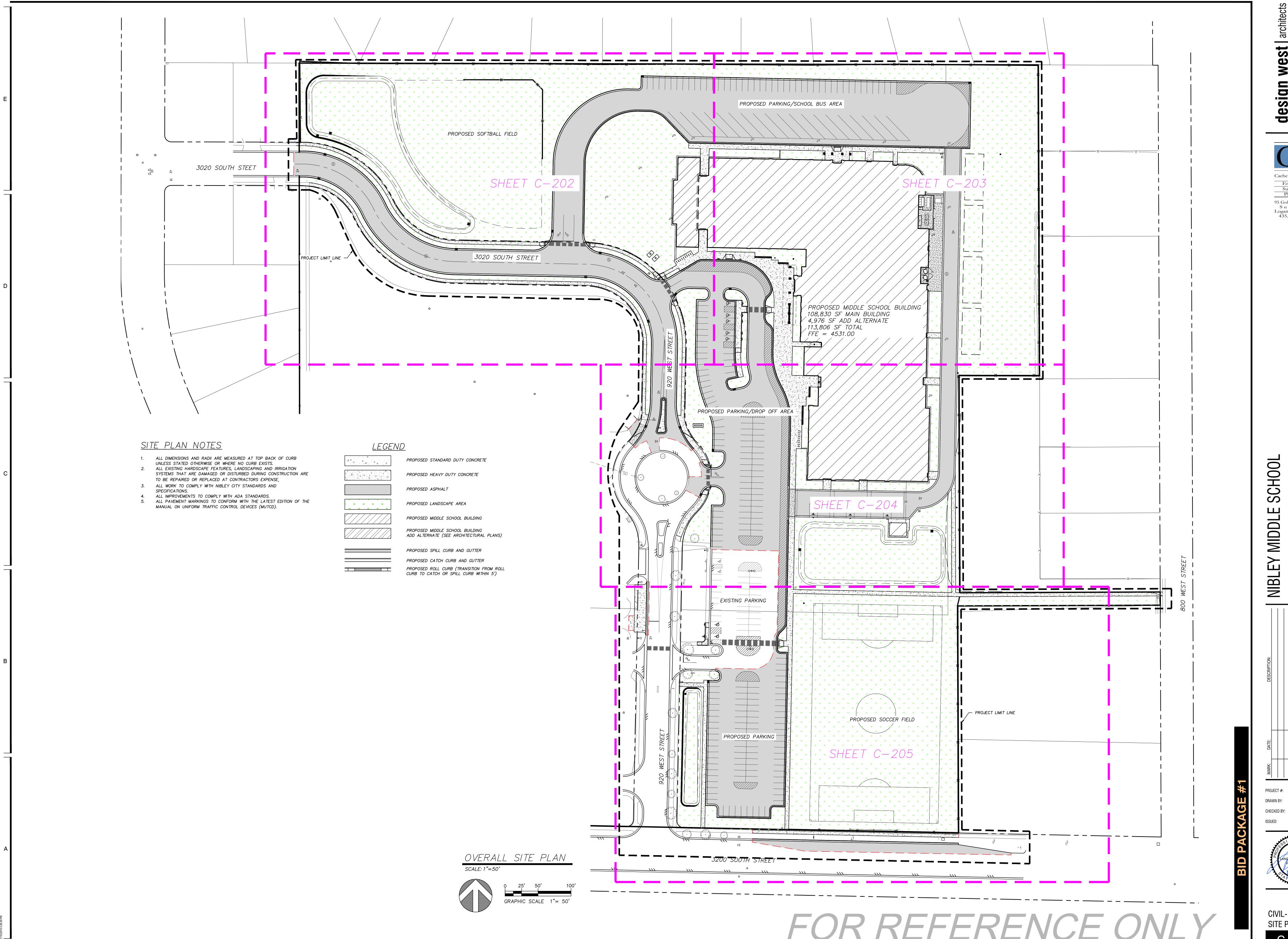
SEE ALSO ARCHITECTURAL PLANS SHEET AS-501/DETAIL 5.



design 255 SOUTH 300 WEST 795 NORTH 400 WEST







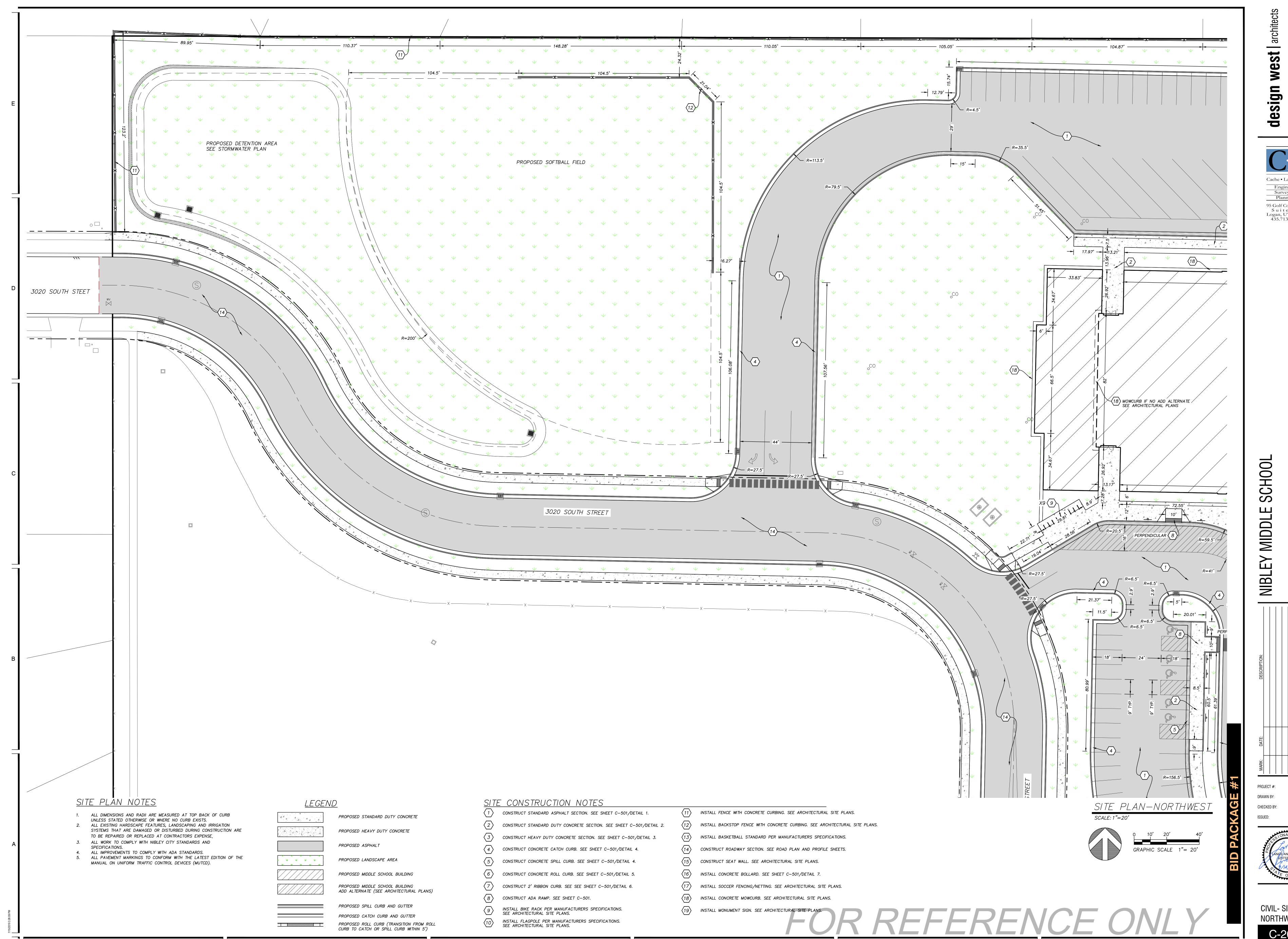
design255 SOUTH 300 WEST
795 NORTH 400 WEST



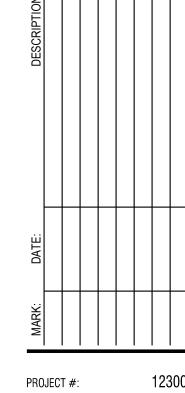


CIVIL- OVERALL SITE PLAN

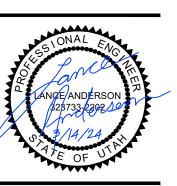
C-201





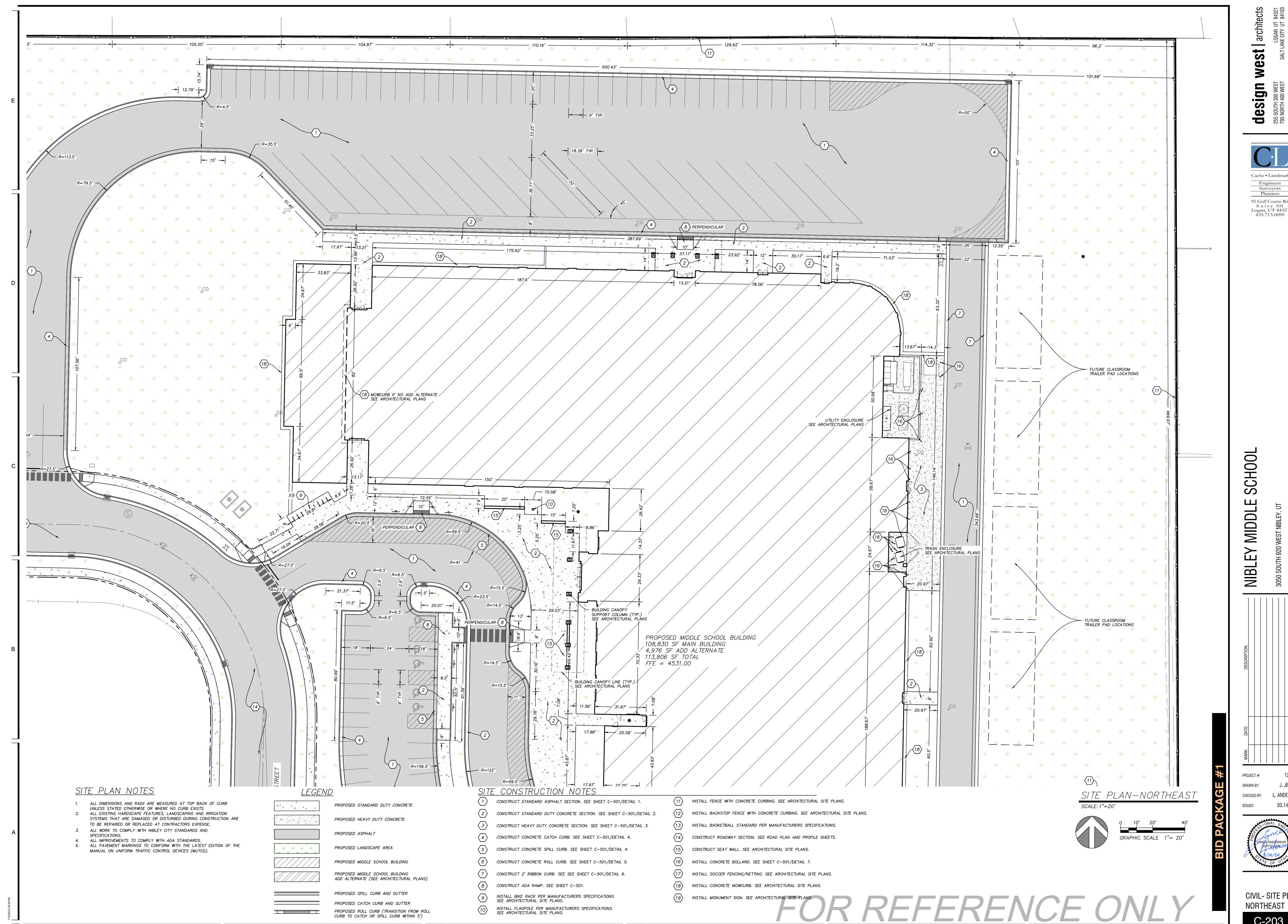


CHECKED BY: L. ANDERSON 03.14.2024



CIVIL- SITE PLAN NORTHWEST



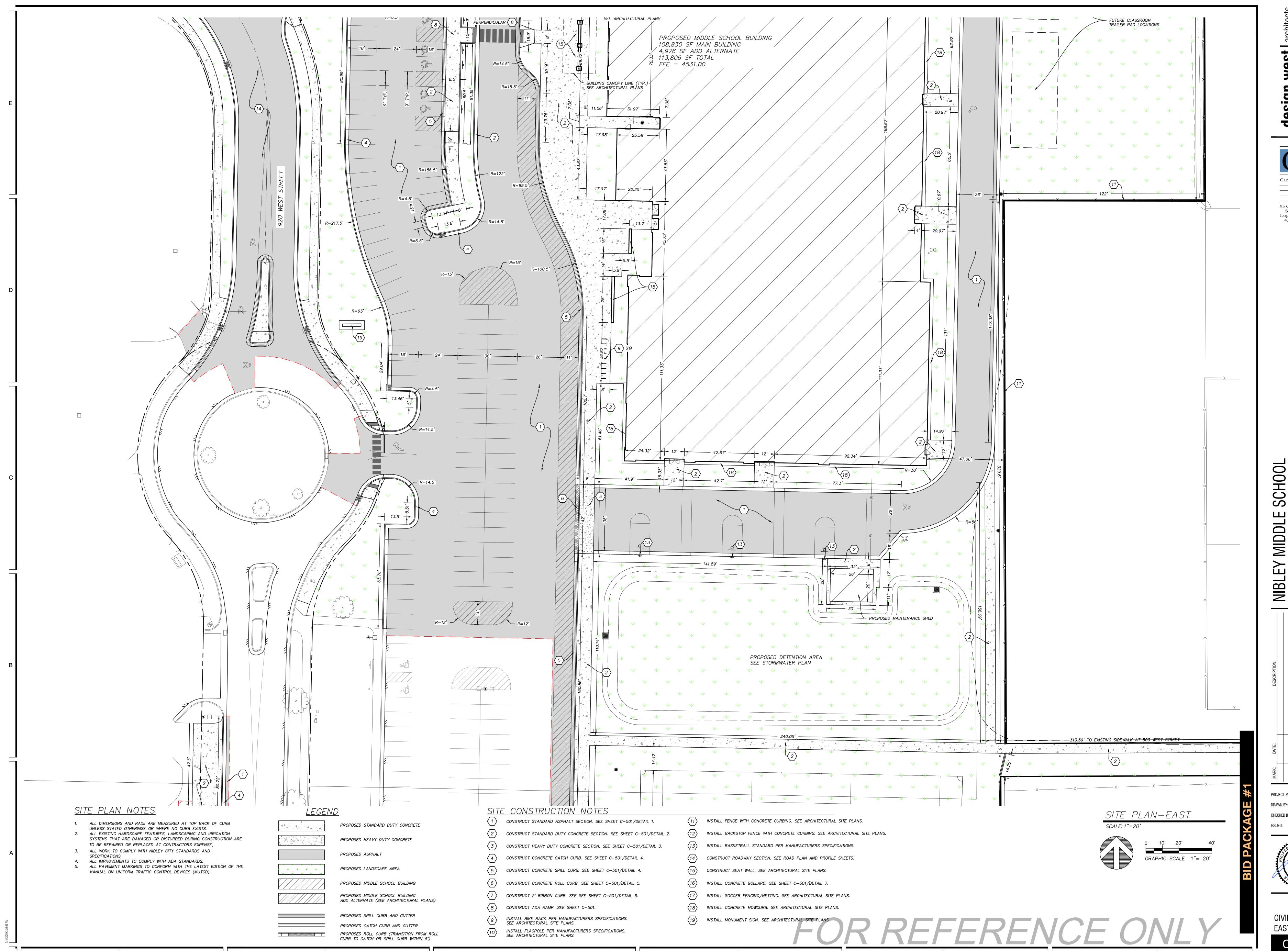




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CIVIL- SITE PLAN NORTHEAST

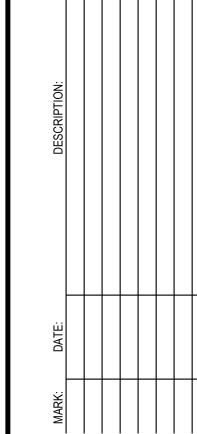


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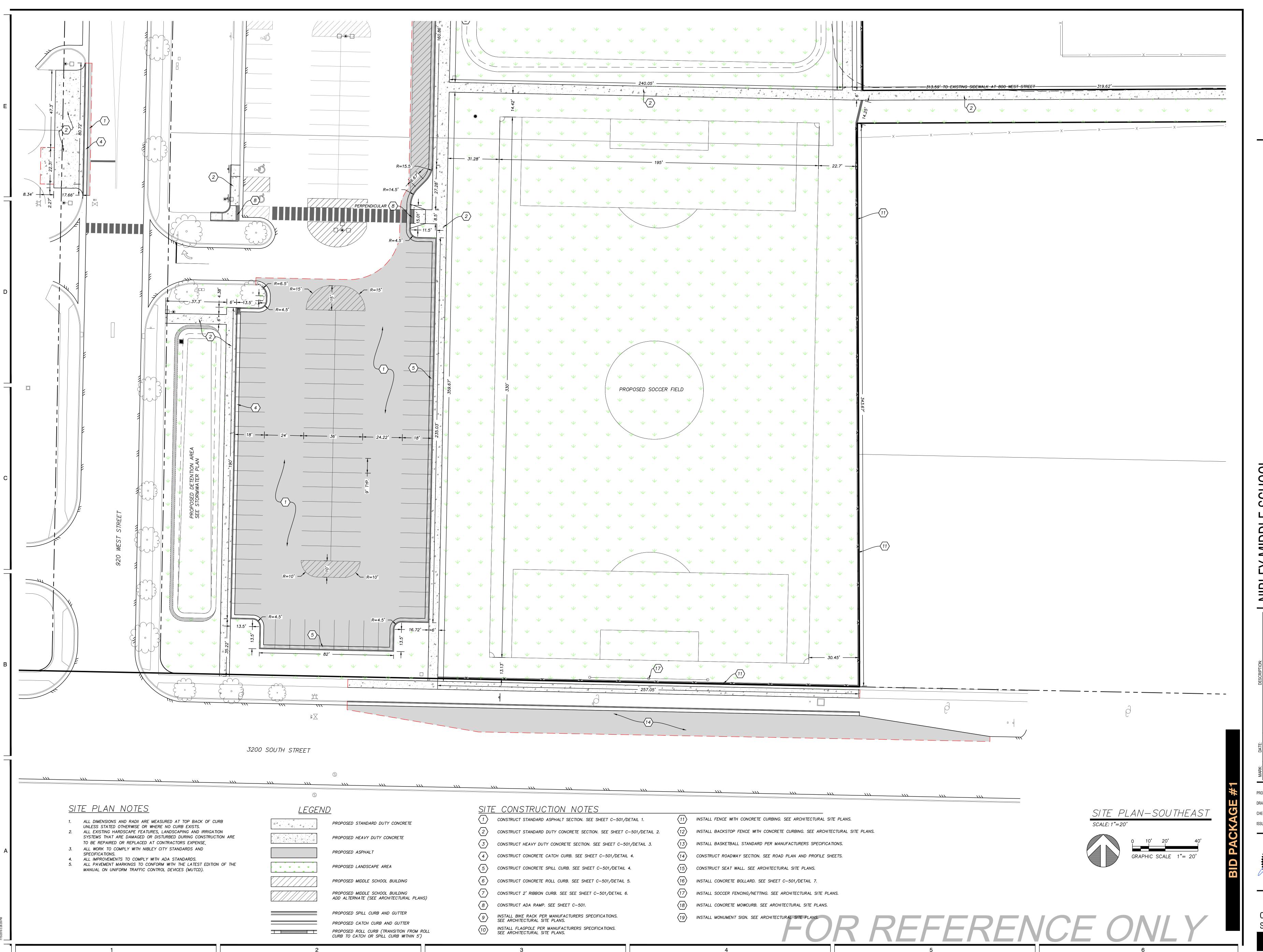
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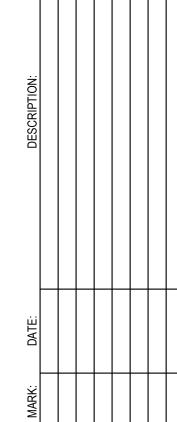


CIVIL- SITE PLAN EAST



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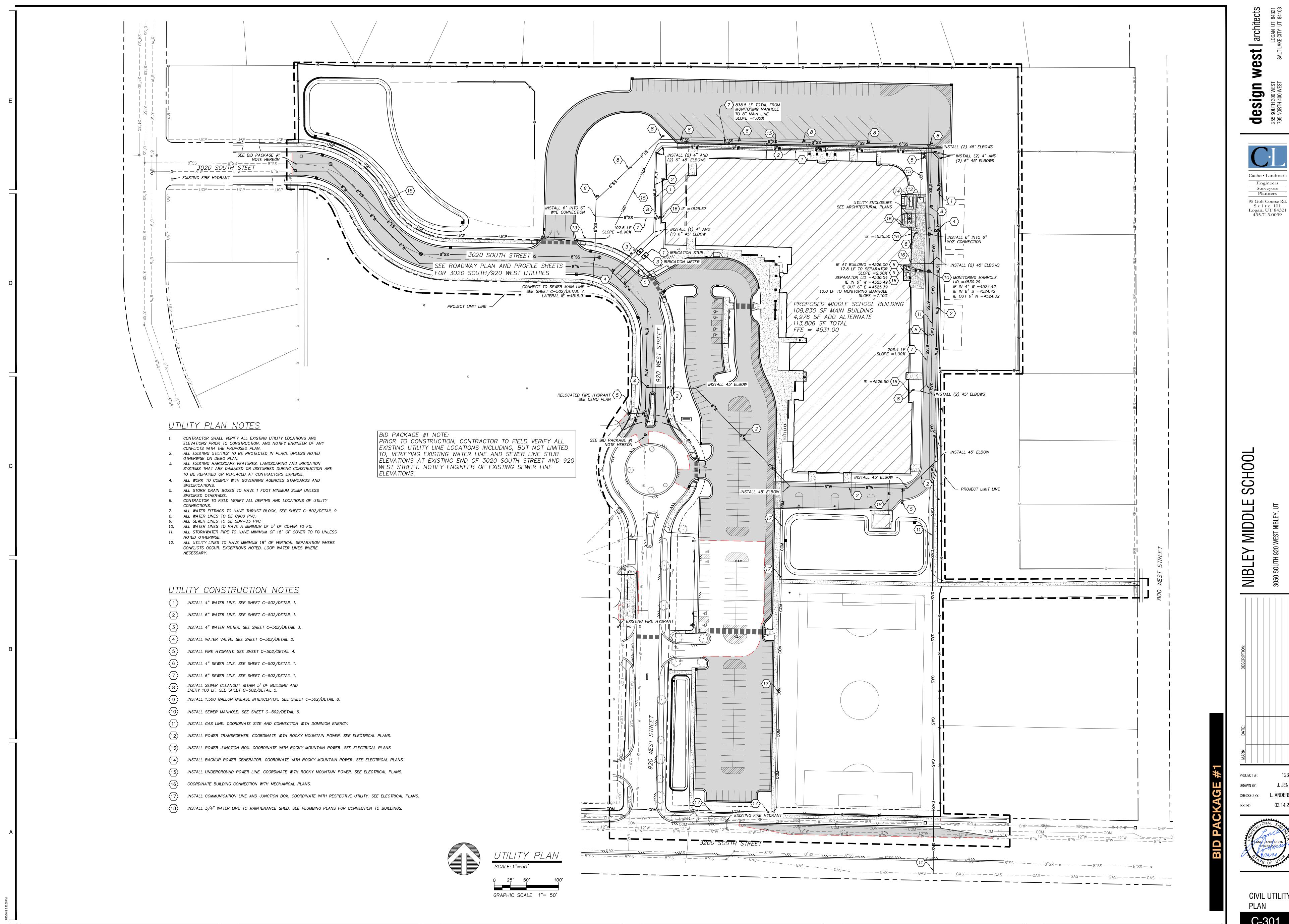
CHECKED BY: L. ANDERSON

03.14.2024



CIVIL- SITE PLAN SOUTHEAST

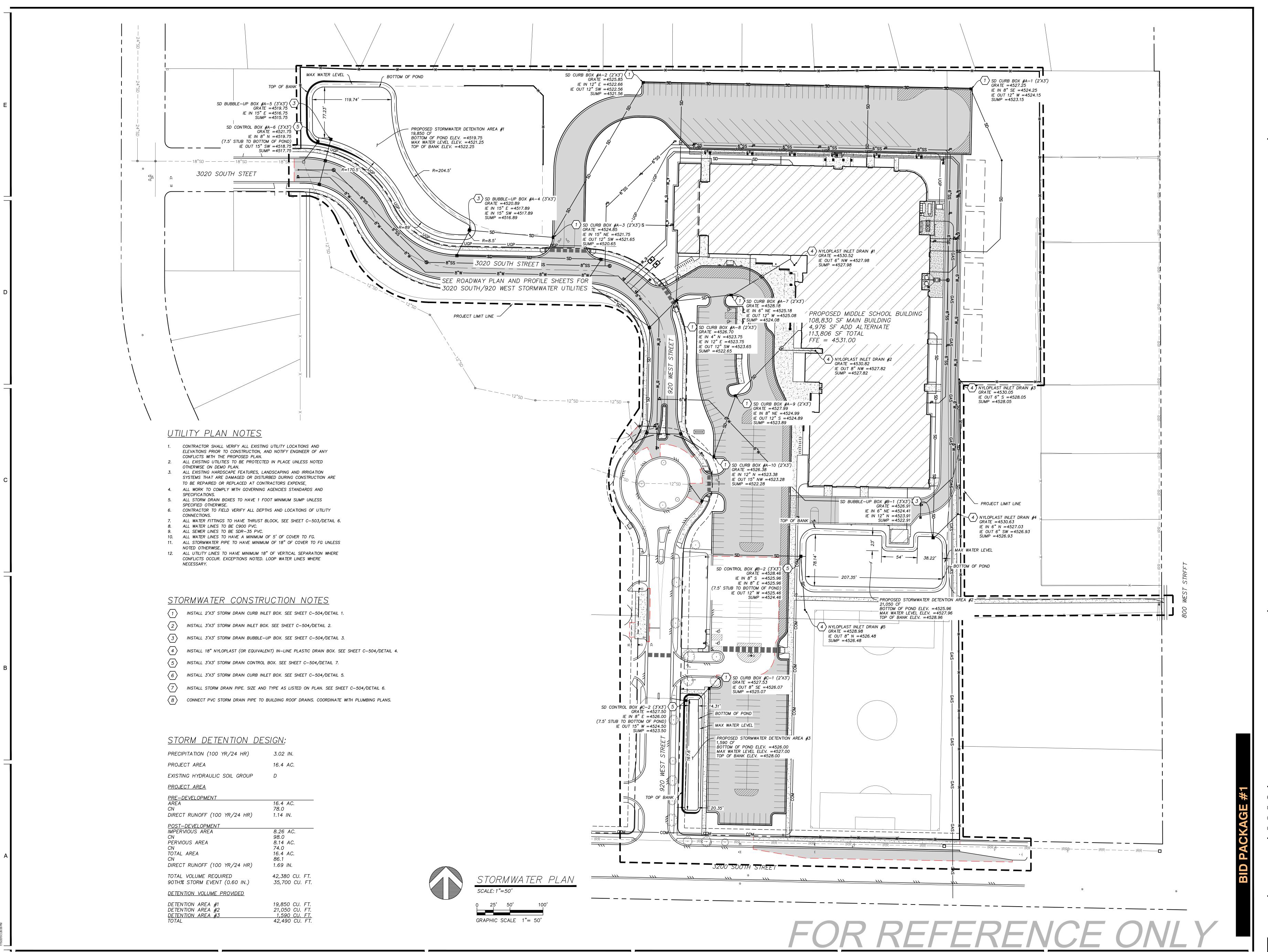




design

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CIVIL UTILITY



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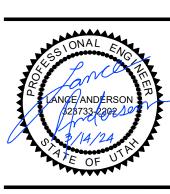
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IBLEY MIDDLE SCHOOL

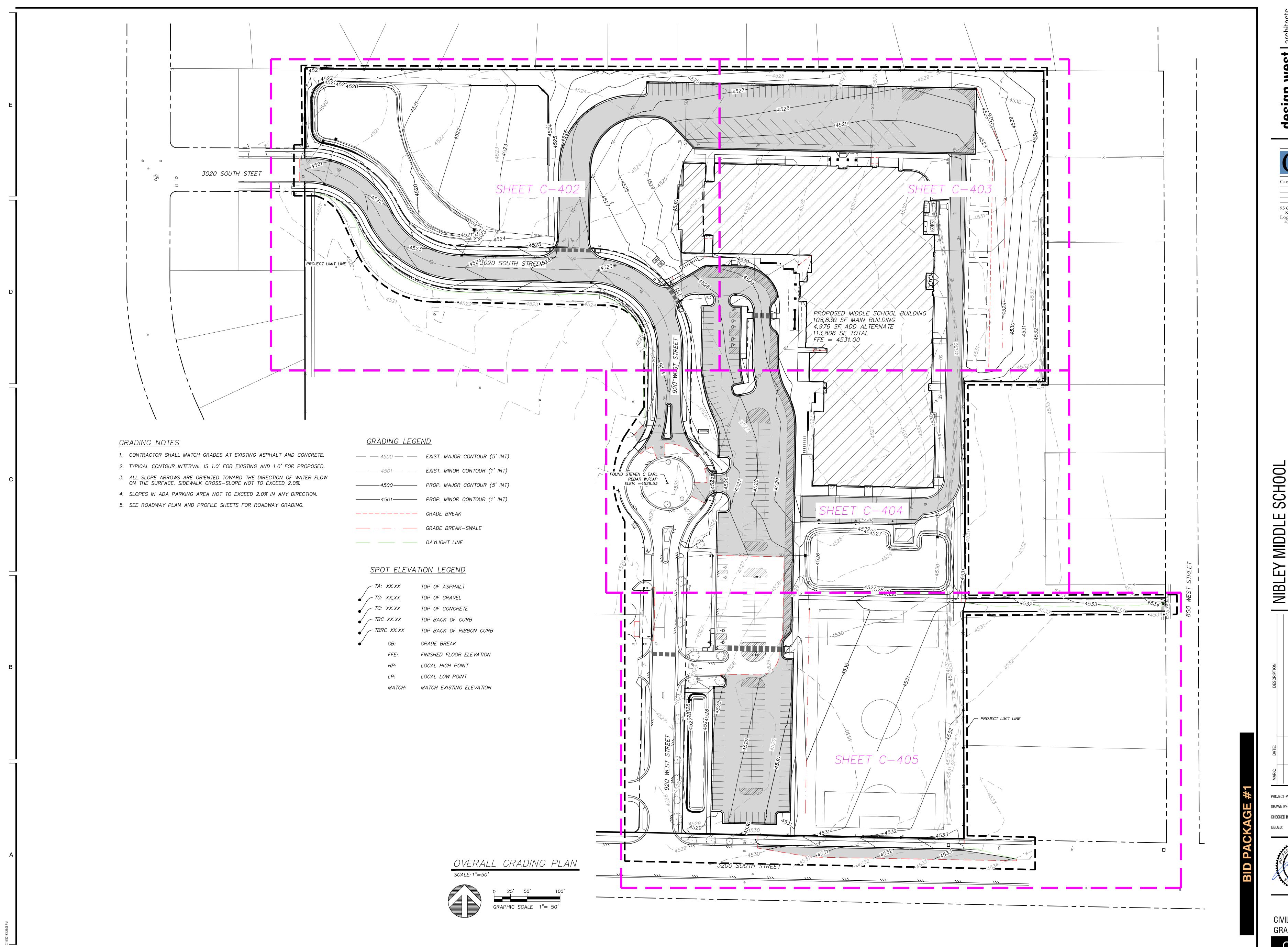
ARK: DATE: DESCRIPTION:

JECT #: 123005
WN BY: J. JENSEN
CKED BY: L. ANDERSON
ED: 03.14.2024



CIVIL STORMWATER PLAN

C-302



West architects
LOGAN UT 84321
SALT LAKE CITY UT 84103

design V 255 SOUTH 300 WEST 795 NORTH 400 WEST



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| NIBLEY MIDDLE SCHOO | 3050 SOUTH 920 WEST NIBLEY, UT

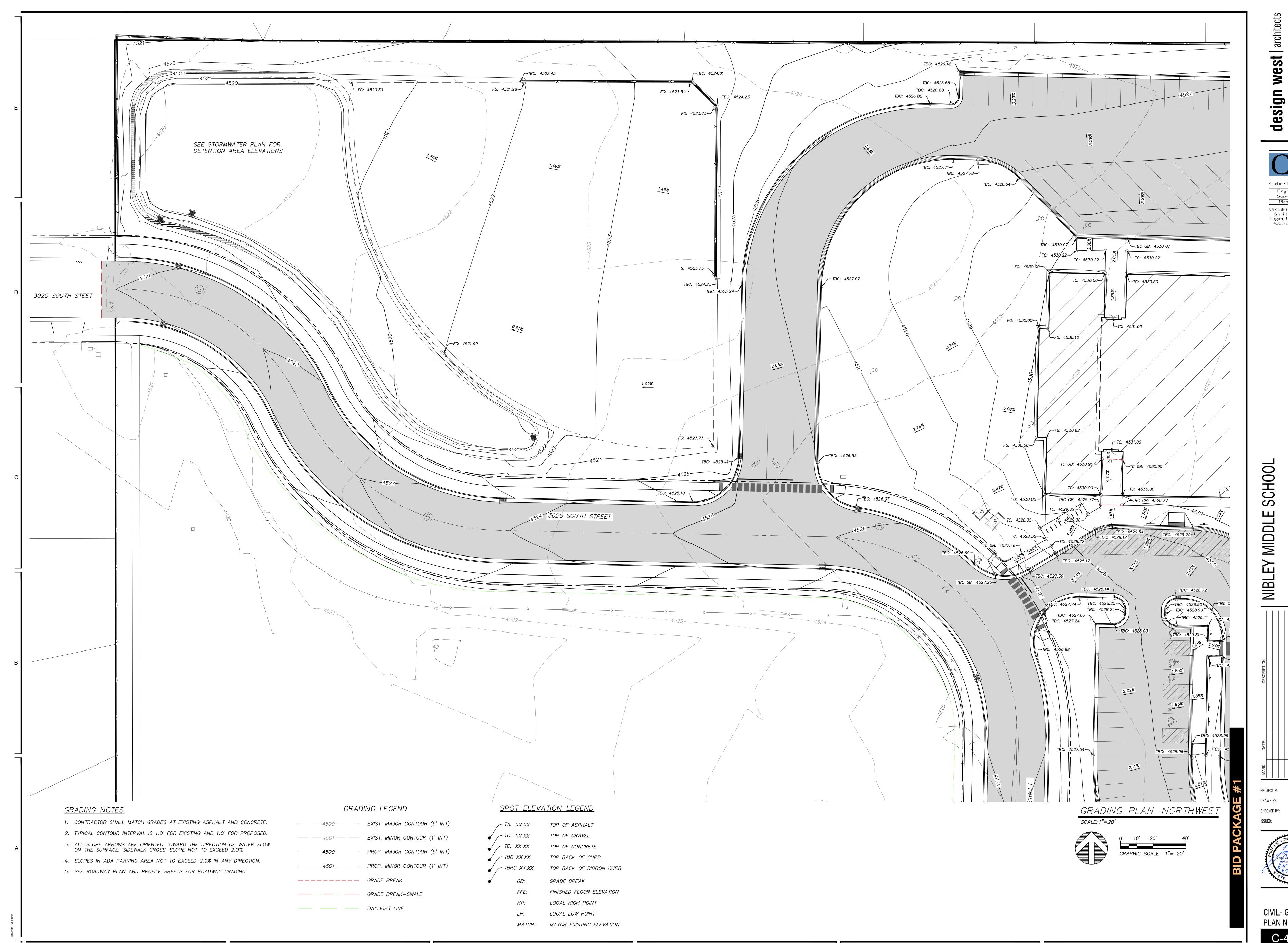
ROJECT #: 1230

ECT #: 123005
/N BY: J. JENSEN
KED BY: L. ANDERSON
D: 03.14.2024



CIVIL- OVERALL GRADING PLAN

C-401

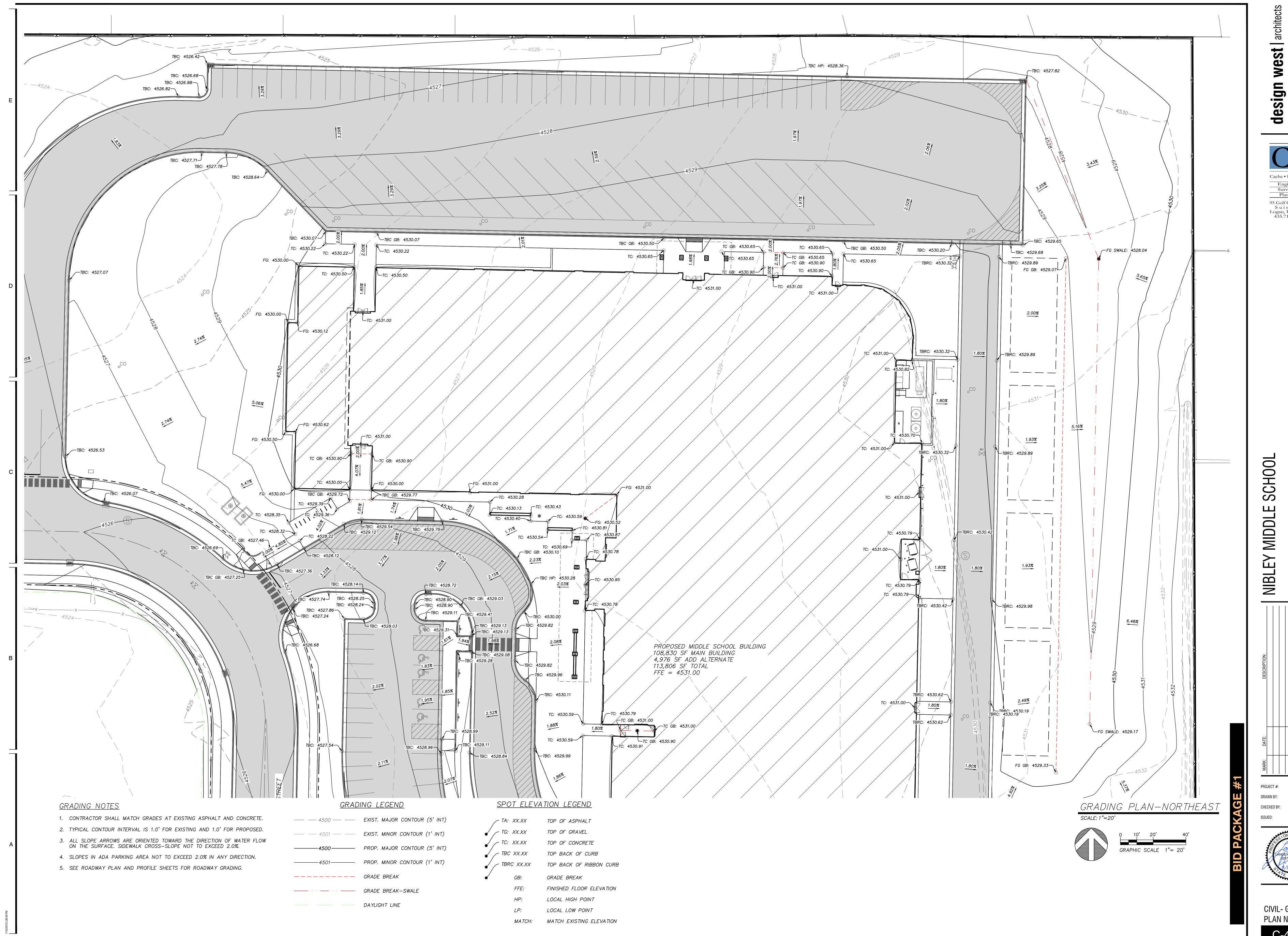


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CIVIL- GRADING PLAN NORTHWEST

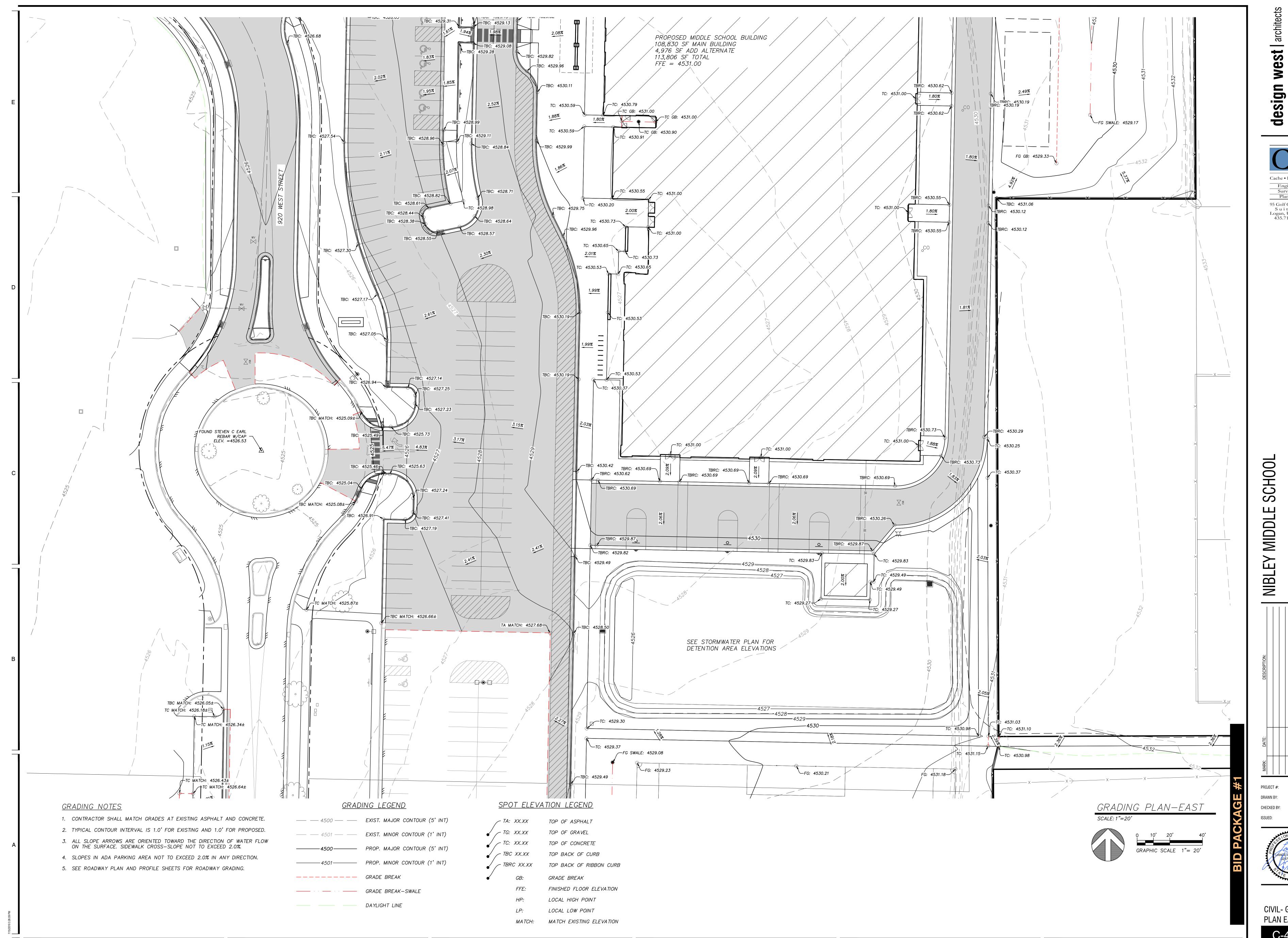


design 255 SOUTH 300 WEST 795 NORTH 400 WEST

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CIVIL- GRADING PLAN NORTHEAST

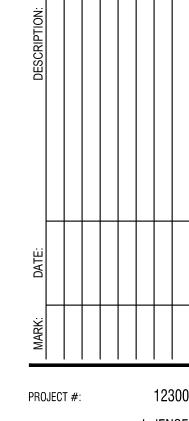


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design 255 SOUTH 300 WEST 795 NORTH 400 WEST

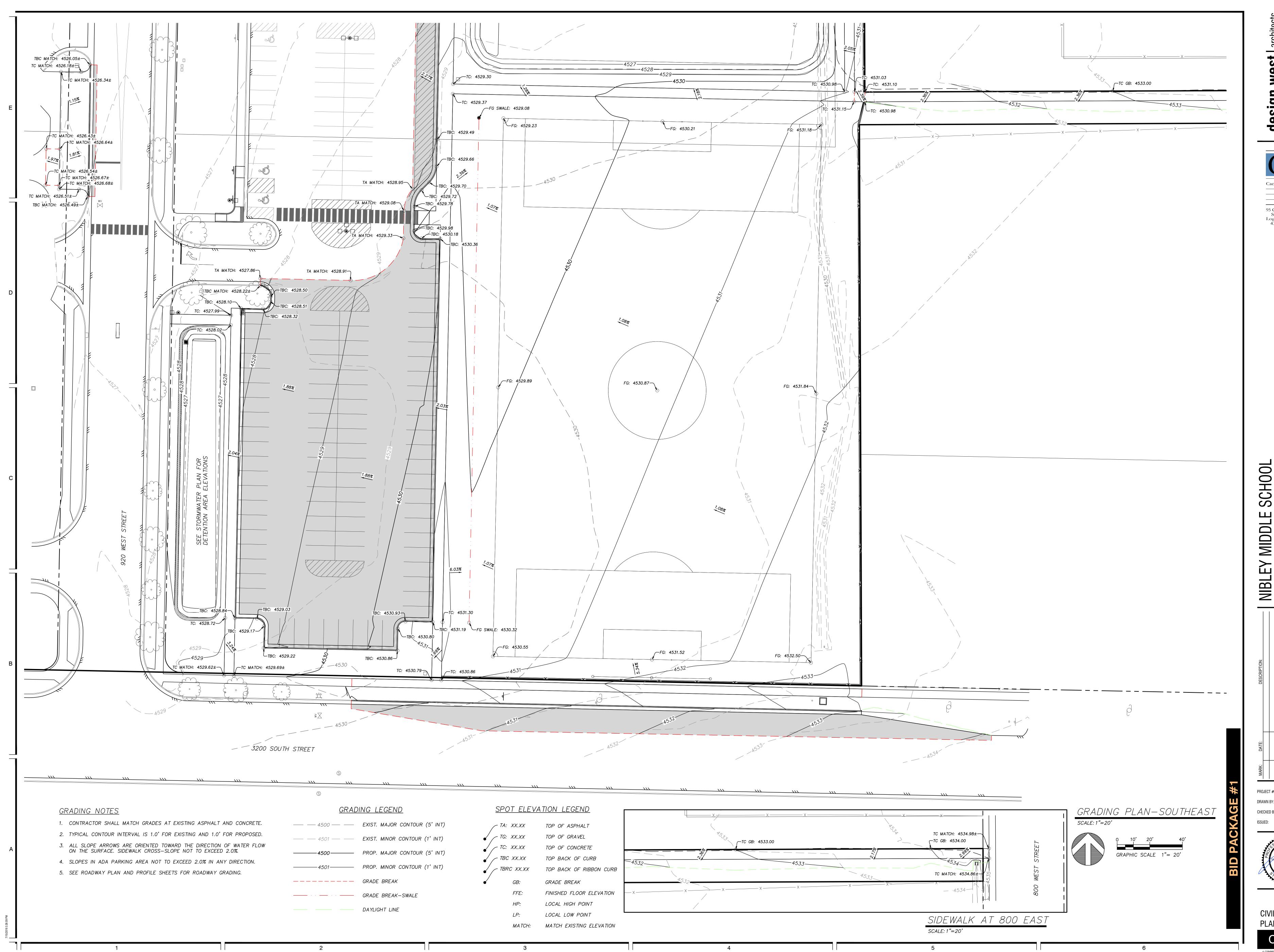


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CIVIL- GRADING PLAN EAST



architects

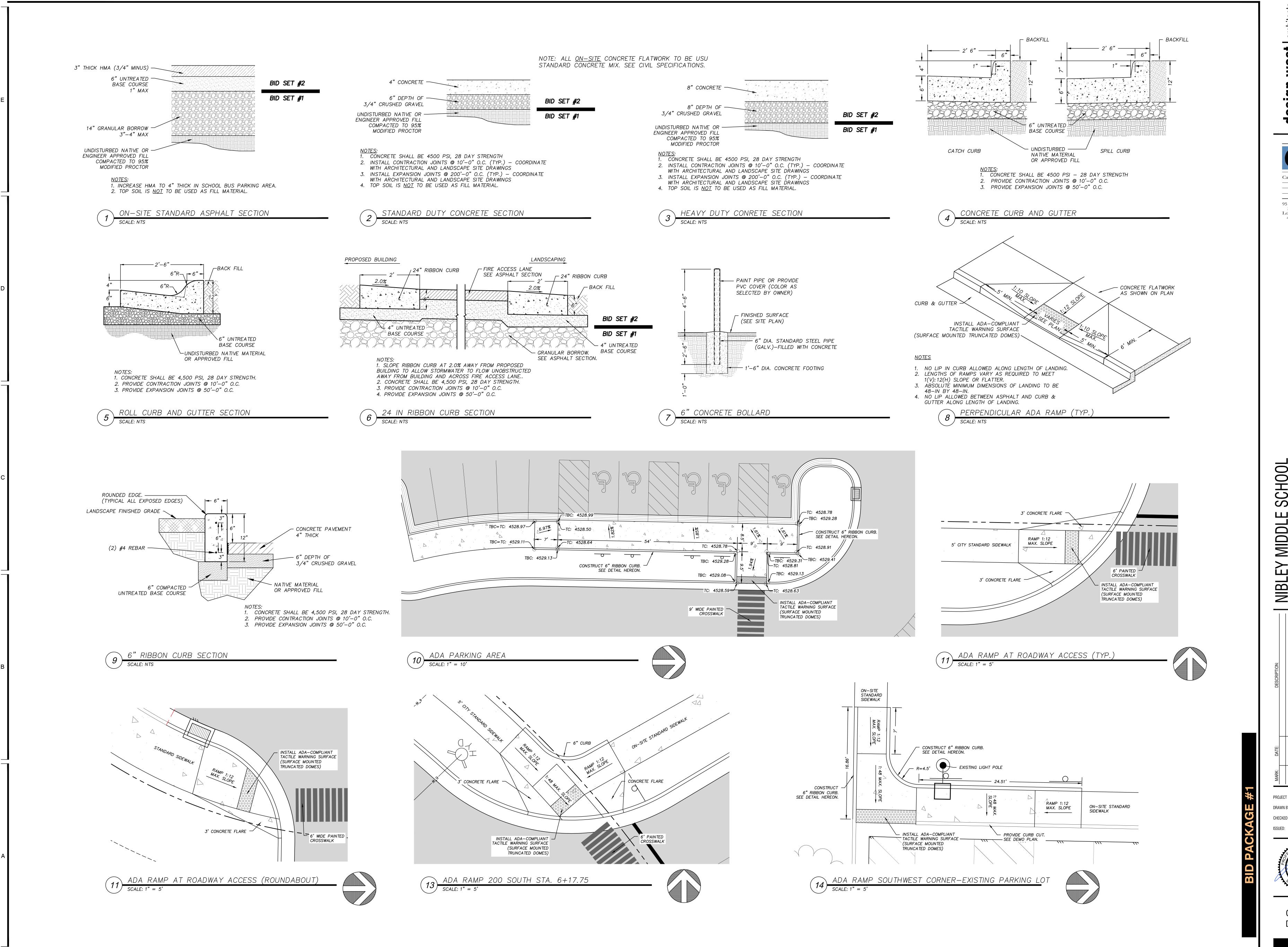
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CIVIL- GRADING PLAN SOUTHEAST



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L. ANDERSON



CIVIL UTILITY **DETAILS**

C-501

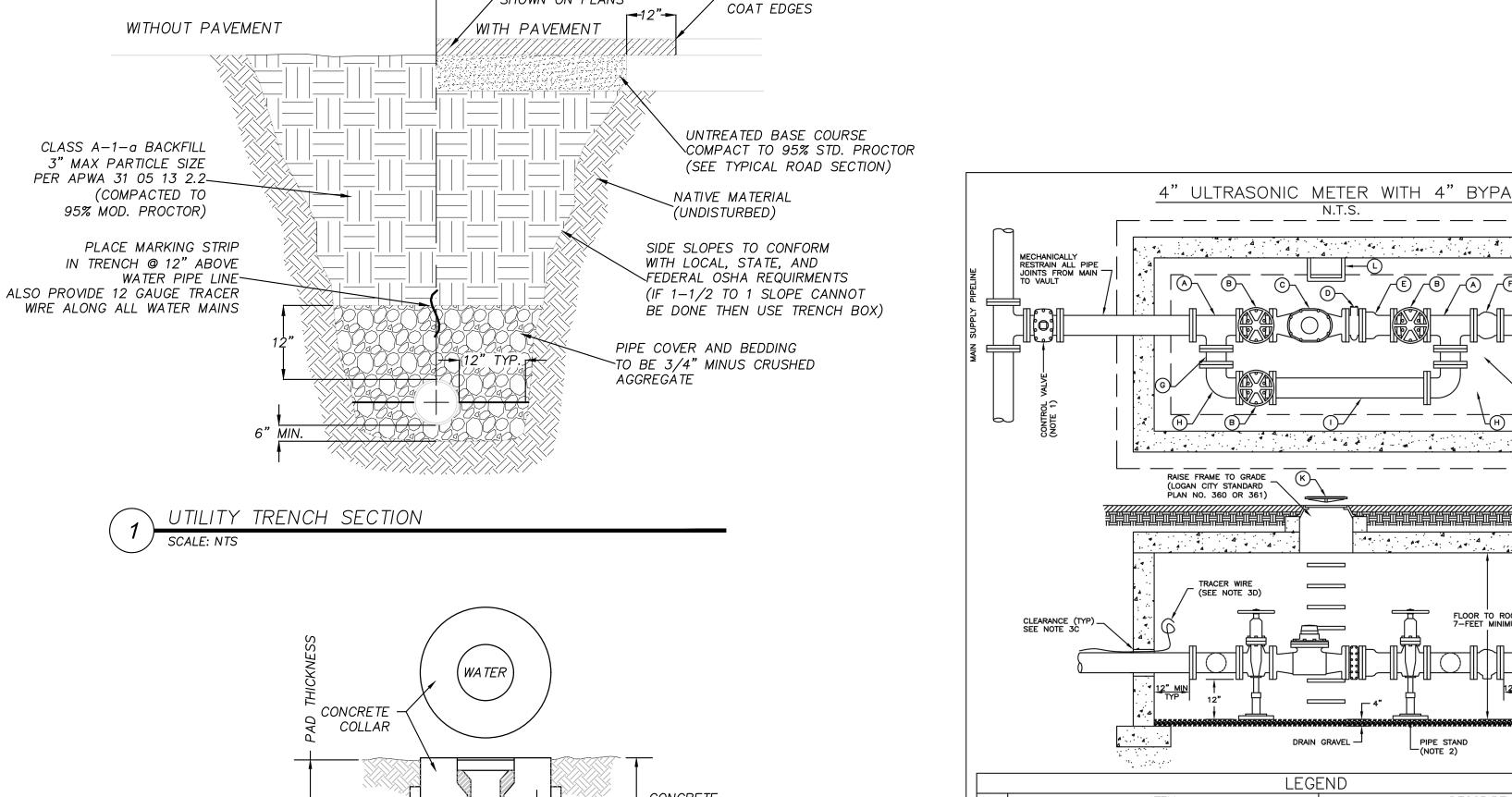
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03.14.2024

CIVIL UTILITY **DETAILS**





SAW CUT REQUIRED

CLEAN AND TACK

PAVEMENT SECTION AS

/SHOWN ON PLANS

COMPACTED GRANULAR BORROW

1. PLACE CONCRETE THAT IS AT

DIMENSION AROUND VALVE BOX OF 12

2. THE CONCRETE COLLAR AROUND THE TOP OF THE VALVE BOX SHALL BE

WATER VALVE DETAIL

(CONCENTRIC RISER)

grout at pipe

penetrations

4 OR 5 FEET NAMETER

PARALLEL TO THE ROAD SURFACE.

Cones shall be

Provide ladder

CONCRETE COLLAR

ALL AROUND

2 layers of

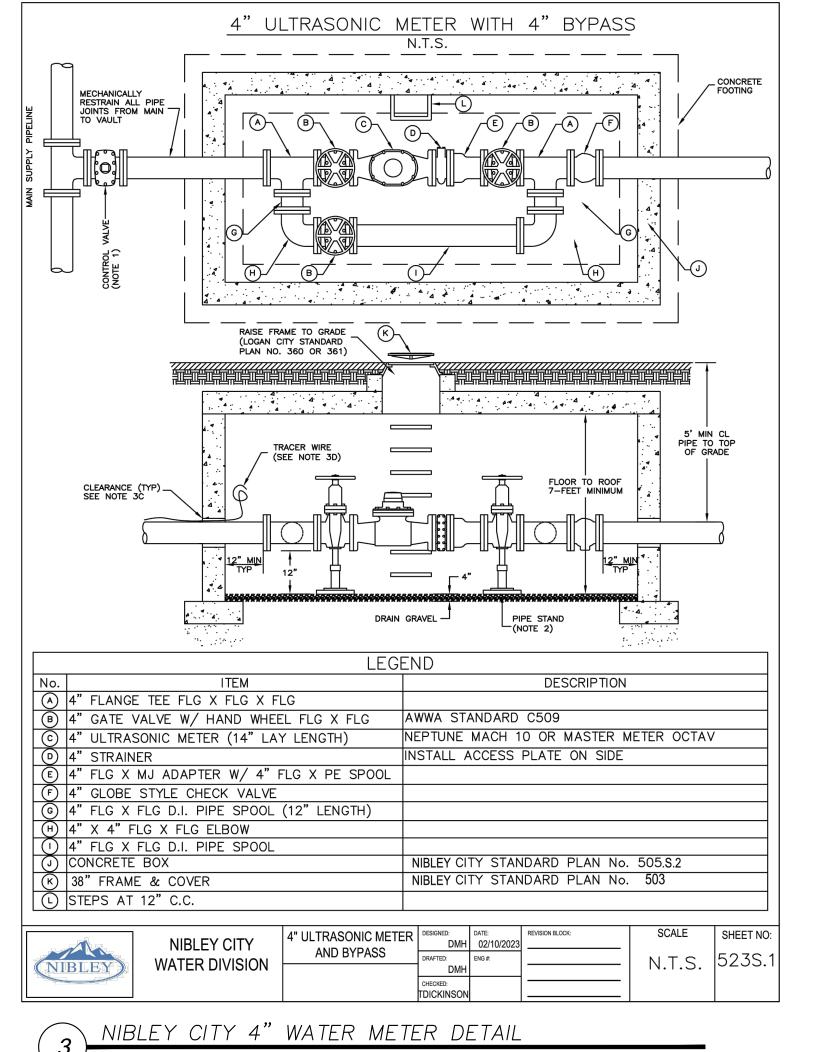
Kent seal

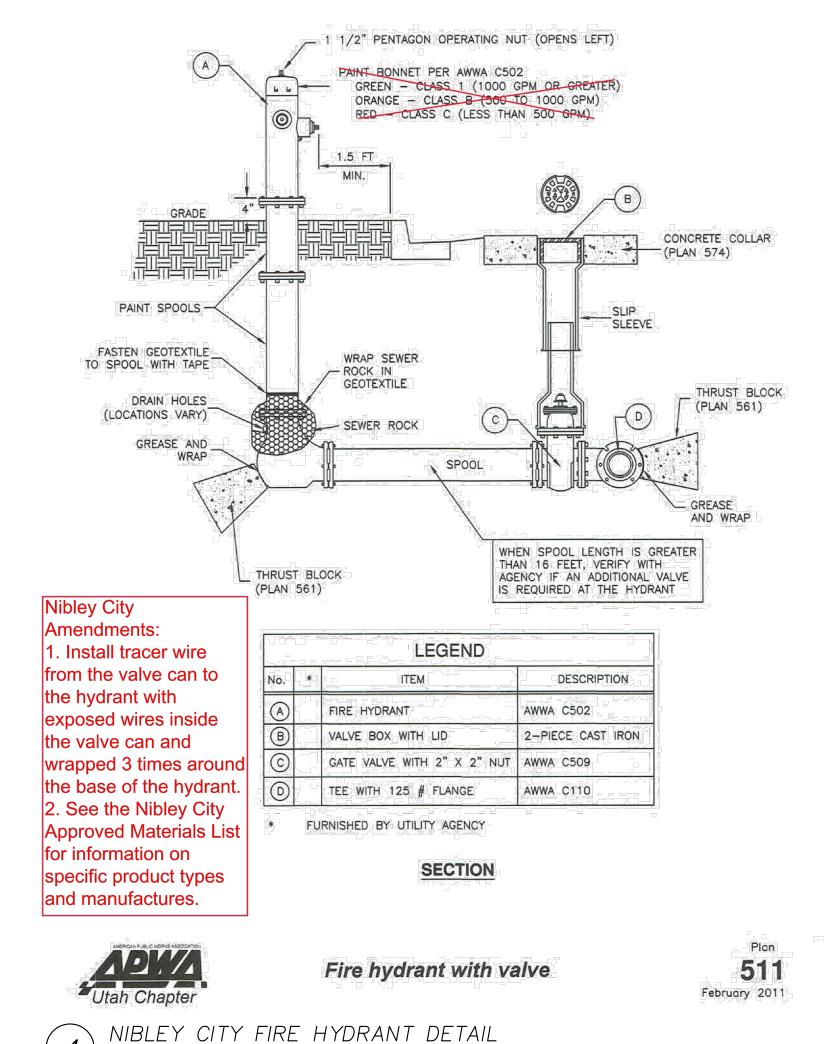
at each

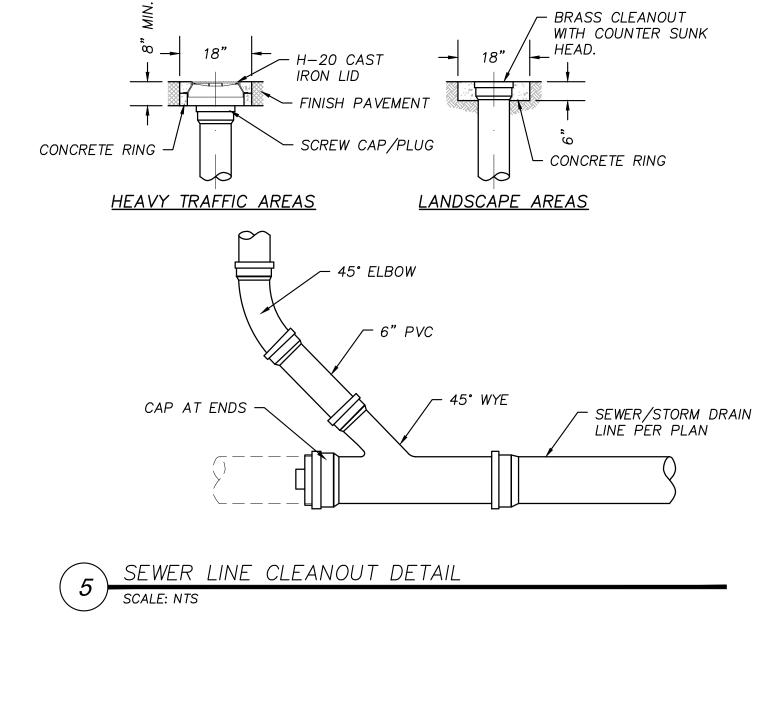
(PLAN 413)

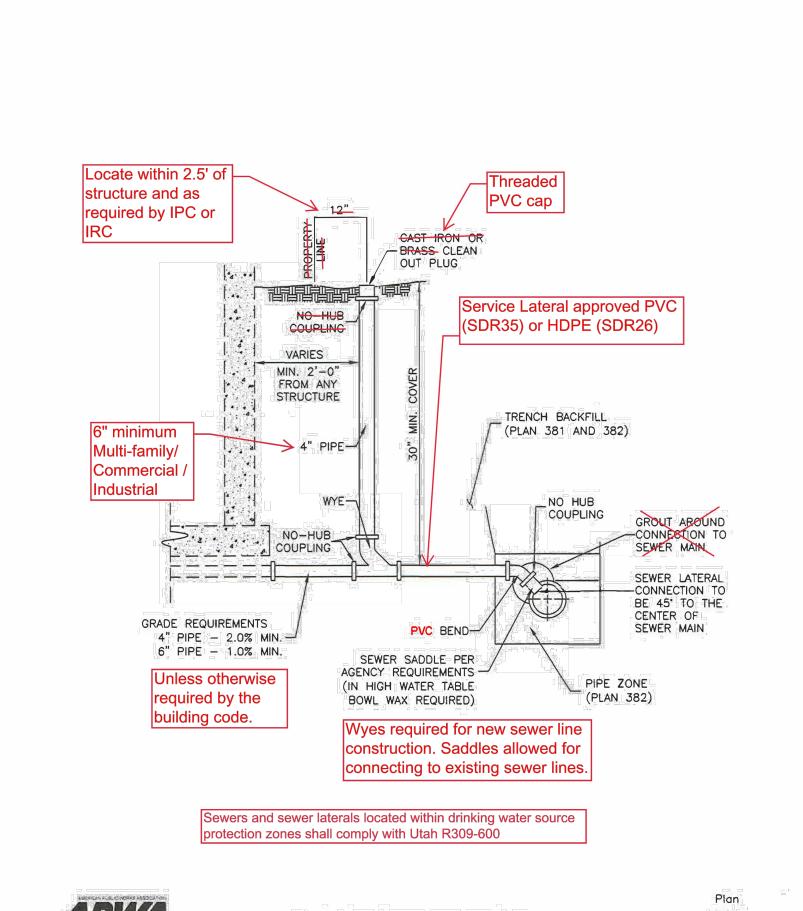
access

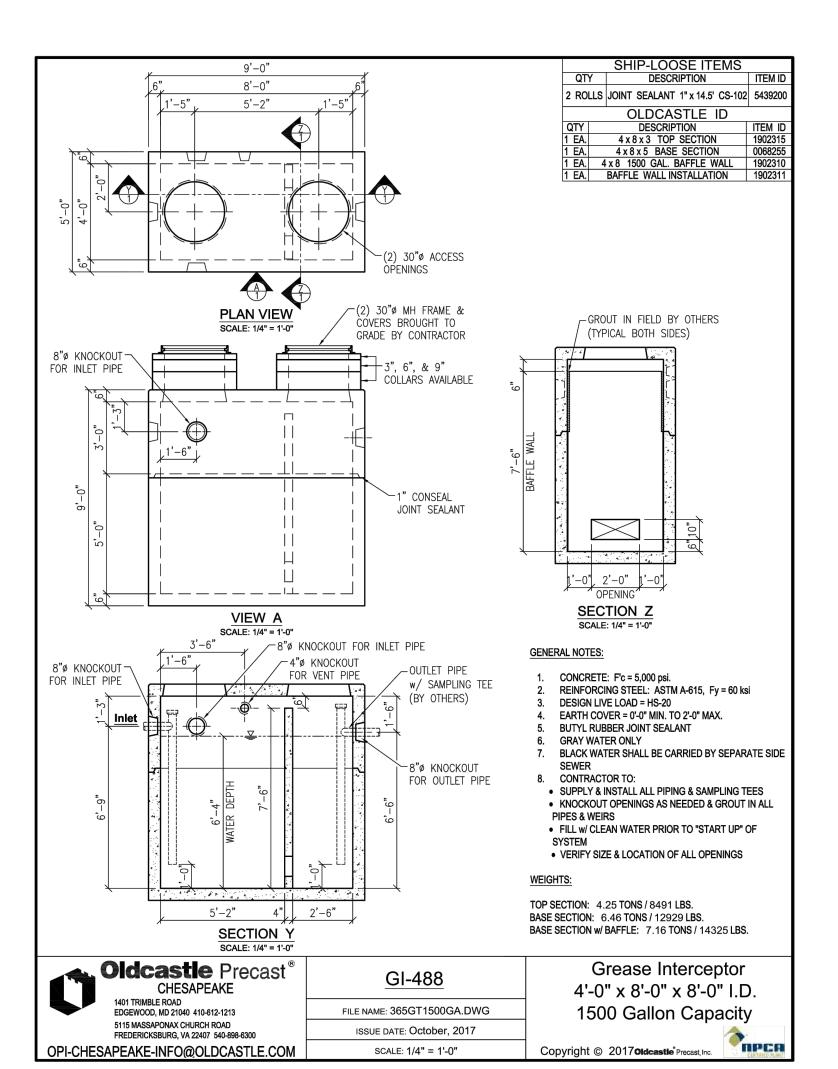
LEAST 8" THICK AND HAVE MIN.











THRUST BLOCK DESIGN INFO								
STATIC PRE	SSURE (PSi)	SOIL BEARING	SOIL BEARING STRENGTH (PSF)					
15	50	20	000	1.50				
THRUST BLOCK AREA REQUIRED (SQ FT)								
PIPE SIZE DEAD E		90° ELBOW	45° ELBOW	22.5° ELBO				
4	1.4	2.0	1.1	0.6				
6	3.2	4.5	2.4	1.2				
8	5.7	8.0	4.3	2.2 3.4				
10	8.8	12.5	6.8					
12	12.7	18.0	9.7	5.0				
14	17.3	24.5	13.2	6.8				
16	22.6	32.0	17.3	8.8				
18	28.6	40.5	21.9	11.2				
20	35.3	50.0	27.0	13.8				
24	50.9	71.9	38.9	19.8				

THRUST BLOCK DETAIL

USE ONLY REDWOOD $^{\perp}$ OR CEDAR TIMBER

AND PIPE.

FOR BLOCKING

BEARING AREA, TYP

TO PLACING CONCRETE

1. NO CONCRETE SHALL BE PLACED WITHIN 1

1/2" OF JOINT OR BOLTS. COVER ALL

2. THRUST BLOCKS SHALL BE REQUIRED AT

ALL 11.25° BENDS OR GREATER 3. PLACE CONCRETE AGAINST UNDISTURBED

5. IF STATIC PRESSURE AND/OR SOIL

REQUIRED THRUST BLOCK AREA

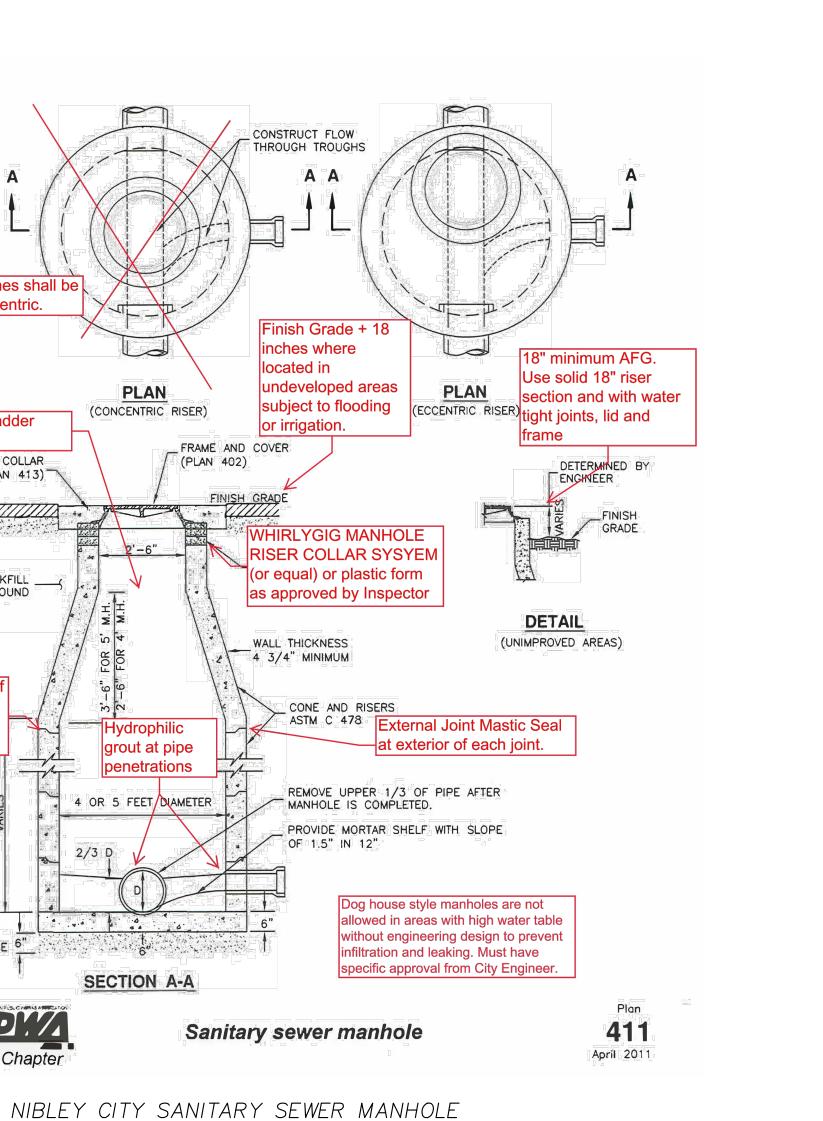
METAL CONTACT W/ POLY-WRAP PRIOR

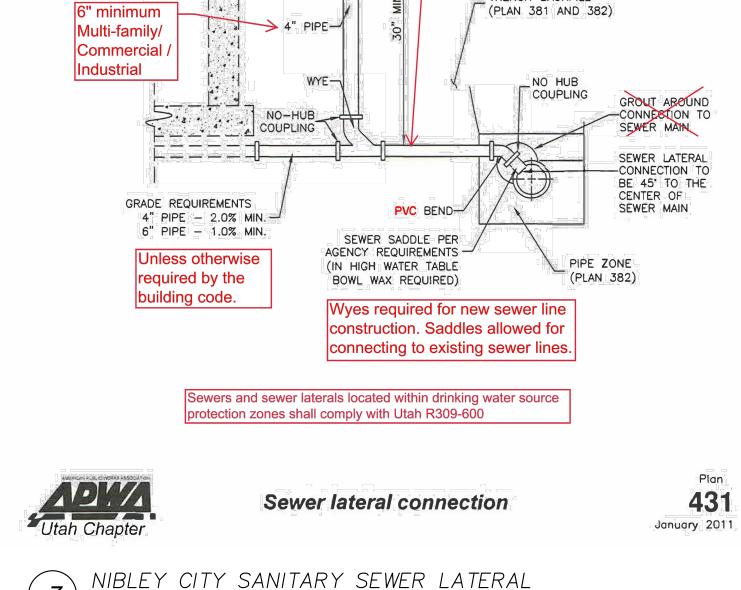
GROUND IN TRENCH BOTTOM AND SIDES.

4. USE 6 MIL VISQUEEN BETWEEN CONCRETE 🕥

BEARING STRENGTH DIFFÉR THAN THOSE

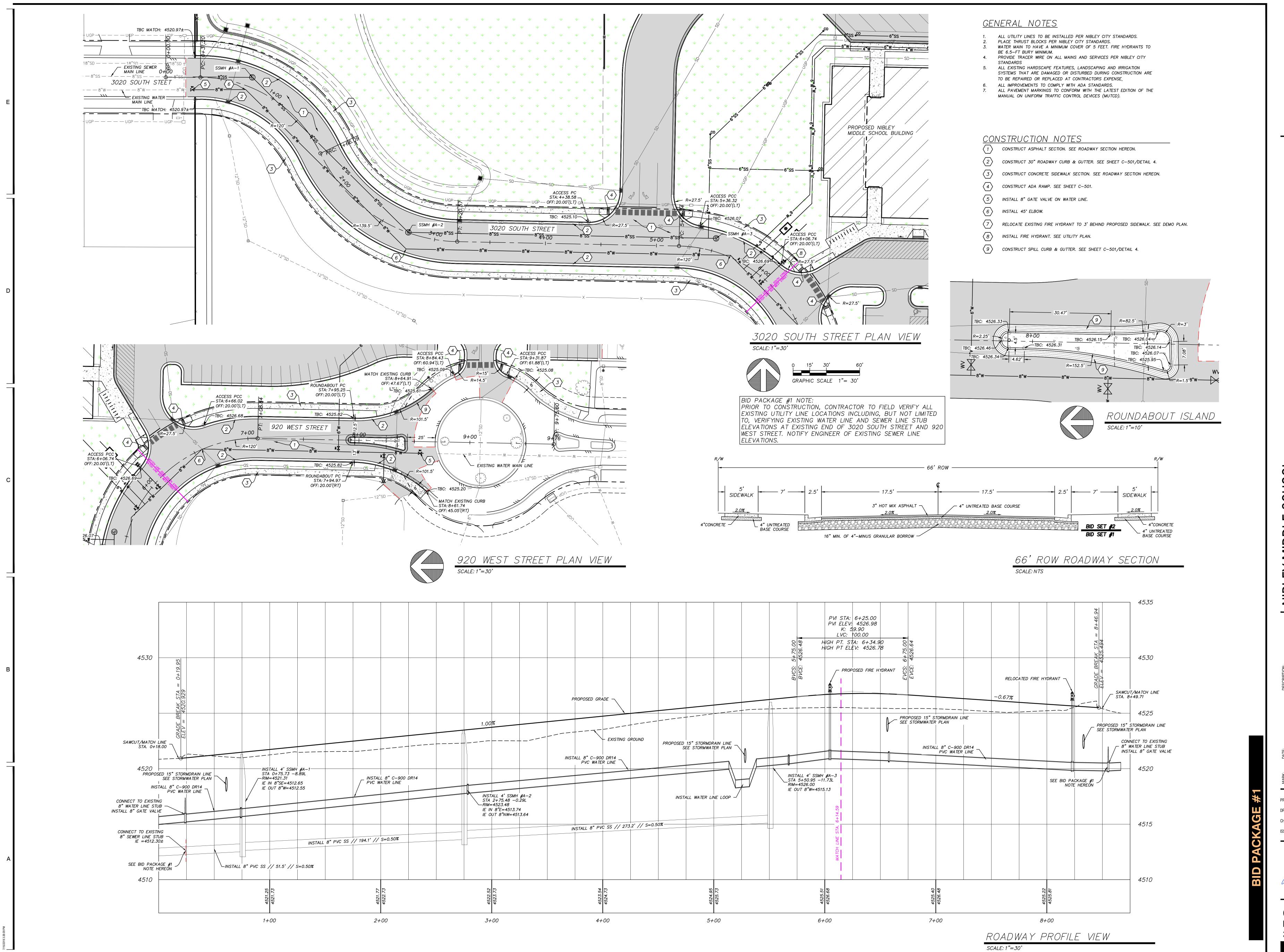
LISTED BELOW, CONTACT ENGINEER FOR





SCALE: NTS

	OLDCASTLE	1500	GALLON	GREASE	INTERCEPTOR
8	SCALE: NTS				



architects ゖゖ

esign



Q 255 795

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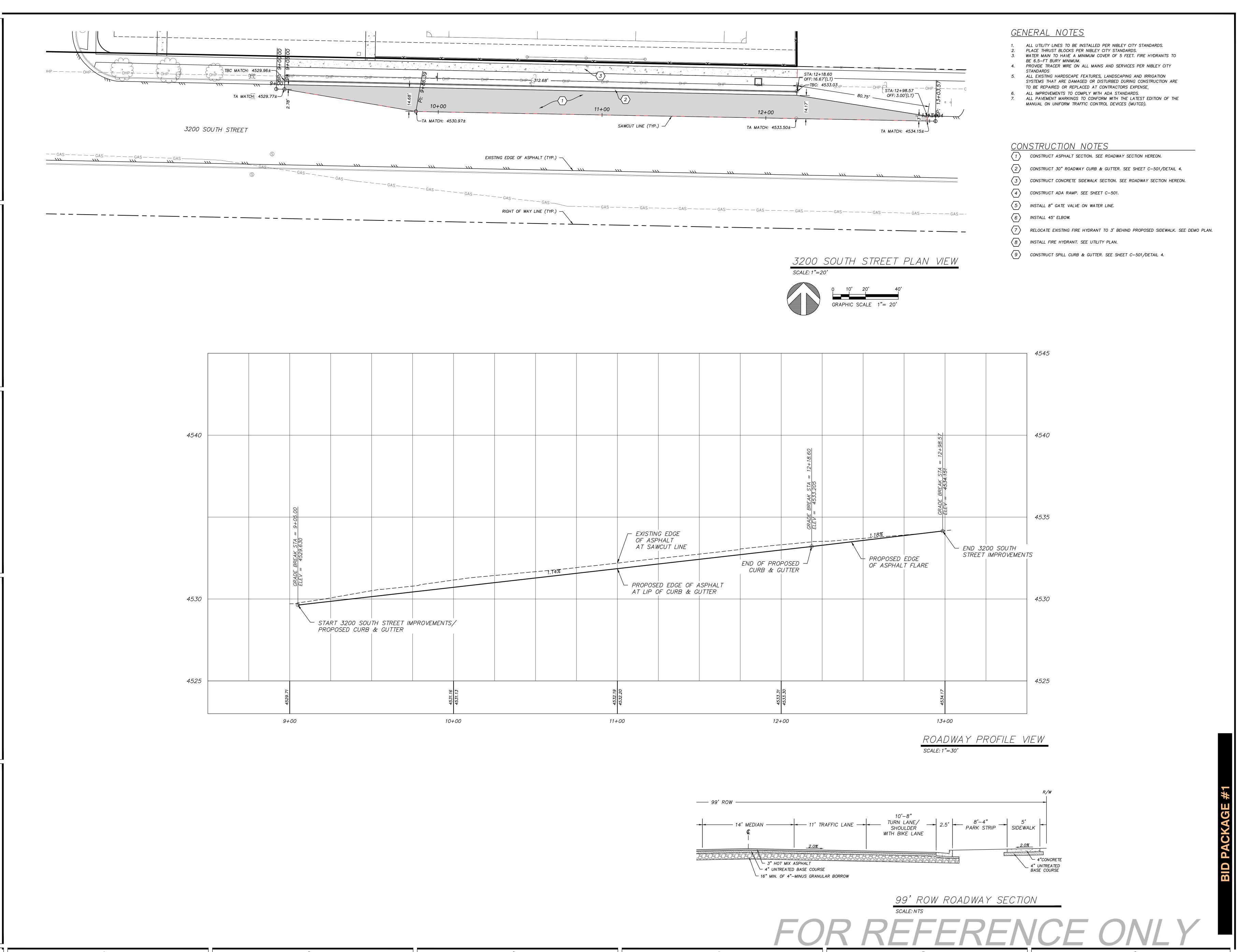
MIDDLE

L. ANDERSON 03.14.2024



CIVIL- ROADWAY PLAN & PROFILE 3020 S / 920 W

C-601



| architects LOGAN UT 84321 KKE CITY UT 84103

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G 255 S N 267 N

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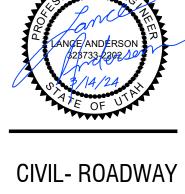
DESCRIPTION:

| NIBL | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 | 1990 |

ECT #: 123005
VN BY: J. JENSEN
SKED BY: L. ANDERSON
ED: 03.14.2024

EKED BY: L. ANDERSON
ED: 03.14.2024

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CIVIL- ROADWAY PLAN & PROFILE 3200 SOUTH

C-602

STRUCTURAL NOTES:

- THE STRUCTURAL NOTES ARE INTENDED TO COMPLEMENT THE PROJECT SPECIFICATIONS WHICH ARE PART OF THE CONSTRUCTION DOCUMENTS. SPECIFIC NOTES AND DETAILS ON THE DRAWINGS SHALL GOVERN OVER THE STRUCTURAL NOTES AND TYPICAL DETAILS. THESE DRAWINGS (AND, WHERE APPLICABLE, ACCOMPANYING WRITTEN SPECIFICATIONS) ARE THE ONLY CONTRACT DOCUMENTS PROVIDED BY ARW ENGINEERS FOR THE PROJECT REPRESENTED
- HEREIN. NOTHING IN ANY DIGITAL MODEL OR DIGITAL FILE RELATED TO THIS PROJECT SHALL BE TAKEN TO SUPERSEDE ANY INFORMATION SHOWN IN THESE DRAWINGS (INCLUDING, BUT NOT LIMITED TO. DIMENSIONS, SIZES, ETC). 3. THE ARCHITECTURAL DRAWINGS ARE THE PRIME CONTRACT DRAWINGS. THE STRUCTURAL DRAWINGS ARE SUPPLEMENTARY TO AND MUST BE USED IN CONJUNCTION WITH THE ARCHITECTURAL DRAWINGS AND OTHER CONSULTANTS DRAWINGS. ALL OMISSIONS OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR SPECIFICATIONS SHALL BE BROUGHT TO THE

ATTENTION OF THE ARCHITECT AND STRUCTURAL ENGINEER BEFORE PROCEEDING WITH ANY WORK

- INVOLVED. IN CASE OF CONFLICT, FOLLOW THE MOST STRINGENT REQUIREMENT AS DIRECTED BY THE ARCHITECT AT NO ADDITIONAL COST TO THE OWNER. 4. SEE SPECIFICATIONS FOR REQUIRED SUBMITTALS. SUBMITTALS SHALL BE MADE IN A TIMELY MANNER AS INDICATED IN SPECIFICATIONS. REVIEW OF SUBMITTALS BY ARW ENGINEERS IS FOR GENERAL COMPLIANCE ONLY AND IS NOT INTENDED AS APPROVAL. CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL SIZES, DIMENSIONS, AND ELEVATIONS ON SUBMITTALS AS RELATED TO DESIGN DOCUMENTS. PREPARATION OF SHOP DRAWINGS FOR STRUCTURAL ELEMENTS WILL REQUIRE INFORMATION (I.E. DIMENSIONS, ETC.) FOUND IN THE ARCHITECTURAL, STRUCTURAL, AND OTHER
- CONSULTANTS DRAWINGS. THE CONTRACTOR SHALL VERIFY ALL CONDITIONS AND DIMENSIONS AT THE SITE. IF ACTUAL CONDITIONS DIFFER FROM THOSE SHOWN ON CONTRACT DOCUMENTS, CONTRACTOR SHALL NOTIFY
- ARCHITECT PRIOR TO FABRICATION OR CONSTRUCTION OF ANY AFFECTED ELEMENTS. 6. THE CONTRACTOR SHALL COORDINATE AND VERIFY ALL LOCATIONS AND SIZES OF MECHANICAL EQUIPMENT OR OTHER EQUIPMENT BEFORE FABRICATING AND ERECTING STRUCTURAL ELEMENTS. SIZES AND LOCATIONS THAT DIFFER FROM THOSE SHOWN ON THE CONTRACT DOCUMENTS SHALL BE REPORTED TO THE ARCHITECT.
- THE CONTRACTOR SHALL SUBMIT A WRITTEN REQUEST TO THE ARCHITECT FOR ARCHITECT AND/OR ENGINEER APPROVAL BEFORE PROCEEDING WITH ANY CHANGES, MODIFICATIONS, OR
- OBSERVATION VISITS TO THE SITE BY ARW ENGINEERS FIELD REPRESENTATIVES SHALL NEITHER BE CONSTRUED AS INSPECTION NOR APPROVAL OF CONSTRUCTION. 9. DURING AND AFTER CONSTRUCTION, BUILDER AND/OR OWNER SHALL KEEP LOADS ON STRUCTURE
- WITHIN THE LIMITS OF DESIGN LOADS AS NOTED IN THESE DOCUMENTS. 10. TYPICAL OR SIMILAR DETAILS AND SECTIONS SHALL APPLY WHERE SPECIFIC DETAILS ARE NOT
- SHOWN. TYPICAL OR SIMILAR DETAILS REFER TO THE CONDITION ADDRESSED AND ARE NOT NECESSARILY DETAILS LABELED "TYPICAL" OR "SIMILAR" IN THE PLANS AND DOCUMENTS. 11. DRAWINGS AND DETAILS HAVE BEEN PREPARED WITH THE INTENT TO VISUALLY REPRESENT
- PLANS OR DETAILS FOR DIMENSIONAL INFORMATION. 12. THE CONTRACTOR SHALL PROVIDE ADEQUATE TEMPORARY SHORING AND BRACING FOR ALL STRUCTURAL ELEMENTS UNTIL THE ENTIRE STRUCTURAL SYSTEM IS COMPLETED. DESIGN OF ALL

INFORMATION PROVIDED IN SCALED FORM; HOWEVER CONTRACTOR/SUPPLIERS SHOULD NOT SCALE

- SHORING AND BRACING IS BY OTHERS AT NO ADDITIONAL COST TO THE OWNER. 13. ENGINEER SHALL NOT BE RESPONSIBLE FOR ACTIVITIES UNDER CONTROL OF THE CONTRACTOR SUCH AS CONSTRUCTION SITE SAFETY, MEANS, METHODS AND SEQUENCING OF CONSTRUCTION. ENGINEER SHALL NOT BE RESPONSIBLE FOR FABRICATION, ERECTION AND CONSTRUCTION REQUIREMENTS AS PRESCRIBED BY OSHA OR OTHER REGULATORY AGENCIES REGARDLESS OF INDICATIONS IN THESE
- 14. NOTICE OF COPYRIGHT: THESE STRUCTURAL DRAWINGS ARE HEREBY COPYRIGHTED BY ARW ENGINEERS, ALL RIGHTS RESERVED. THESE DOCUMENTS DEFINE A STRUCTURE AND ARE INSTRUMENTS OF SERVICE, FOR ONE USE ONLY. REPRODUCTION AND DISTRIBUTION OF THESE DRAWINGS IS ONLY ALLOWED AS REQUIRED FOR REGULATORY AGENCIES AND FOR CONVEYANCE OF INFORMATION TO PARTIES INVOLVED IN THE CONSTRUCTION OF THIS PROJECT. THESE DOCUMENTS SHALL NOT BE REPRODUCED OR COPIED, IN PART OR WHOLE BY ANY PARTY FOR USE IN PREPARATION OF SHOP DRAWINGS OR OTHER SUBMITTALS.
- 15. WHERE THE WORD "SHALL" OCCURS IN THESE DRAWINGS AND ANY ACCOMPANYING SPECIFICATIONS, IT IS CONSIDERED A MANDATORY OBLIGATION AND SYNONYMOUS WITH THE PHRASE "HAS DUTY TO".

B. STATEMENT OF SPECIAL INSPECTIONS AND SPECIAL INSPECTIONS

- 1. THE DESIGNATED SEISMIC/WIND SYSTEMS AND SEISMIC/WIND-FORCE-RESISTING SYSTEMS THAT ARE SUBJECT TO SPECIAL INSPECTIONS IN ACCORDANCE WITH IBC SECTION 1705.12 AND 1705.13 ARE IDENTIFIED ON THESE DOCUMENTS WITH A CIRCLE "L". ALL OTHER ITEMS REQUIRING SPECIAL INSPECTION ARE IDENTIFIED IN THE SPECIAL INSPECTION SCHEDULE ON SHEETS S-012 AND S-013. SPECIAL INSPECTIONS AND TESTING ARE TO BE PROVIDED AS REQUIRED BY IBC SECTIONS 1704
- THROUGH 1705 AND OTHER APPLICABLE SECTIONS OF THE IBC. THE TYPE AND FREQUENCY OF TESTING AND SPECIAL INSPECTIONS SHALL BE AS NOTED IN THE SPECIAL INSPECTION SCHEDULE. JOB SPECIFICATIONS, AND ACCORDANCE WITH IBC SECTION 110 AND CHAPTER 17. CONTRACTOR SHALL COORDINATE AND COOPERATE WITH REQUIRED INSPECTIONS. 3. ALL TESTING AND SPECIAL INSPECTION SHALL BE PROVIDED BY A QUALIFIED INDEPENDENT SPECIAL
- INSPECTION AGENCY IN ACCORDANCE WITH IBC 1704 AND AS OUTLINED IN THE JOB SPECIFICATIONS. REPORTS OF FINDINGS OR DISCREPANCIES SHALL BE NOTED AND FORWARDED TO THE CONTRACTOR. ARCHITECT, ENGINEERS, AND BUILDING OFFICIAL IN A TIMELY MANNER.
- STRUCTURAL OBSERVATION VISITS SHALL BE PERFORMED BY A REPRESENTATIVE FROM ARW ENGINEERS IN ACCORDANCE WITH THE CONTRACT AS NEEDED TO OBSERVE THE CONSTRUCTION OF CRITICAL BUILDING ELEMENTS (I.E. FOOTINGS, BRACED FRAMES, MOMENT FRAMES, DRAG STRUTS AND THEIR CONNECTIONS, COLLECTORS, AND ROOF AND FLOOR DIAPHRAGMS). STRUCTURAL OBSERVATION REPORTS FOR EACH VISIT SHALL BE SENT DIRECTLY TO THE ARCHITECT FOR DISTRIBUTION TO THE CONTRACTOR AND BUILDING OFFICIAL STRUCTURAL OBSERVATION VISITS SHALL NEITHER BE CONSTRUED AS SPECIAL INSPECTION NOR APPROVAL OF COMPLETED CONSTRUCTION.
- 5. IN ACCORDANCE WITH IBC 1704.4, THE CONTRACTOR SHALL SUBMIT A WRITTEN CONTRACTOR'S STATEMENT OF RESPONSIBILITY TO THE BUILDING OFFICIAL AND OWNER. THE STATEMENT SHALL BE SUBMITTED PRIOR TO THE CONSTRUCTION OF ANY SEISMIC/WIND-FORCE-RESISTING SYSTEM. DESIGNATED SEISMIC/WIND SYSTEM, OR COMPONENT IDENTIFIED IN THESE DOCUMENTS WITH A

C. BASIS OF DESIGN

- 1. GOVERNING BUILDING CODE : INTERNATIONAL BUILDING CODE (IBC) 2021
- SUSPENDED FLOOR LOADS a. LIVE LOAD = 40 PSF AT CLASSROOMS, 80 PSF AT CORRIDORS (UNREDUCED) b. DEAD LOAD = 70 PSF
- ROOF LOADS a. FLAT-ROOF SNOW LOAD, Pf: 33 PSF
 - 1. GROUND SNOW LOAD, Pg: 43 PSF 2. SNOW EXPOSURE FACTOR, Ce: 1.0
 - SNOW LOAD IMPORTANCE FACTOR, Is: 1.1 THERMAL FACTOR, Ct: 1.0
 - 5. SLOPE FACTOR, C_S: 1.0 6. SNOW DRIFT: SHOWN ON PLANS WHERE APPLICABLE.
- b. LIVE LOAD = 20 PSF c. DEAD LOAD = 20 PSF 4. WIND DESIGN
- a. BASIC WIND SPEED (3 SECOND GUST): 110 MPH b. WIND EXPOSURE : C
- c. INTERNAL PRESSURE COEFFICIENT, GCPI: 0.18
- d. COMPONENT AND CLADDING DESIGN WIND PRESSURE SHALL BE AS REQUIRED PER ASCE 7-16. 5. SEISMIC DESIGN:
- a. SEISMIC IMPORTANCE FACTOR, I_E: 1.25
- b. SITE CLASS : E MAPPED SPECTRAL RESPONSE ACCELERATIONS : $S_8 = 1.079$, $S_1 = 0.363$
- d. SPECTRAL RESPONSE COEFFICIENTS: $S_{DS} = 0.863$, $S_{D1} = 0.617$ e. SEISMIC DESIGN CATEGORY: D f. BASIC SEISMIC-FORCE-RESISTING SYSTEM: SPECIAL REINFORCED CONCRETE/MASONRY
- SHEARWALLS SEISMIC RESPONSE COEFFICIENT, Cs: .216
- . RESPONSE MODIFICATION FACTOR, R: 5 i. ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE

D. FOUNDATION

- GENERAL a. DESIGN SOIL PRESSURE: 3000 PSF
- b. SOILS REPORTS BY: A CACHE CORP REPORT #: 1220008 (HYDE PARK SITE) DATED NOVEMBER 12, 2022

THAT THE DIMENSIONS ARE INCREASED 3" ON ALL SIDE.

- REPORT #: 1230024 (NIBLEY SITE) DATED DECEMBER 11, 2023 c. SOIL PREPARATION UNDER FOUNDATIONS AND SLABS-ON-GRADE SHALL BE IN ACCORDANCE WITH
- THE SOILS REPORT d. TOP OF FOOTING ELEVATIONS SHOWN ON THE FOOTING AND FOUNDATION PLAN ARE BASED ON PRELIMINARY GRADING INFORMATION AND SHALL BE VERIFIED PRIOR TO CONSTRUCTION. STEPS WHERE SHOWN ARE AT APPROXIMATE LOCATIONS. ACTUAL STEP LOCATIONS SHALL BE AT THE
- CONTRACTOR'S DISCRETION BASED UPON FIELD CONDITIONS, ALL EXTERIOR FOUNDATIONS SHALL BEAR A MINIMUM OF 30" INCHES BELOW LOWEST ADJACENT FINAL GRADE. e. ALL WALLS (EXCEPT CANTILEVERED RETAINING WALLS) SHALL BE ADEQUATELY BRACED AGAINST LATERAL MOVEMENT PRIOR TO BACKFILLING. DESIGN AND ERECTION OF BRACING/SHORING SHALL
- SUPPORTING STRUCTURAL ELEMENTS ARE IN PLACE AND HAVE ATTAINED FULL STRENGTH. UNLESS NOTED OTHERWISE, ALL FOOTINGS AT COLUMNS SHALL BE CENTERED BELOW COLUMNS. UNLESS NOTED OTHERWISE, ALL FOOTINGS SHALL HAVE VERTICAL FACES FORMED WITH STANDARD FORMING MATERIALS (WOOD, METAL, ETC.). WITH PRIOR APPROVAL OF ARCHITECT AND ENGINEER, CONCRETE FOR FOOTINGS CAN BE PLACED IN EXCAVATED SOIL "FORMS" PROVIDED

BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR. BRACING SHALL REMAIN IN PLACE UNTIL

E. CONCRETE

- 1. ALL CONCRETE MIX DESIGNS SHALL COMPLY WITH THE PROJECT SPECIFICATIONS AND THE
- REQUIREMENTS LISTED BELOW: a. FOOTINGS, GRADE BEAMS, FOUNDATION WALLS: 1. WHERE THE TOP OF THE ELEMENT IS EXPOSED OR IS LOCATED WITHIN 30" OF THE LOWEST ADJACENT GRADE (EXPOSURE CATEGORY F2):
 - a. 28 DAY COMPRESSIVE STRENGTH: 4500 PSI b. MAXIMUM W/C RATIO: c. MAXIMUM AGGREGATE SIZE :
 - d. AIR CONTENT: SEE SCHEDULE BELOW 2. WHERE THE TOP OF THE ELEMENT IS NOT EXPOSED OR IS NOT LOCATED WITHIN 30" OF THE LOWEST ADJACENT GRADE (EXPOSURE CATEGORY F0): a. 28 DAY COMPRESSIVE STRENGTH: 3000 PSI
- b. INTERIOR SLABS ON GRADE (EXPOSURE CATEGORY F0): 1. 28 DAY COMPRESSIVE STRENGTH: 3000 PSI
- c. INTERIOR SUSPENDED SLABS (EXPOSURE CATEGORY F0):
- 1. 28 DAY COMPRESSIVE STRENGTH: 3000 PSI 1. 28 DAY COMPRESSIVE STRENGTH: 4500 PSI
- e. TOTAL AIR CONTENT FOR CONCRETE EXPOSED TO CYCLES OF FREEZING AND THAWING SHALL BE DETERMINED IN ACCORDANCE WITH THIS SCHEDULE. TOLERANCE ON AIR CONTENT AS DELIVERED SHALL BE +/- 1.5 PERCENT. NOMINAL MAXIMUM TARGET AIR CONTENT, PERCENT
- AGGREGATE SIZE, IN.
- 2. WATER USED IN MIXING CONCRETE SHALL CONFORM TO ASTM C1602. 3. NO CONDUIT, PIPES, DUCTS, SLEEVES, ETC. SHALL BE PLACED IN STRUCTURAL CONCRETE UNLESS SPECIFICALLY DETAILED OR APPROVED BY THE STRUCTURAL ENGINEER. NO ALUMINUM PRODUCTS SHALL BE EMBEDDED IN CONCRETE. PENETRATIONS THRU STRUCTURAL CONCRETE ELEMENTS MUST BE APPROVED BY THE ENGINEER AND SHALL BE BUILT INTO THE ELEMENT PRIOR TO CONCRETE
- 4. REFER TO ARCHITECTURAL DRAWINGS FOR MOLDS, GROOVES, ORNAMENTS, ETC. TO BE CAST IN TO CONCRETE, AND FOR EXTENT AND LOCATION OF DEPRESSIONS, CURBS, RAMPS, ETC. 5. UNLESS NOTED OTHERWISE, MINIMUM REINFORCING IN ALL CONCRETE FOUNDATION WALLS SHALL BE AS FOLLOWS:
- THICKNESS BOTTOM BARS VERTICAL HORIZONTA #4 AT 18"O.C. #4 AT 12"O.C. #4 AT 18"O.C. EACH FACE #4 AT 12"O.C. EACH FACE UNLESS NOTED OTHERWISE, CONCRETE SLABS ON EARTH SHALL BE UNREINFORCED AS FOLLOWS:
- WHERE REINFORCING IS PROVIDED REINFORCING SHALL BE CONTINUOUSLY SUPPORTED AT 36"O.C. MAXIMUM SPACING. 6. UNLESS NOTED OTHERWISE, FOR NON-DETAILED OPENINGS IN CONCRETE WALLS LARGER THAN 12" AND SMALLER THAN 24" IN ANY DIRECTION ADD (2) #5 BARS ON ALL SIDES IN ADDITION TO REGULAR WALL REINFORCING AND EXTEND 24" EACH WAY BEYOND OPENING. IF 24" IS NOT AVAILABLE ON EVERY SIDE, NOTIFY STRUCTURAL ENGINEER FOR FURTHER DIRECTION. OPENINGS SHALL HAVE A MINIMUM
- OF 12" OF CONCRETE ABOVE THE OPENING, TYP. 7. CONSTRUCTION JOINTS NOT SHOWN ON THE PLANS SHALL BE MADE AND LOCATED SO AS TO NOT IMPAIR THE STRENGTH OF THE STRUCTURE AND AS APPROVED BY THE STRUCTURAL ENGINEER. PROVIDE 2 X 4 (SHAPED) KEYWAY IN ALL VERTICAL AND HORIZONTAL JOINTS UNLESS NOTED OR DETAILED OTHERWISE. ALL STEEL REINFORCING SHALL BE CONTINUOUS THROUGH COLD JOINTS UNLESS NOTED OTHERWISE. SEE TYPICAL DETAILS FOR COLD/CONSTRUCTION JOINTS FOR SLABS ON
- 8. WHERE NEW CONCRETE IS PLACED AGAINST PREVIOUSLY HARDENED CONCRETE, THE JOINT SHALL BE CLEAN AND FREE OF LAITANCE. IMMEDIATELY BEFORE NEW CONCRETE IS PLACED, CONSTRUCTION JOINTS SHALL BE PREWETTED AND STANDING WATER REMOVED.

F. ANCHOR BOLTS/EMBEDDED BOLTS

- 1. ALL ANCHOR BOLTS SHALL HAVE ASTM A-563 HEAVY HEX NUT AND ASTM F-436 WASHERS AT STANDARD OR OVERSIZED HOLES PER AISC SPECIFICATION TABLE J3.3. WHERE HOLE SIZES DO NOT COMPLY WITH THE LIMITATIONS FOR OVERSIZED HOLES THE STRUCTURAL ENGINEER SHALL BE NOTIFIED TO DETERMINE STEEL PLATE WASHER REQUIREMENTS. ANCHOR BOLTS SHALL COMPLY a. AT ALL ANCHOR BOLTS (UNLESS NOTED OTHERWISE) - ASTM F1554 GRADE 36 HEADED BOLTS.
- (ASTM A36 THREADED ROD MAY BE USED WITH DOUBLE NUT AND WASHER.) 2. EMBEDDED BOLTS IN MASONRY SHALL BE (UNLESS NOTED OTHERWISE) ASTM A-307 GRADE HEADED
- 3. SEE TYPICAL ANCHOR BOLT DETAIL FOR DEFINITIONS OF EMBEDMENT LENGTH, ETC 4. FURNISH TEMPLATES AND OTHER DEVICES AS NECESSARY FOR PRESETTING ALL BOLTS PRIOR TO PLACING CONCRETE AND/OR GROUT. 5. IF THREADED RODS ARE USED AS PERMITTED ABOVE, THEY SHALL BE CLEAR OF SOIL AND DIRT.
- 6. WHERE REQUIRED FOR ERECTION, HOLES LARGER THAN OVERSIZED MAY BE PERMITTED WITH THE USE OF STEEL PLATE WASHERS AT THE DISCRETION OF THE STRUCTURAL ENGINEER.

G. ADHESIVE/MECHANICAL ANCHORS

- 1. WITHOUT WRITTEN APPROVAL OF THE ENGINEER, CONTRACTOR SHALL NOT SUBSTITUTE POST-INSTALLED ANCHORS WHERE CAST-IN-PLACE ANCHORS ARE SPECIFIED IN THE DRAWINGS. 2. WHERE STRUCTURAL DETAILS SPECIFY SPECIFIC BRANDS AND/OR TYPES OF ADHESIVES OR ANCHORS, SUBSTITUTIONS OF OTHER BRANDS AND/OR TYPES IS NOT ALLOWED, WITHOUT WRITTEN
- APPROVAL OF THE ENGINEER 3. SUBSTITUTION REQUESTS FOR ALTERNATE PRODUCTS SHALL BE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER OF RECORD PRIOR TO USE. SUBSTITUTION REQUESTS SHALL INCLUDE AN
- ICC ESR OR IAPMO REPORT AND SUPPORTING CALCULATIONS INDICATING COMPLIANCE WITH DESIGN 4. ALL ADHESIVE/MECHANICAL ANCHORS SHALL BE INSTALLED, INCLUDING HOLE DRILLING AND PREPARATION, IN ACCORDANCE WITH AN APPROVED INDEPENDENT EVALUATION REPORT (ICC-ES,

IAPMO, OR APPROVED EQUAL), AS INDICATED BELOW, AND IN ACCORDANCE WITH ALL

- MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII). 5. INSTALLERS SHALL BE, AT A MINIMUM, TRAINED FOR THE SPECIFIC APPLICATION INSTALLATION TECHNIQUE FOR THE SPECIFIC PRODUCT BY THE PRODUCT MANUFACTURERS FIELD EMPLOYEE OR SHALL POSSESS A TRAINING CARD OBTAINED BY THE MANUFACTURERS ONLINE TRAINING PROGRAM. 6. ADHESIVE ANCHORS SHALL BE INSTALLED IN CONCRETE HAVING A MINIMUM AGE OF 21 DAYS AT TIME
- OF ANCHOR INSTALLATION. ADHESIVE ANCHORS SHALL NOT BE FULLY LOADED UNTIL CONCRETE HAS 7. ADHESIVE ANCHORS SHALL CONSIST OF REINFORCING BAR OR THREADED RODS AS INDICATED IN THESE DOCUMENTS. 8. UNLESS APPROVED BY THE ENGINEER OF RECORD, CONCRETE AND DRILLED ANCHOR HOLES SHALL BE DRY AND FREE OF WATER FOR 14 DAYS PRIOR TO ADHESIVE INSTALLATION. CONTACT THE

ENGINEER OF RECORD FOR GUIDANCE IF THE CONTRACTOR CHOOSES TO INSTALL IN DAMP, WATER-

- SATURATED, OR WATER-FILLED HOLES. 9. CONCRETE TEMPERATURE AT THE TIME OF INSTALLATION SHALL BE MONITORED BY THE CONTRACTOR. CONTRACTOR SHALL COMPLY WITH ALL MANUFACTURER'S PRINTED INSTALLATION
- INSTRUCTIONS (MPII) RELATIVE TO SUBSTRATE TEMPERATURE. 10. INSTALLATION OF ADHESIVE ANCHORS HORIZONTALLY OR UPWARDLY INCLINED TO SUPPORT SUSTAINED TENSION LOADS SHALL BE PERFORMED BY PERSONNEL CERTIFIED BY AN APPLICABLE CERTIFICATION PROGRAM. CERTIFICATION SHALL INCLUDE WRITTEN AND PERFORMANCE TESTS IN ACCORDANCE WITH THE ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM. OR EQUIVALENT IN ACCORDANCE WITH ACI 318-19 26.7.2 (e) PROOF OF CURRENT CERTIFICATION SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO INSTALLATION. CONTINUOUS SPECIAL
- INSPECTION SHALL BE PROVIDED FOR THESE ANCHORS. 11. UNLESS NOTED OTHERWISE, ALL ADHESIVE ANCHORS INTO CONCRETE SHALL BE: a. HILTI HIT-RE 500V3 (ESR-3814), OR HILTI HIT-HY 200-V3 (ESR-4868).
- SIMPSON SET-3G (ESR-4057), OR AT-XP (ER-263). c. DEWALT PURE 110+ (ESR-3298), OR AC200+ GOLD (ESR-4027-COLD WEATHER). 12. UNLESS NOTED OTHERWISE, ALL ADHESIVE ANCHORS INTO GROUTED MASONRY (CMU) SHALL BE: a. HILTI HIT-HY 270 (ESR-4143).
- b. SIMPSON SET-XP (ER-265), OR AT-XP (ER-281). c. DEWALT AC100+ GOLD (ESR-3200). 13. UNLESS NOTED OTHER WISE, ALL MECHANICAL ANCHORS INTO CONCRETE SHALL BE:
- a. HILTI KWIK BOLT-TZ2 (ESR-4266). b. SIMPSON STRONG-BOLT 2 (ESR-3037). 14. UNLESS NOTED OTHERWISE, ALL MECHANICAL ANCHORS INTO GROUTED MASONRY (CMU) SHALL BE: a. HILTI KWIK BOLT-TZ2 (ESR-4561).
- b. SIMPSON STRONG BOLT 2 (ER-240). c. DEWALT SCREWBOLT+ (ESR-4042). 15. UNLESS NOTED OTHERWISE, ALL SCREW ANCHORS INTO CONCRETE SHALL BE:
- a. SIMPSON TITEN HD (ESR-2713). b. DEWALT SCREWBOLT+ (ESR-3889). c. HILTI KH-EZ (ESR-3027) 16. UNLESS NOTED OTHERWISE, ALL SCREW ANCHORS INTO GROUTED MASONRY (CMU) SHALL BE:
- a. SIMPSON TITEN HD (ESR-1056). b. DEWALT SCREWBOLT+ (ESR-1678). c. HILTI KH EZ (ESR-3056). 17. ALL MASONRY CELLS WITHIN 8" OF THE ANCHOR SHALL BE SOLID GROUTED. 18. THE TESTING LABORATORY WILL PERFORM VISUAL INSPECTION OF ANCHORS AND DOWELS AS
- REPORT. TENSION TESTING CAN BE REQUIRED AT THE DIRECTION OF THE STRUCTURAL ENGINEER OF RECORD OR THE SPECIAL INSPECTOR. 19. IF REINFORCEMENT IS ENCOUNTERED DURING DRILLING, ABANDON THAT HOLE AND SHIFT THE ANCHOR LOCATION TO AVOID THE REINFORCEMENT. PROVIDE A MINIMUM SPACE OF (2) ANCHOR HOLE DIAMETERS OR 2 INCHES, WHICH EVER IS LARGER, OF SOUND CONCRETE/MASONRY BETWEEN THE ANCHOR AND THE ABANDONED HOLE. FILL THE ABANDONED HOLE WITH NON-SHRINK GROUT OR AN APPROVED ANCHORING ADHESIVE. AT CONTRACTORS OPTION, LOCATE EXISTING REINFORCEMENT

MEMBERS, OR OTHER STEEL ASSEMBLIES ATTACHED WITH MECHANICAL ANCHORS.

PRIOR TO DRILLING/CORING. IF THE ANCHOR OR DOWEL CANNOT BE SHIFTED AS NOTED ABOVE, THE NGINEER WILL DETERMINE A NEW LOCATION. 20. LOCATE REINFORCEMENT AND CONFIRM FINAL ANCHOR LOCATIONS PRIOR TO FABRICATING PLATES

SPECIFIED IN THE SPECIAL INSPECTION SCHEDULE AND THE APPROVED INDEPENDENT EVALUATION

H. SUSPENDED CONCRETE SLABS / SLABS ON METAL DECK

- 1. UNLESS NOTED OTHERWISE, ALL CONCRETE SLABS ON METAL DECK SHALL BE 5 1/2" TOTAL THICKNESS NORMAL WEIGHT CONCRETE WITH A WEIGHT LESS THAN 145 POUNDS PER CUBIC FOOT, REINFORCED WITH 6 X 6 - W1.4 X W1.4 WELDED WIRE FABRIC. REINFORCING STEEL SHALL BE CHAIRED TO 1" TOP COVER AT ALL BEAM LOCATIONS. EXCEPT WHERE SPECIFICALLY DETAILED, FIBER MESH MAY BE USED IN PLACE OF REINFORCEMENT IN SLABS ON DECK WHEN USED IN ACCORDANCE WITH AN APPROVED ICC RESEARCH REPORT AND WHERE APPROVED BY THE ENGINEER. WHERE THE SLAB CONSTRUCTION IS USED TO OBTAIN A UL FIRE RATING, THE PROPOSED FIBER MESH SHALL HAVE UL ACCEPTANCE AS AN APPROVED ALTERNATIVE TO WELDED WIRE FABRIC. 2. AROUND OPENINGS IN SUSPENDED CONCRETE SLABS, ADD REINFORCING BARS EQUIVALENT TO BARS CUT BY OPENING WITH HALF ON EACH SIDE OF OPENING. BARS PARALLEL TO PRINCIPAL REINFORCING
- BEYOND OPENING. 3. SLAB PENETRATIONS LESS THAN 6" IN ALL DIRECTIONS WITH A CLEAR SPACING OF AT LEAST 3 TIMES THE LONGEST DIMENSION, DO NOT REQUIRE SUPPLEMENTAL REINFORCING. OTHERWISE, THE PENETRATIONS SHALL BE FRAMED ON 4 SIDES WITH STEEL ANGLES OR BENT PLATES (SEE TYPICAL DETAIL) UNLESS NOTED OTHERWISE.

SHALL RUN FULL LENGTH OF SPAN. BARS PARALLEL TO TEMPERATURE REINFORCING SHALL RUN 24"

- 4. EVERY EFFORT SHALL BE MADE TO PROVIDE A LEVEL FINISHED FLOOR WHILE MAINTAINING THE MINIMUM INDICATED SLAB THICKNESS. 5. CONTROL JOINTS IN SUSPENDED CONCRETE SLABS AND CONCRETE SLABS ON DECK SHALL NOT BE
- USED UNLESS SPECIFICALLY APPROVED AND DETAILED BY THE ENGINEER. 6. SEE TYPICAL DETAILS WHEN SLABS ARE MADE COMPOSITE WITH STEEL BEAMS. 7. NO CONDIUT IS ALLOWED IN CONCRETE SLABS ON METAL DECK. 8. WHERE CONDUIT IS CLUSTERED TOGETHER TO RISE ABOVE SLAB OR PENETRATE SLAB, PENETRATION
- IN SLAB MUST BE SUPPORTED AS NOTED IN NOTE H.3 ABOVE. 9. CONTRACTOR SHALL PROVIDE ALL TEMPORARY SHORING, BRACING, AND GUYING AS REQUIRED DURING ERECTION AND PLACEMENT OF SUSPENDED CONCRETE SLABS ON METAL DECK. 10. IN ALL SLABS NOT COVERED BY CARPET INSTALL #4 REINFORCING STEEL @ 12"O.C. EACH WAY.

I. REINFORCING STEEL

- 1. REINFORCING BAR STRENGTH REQUIREMENTS:
- a. ALL REINFORCING BARS SHALL CONFORM TO ASTM STANDARD A-615 GRADE 60 AND ALL WELDED WIRE FABRIC SHALL CONFORM TO ASTM STANDARD A-1064 AND SHALL BE SUPPLIED IN FLAT SHEETS. ADEQUATELY TIE AND SUPPORT ALL REINFORCING STEEL AS SPECIFIED BY ACI 117, TO MAINTAIN EXACT REQUIRED POSITION. HEADED SHEAR STUD ASSEMBLIES SHALL CONFORM TO ASTM A1044.
- 3. STEEL DISCONTINUOUS FIBER REINFORCEMENT SHALL BE DEFORMED AND CONFORM TO ASTM A820 AND SHALL HAVE A LENGTH TO DIAMETER RATIO NOT SMALLER THAN 50 AND NOT GREATER THAN 100. 4. HEADED DEFORMED BARS SHALL CONFORM TO ASTM A970. OBSTRUCTIONS OR INTERRUPTIONS OF THE BAR DEFORMATIONS, IF ANY, SHALL NOT EXTEND MORE THAN 2 BAR DIAMETERS FROM THE BEARING FACE OF THE HEAD.
- 5. ALL REINFORCING STEEL SHALL BE TIED IN PLACE AND ADEQUATELY SUPPORTED PRIOR TO PLACING CONCRETE. WET STABBING OF ANY REINFORCING STEEL IS NOT PERMITTED, UNLESS SPECIFICALLY DETAILED OTHERWISE OR APPROVED BY THE ENGINEER. 6. ALL FIELD BENT DOWELS SHALL BE GRADE 40 WITH SPACING INDICATED REDUCED BY 1/3. 7. UNLESS NOTED OTHERWISE, REINFORCEMENT SHALL HAVE THE FOLLOWING CONCRETE COVERAGE:
- b. EXPOSED TO EARTH OR WEATHER: 1. #6 & LARGER 2" 2. #5 & SMALLER1-1/2"
- c. NOT EXPOSED TO WEATHER OR EARTH . SLABS, WALLS, JOISTS, #11 & SMALLER 3/4"
- BEAMS, COLUMNS: MAIN REINFORCING OR TIES 1-1/2" d. SLAB ON GRADE :

a. CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH 3"

- 1. PLACE REINFORCING AT CENTER OF SLAB UNLESS INDICATED OTHERWISE. 8. EXCEPT WHERE NOTED ON PLANS OR DETAILS CONTINUOUS REINFORCEMENT SHALL BE SPLICED AT
- POINTS OF MINIMUM STRESS BY LAPPING PER THE REBAR LAP SCHEDULE. 9. REINFORCING STEEL MAY BE SPLICED WITH MECHANICAL COUPLERS THAT HAVE A TENSION CAPACITY OF AT LEAST 125% OF THE STRENGTH OF THE BAR. MECHANICAL COUPLERS SHALL BE A POSITIVE CONNECTING TYPE COUPLER, AND SHALL BE INSTALLED IN ACCORDANCE WITH AN APPROVED ICC RESEARCH REPORT. WHERE THESE ARE USED, SPLICES ON ADJACENT BARS SHALL BE STAGGERED AT LEAST 24 INCHES ALONG THE LENGTH OF THE BARS.
- 10. ALL VERTICAL REINFORCING IN STRUCTURAL ELEMENTS ABOVE SHALL BE SPLICED WITH MATCHING DOWELS EMBEDDED WITHIN THE FOOTINGS OR STRUCTURE BELOW. SPLICE LENGTHS SHALL COMPLY WITH REBAR LAP SCHEDULE. DOWELS INTO FOOTINGS SHALL TERMINATE WITH A STANDARD HOOK, AND SHALL EXTEND TO WITHIN 4" OF THE BOTTOM OF THE FOOTING, BUT NEED NOT EXTEND MORE THAN 20" INTO FOOTING. FOR MASONRY CONSTRUCTION SEE STRUCTURAL NOTE L.6.A. 11. DO NOT WELD REINFORCING EXCEPT AS NOTED ON PLANS, WHERE REINFORCING IS WELDED, USE
- ASTM A-706 REINFORCING. 12. REINFORCING BARS, TIES, AND TENDONS SHALL BE SUPPORTED BY NYLON CONES, PLASTIC-COATED TIE-WIRES, OR PLASTIC-COATED CHAIRS. REINFORCING IN FOOTINGS IS PERMITTED TO BE SUPPORTED 13. UNLESS NOTED OTHERWISE, HOOKS, STIRRUPS, TIES, AND OTHER BENDS IN REINFORCING STEEL SHALL MEET THE STANDARDS SET FORTH IN ACI 318/318R-19. UNLESS OTHERWISE PERMITTED BY THE ENGINEER, ALL REINFORCEMENT SHALL BE BENT COLD. REINFORCEMENT PARTIALLY EMBEDDED IN
- PERMITTED BY THE ENGINEER. 14. UNLESS SPECIFICALLY NOTED AND/OR DETAILED IN THE STRUCTURAL DRAWINGS CONDUIT SHALL NOT BE IN CONTACT WITH REINFORCING STEEL.

CONCRETE SHALL NOT BE FIELD BENT, EXCEPT AS SHOWN ON THESE DRAWINGS OR OTHERWISE

(STRUCTURAL NOTES CONTINUED ON SHEET S-002)

LEGEND OF SYMBO	LS AND ABBI	REVIATIONS
AB = ANCHOR BOLT		— FOOTING MARK
ABV = ABOVE ARCH = ARCHITECT	•	— TOP OF FOOTING ELEVATION
BLW = BELOW BN = BOUNDARY NAILING	•	— SECTION MARK
BS = BOUNDARY SCREW BRB = BUCKLING RESTRAINED BRACE		— SHEET NUMBER
BRBF = BUCKLING RESTRAINED BRACE FRAME CJP = COMPLETE JOINT PENETRATION CL = CENTERLINE	•	TOP OF FOUNDATION WALL OR COLUMN PIER ELEVATION
CMU = CONCRETE MASONRY UNIT	•	— SHEAR WALL - SEE SCHEDULE
COL = COLUMN CONC = CONCRETE	•	— MIN. LENGTH OF SHEAR WALL
CP = CONCRETE PIER DC = DEMAND CRITICAL	ss	— FOOTING STEP
$DIA / \emptyset = DIAMETER$ $DBA = DEFORMED BAR ANCHOR$		— MASONRY WALL
DBE = DECK BEARING ELEVATION		MAGGINIT WALL
ELEV = ELEVATION EN = EDGE NAILING EOD = EDGE OF DECK FDN = FOUNDATION		 DEPRESS FDN./WALL AND POUR FLOOR SLAB OVER AT MASONRY FOUNDATION WALL
FTG = FOOTING FFE = FINISHED FLOOR ELEVATION GB = CONCRETE GRADE BEAM HSA = HEADED STUD ANCHOR	4 7 7	— DEPRESS FDN./WALL AND POUR FLOOR SLAB OVER AT CONCRETE FOUNDATION WALL
JBE = JOIST BEARING ELEVATION KB = KICKER BRACE		— MASONRY BEAM
MAX = MAXIMUM		
MB = MASONRY BEAM MC = MASONRY COLUMN	4	— CONCRETE BEAM
ECH = MECHANICAL EZZ = MEZZANINE	1.P	HD - SIMPSON HOLDOWN SIZE
IN = MINIMUM	NO ST	POST - SIZE OF END POST CONNECTED TO HOLDOWN
MJ = MASONRY JAMB MW = MASONRY WALL	× 20°	"A" - PLAN CONFIGURATION AT
NS, FS = NEAR SIDE, FAR SIDE DAE = OR APPROVED EQUAL		HOLDOWN AT FOUNDATION
OPP = OPPOSITE	-	
PAF = POWDER ACTUATED FASTENER PL = PLATE	—— <u>L</u> ——	FRAMING ANGLE SEE TYPICAL DET
REINF = REINFORCING REQ'D = REQUIRED SIM = SIMILAR	——с—	FRAMING CHANNEL SEE TYPICAL DETAIL
SSH = STEEL STUD HEADER SSJ = STEEL STUD JAMB SSS = STEEL STUD SILL SSW = STEEL STUD WALL	(L)	ITEMS, DETAILS, & SYSTEMS WHICH ARE PART OF THE LATERAL FORCE RESISTING SYSTEM.
TOB = TOP OF BEAM ELEVATION TOC = TOP OF CONCRETE SLAB		BRACED FRAME
TOF = TOP OF CONCRETE SLAB TOF = TOP OF FOOTING TOG = TOP OF GIRDER ELEVATION TOM = TOP OF MASONRY	→	MOMENT RESISTING CONNECTION SEE DETAIL
TOS = TOP OF STEEL ELEVATION TYP = TYPICAL UNO = UNLESS NOTED OTHERWISE	$\longrightarrow \Diamond \triangleright$	MOMENT RESISTING CANTILEVER CONNECTIONS - SEE DETAIL
	KB	KICKER BRACE

Structural Sheet Index										
SHEET NUMBER	SHEET NAME									
S-001	STRUCTURAL NOTES									
S-002										
S-010	SCHEDULES									
S-011	SCHEDULES									
S-012	SCHEDULES									
S-013	SCHEDULES									
S-110	FOOTING AND FOUNDATION PLAN - OVERALL									
S-111	FOOTING AND FOUNDATION PLAN - AREA A									
S-112	FOOTING AND FOUNDATION PLAN - AREA B									
S-113	FOOTING AND FOUNDATION PLAN - AREA C									
S-114	FOOTING AND FOUNDATION PLAN - AREA D									
S-115	FOOTING AND FOUNDATION PLAN - AREA E									
S-116	FOOTING AND FOUNDATION PLAN - AREA F									
S-120	FLOOR FRAMING PLAN - OVERALL									
S-121	FLOOR FRAMING PLAN - AREA A									
S-122	FLOOR FRAMING PLAN - AREA B									
S-123	FLOOR FRAMING PLAN - AREA C									
S-124	FLOOR FRAMING PLAN - AREA F									
S-130	ROOF FRAMING PLAN - OVERALL									
S-131	ROOF FRAMING PLAN - AREA A									
S-132	ROOF FRAMING PLAN - AREA B									
S-133	ROOF FRAMING PLAN - AREA C									
S-134	ROOF FRAMING PLAN - AREA D									
S-135	ROOF FRAMING PLAN - AREA E									
S-136	ROOF FRAMING PLAN - AREA F									
S-140	SPORTS STORAGE FOOTING, FDN, AND ROOF FRAMING PLAN									
S-201	TYPICAL DETAILS									
S-202	TYPICAL DETAILS									
S-210	FOOTING & FOUNDATION DETAILS									
S-211	FOOTING & FOUNDATION DETAILS									
S-220	FLOOR FRAMING DETAILS									
S-230	ROOF FRAMING DETAILS									
S-231	ROOF FRAMING DETAILS									
S-301	ELEVATIONS									
S-401	SCHEMATIC REFERENCE									

K. OPEN WEB JOISTS AND GIRDERS

- 1. ALL OPEN WEB STEEL JOISTS AND GIRDERS SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF "STANDARD SPECIFICATIONS AND CODE OF STANDARD PRACTICE" OF
- 2. (###/###) DENOTES APPLIED TOTAL AND LIVE UNIFORMLY DISTRIBUTED LOADS IN POUNDS PER LINEAR FOOT OF JOIST, RESPECTIVELY.
- 3. SEE JOIST LOAD PROFILES FOR SPECIALLY LOADED JOISTS. 4. CONCENTRATED POINT LOADS, NOT SPECIFICALLY SHOWN ON THE PLANS, OF LESS THAN 100 POUNDS FOR MECHANICAL UNITS. FIRE SPRINKLER MAINS, AND OTHER EQUIPMENT SHALL BE ALLOWED WITHOUT REQUIRING ADDITIONAL WEB MEMBERS TO BE INSTALLED. WHERE THE LOAD EXCEEDS 100
- POUNDS, THE LOAD SHALL BE SUPPORTED WITHIN 6" OF A CHORD PANEL POINT. SUPPORT BEYOND 6" FROM PANEL POINTS CAN BE PROVIDED BY ADDING (2) L2 x 2 x 1/4 DIAGONALS TO THE NEAREST OPPOSITE CHORD PANEL POINT PER THE TYPICAL DETAIL. CONNECTIONS SHALL BE MADE CONCENTRIC TO THE CHORD ANGLES. BEAM CLAMPS, OR SIMILAR ECCENTRIC ATTACHMENTS, ARE NOT ALLOWED, EXCEPT AS INDICATED BELOW. BEAM CLAMPS, OR SIMILAR ATTACHMENTS THAT ARE NOT CENTERED ON THE CHORD ANGLES MAY ONLY BE USED FOR LOADS LESS THAN 10 POUNDS. SEE JOIST SUBMITTAL FOR ADDITIONAL REQUIREMENTS. ALL LOADS PROVIDED FOR IN THIS NOTE SHALL BE ACCOUNTED FOR IN THE SPECIFIED DESIGN LOADS.
- 5. ANY BRACING REQUIRED FOR MISCELLANEOUS ITEMS (I.E. DUCTWORK, PIPING, ETC.) MUST CONNECT TO THE TOP CHORD OF THE JOIST OR GIRDER. BRACING TO THE BOTTOM CHORD IS NOT ALLOWED
- UNLESS SPECIFICALLY DETAILED THAT WAY ON THE PLANS. 6. PROVIDE SPECIAL BEARING ENDS AS REQUIRED AT SLOPED BEARING CONDITIONS. CONTRACTOR
- SHALL COORDINATE WITH OTHER STRUCTURAL ELEMENTS. ALL JOISTS SHALL BE CAMBERED PER SJI SPECIFICATIONS, UNLESS NOTED OTHERWISE.
- 8. FIELD MODIFICATIONS (INCLUDING HOLES IN THE CHORD OR WEB MEMBERS) SHALL NOT BE MADE TO ANY JOIST OR GIRDER WITHOUT PRIOR APPROVAL BY THE MANUFACTURER. 9. FABRICATORS AND SUPPLIERS SHALL COORDINATE PAINT/FINISHES WITH REQUIREMENTS FOR DIRECT APPLIED INSULATION, FIREPROOFING, ETC. AS NOTED IN THE PROJECT SPECIFICATIONS.
- 10. JOIST BRIDGING SHALL BE PROVIDED AS REQUIRED BY THE JOIST MANUFACTURER AND SJI STANDARDS. BRIDGING WHERE SHOWN ON THE STRUCTURAL DRAWINGS IS A SCHEMATIC
- REPRESENTATION ONLY. SEE JOIST MANUFACTURER FOR BRIDGING SIZE, CONNECTIONS, TYPE AND 11. WHERE ADDED LOADS ARE SHOWN ON THE JOISTS BUT NOT SPECIFICALLY DIMENSIONED. THE JOIST
- DESIGNER SHALL PLACE THOSE LOADS ON THE JOIST AT A LOCATION THAT RESULTS IN THE HIGHEST STRESS IN THE MEMBERS. THE DESIGNER MAY ASSUME THAT THE LOAD OCCURS WITHIN 10 FEET OF A SCALED DIMENSION.
- 12. FABRICATOR MUST SUBMIT A CERTIFICATE OF COMPLIANCE TO THE BUILDING OFFICIAL PER IBC 2207.5 STATING THAT WORK WAS PERFORMED IN ACCORDANCE WITH APPROVED CONSTRUCTION DOCUMENTS AND WITH SJI SPECIFICATIONS. 13. UNLESS NOTED OTHERWISE, ROOF JOISTS AND GIRDERS SHALL BE DESIGNED FOR A NET WIND UPLIFT
- 14. ALL ROOF JOISTS BEARING ON EXTERIOR WALLS SHALL BE DESIGNED TO TRANSFER 3.75 KIPS (ULTIMATE) TOP CHORD AXIAL FORCE THROUGH THE BEARING SHOE.
- 15. ALL JOISTS AT GRID LINES SHALL BE DESIGNED TO RESIST A 10 KIP TOP CHORD AXIAL FORCE
- 16. ALL STANDARD AND NON STANDARD SJI JOISTS SHALL BE DESIGNED FOR THE FOLLOWING DEFLECTION CRITERIA: a. LIVE LOAD: L/240
- 17. JOIST MANUFACTURER SHALL APPLY ADDITIONAL POINT OR LINE LOAD AS REQUIRED TO SUPPORT FIRE PROTECTION MAINLINES 4" DIAMETER OR GREATER. JOIST MANUFACTURER SHALL COORDINATE WITH GENERAL CONTRACTOR TO OBTAIN LOCATIONS AND WEIGHTS OF THESE LINES. SEISMIC BRACING LOADS FOR FIRE PROTECTION MAINLINES SHALL ALSO BE ACCOUNTED FOR IN THE JOIST MANUFACTURER'S DESIGN.

L. MASONRY

- 1. ALL HOLLOW MASONRY UNITS SHALL CONFORM TO ASTM C-90.
- f'm (MINIMUM, FACTORED) 2,000 PSI MINIMUM UNIT STRENGTH 2,000 PSI (TESTED IN ACCORDANCE WITH ASTM C-140) ACCEPTABLE RANGE OF UNIT WEIGHT: 105 PCF TO 125 PCF
- 2. ALL GROUT (SITE MIXED OR PRE-MIXED) SHALL CONFORM TO ASTM C-476 OR SECTION 2.2A OF TMS 602-16. GROUT SHALL BE PLACED WITH SUFFICIENT WATER FOR POURING WITHOUT SEGREGATION. DO
- NOT USE MORTAR FOR GROUT. MECHANICALLY VIBRATE ALL GROUT. . GROUT STOPS SHALL BE AN APPROVED PRODUCT DESIGNED AND MANUFACTURED FOR USE AS A GROUT STOP. GROUT STOP SUBMITTALS SHALL BE SUBMITTED TO THE ARCHITECT AND ENGINEER
- FOR REVIEW. OTHER GROUT STOP MATERIALS SUCH AS ASPHALT IMPREGNATED MATERIALS ARE NOT
- MORTAR SHALL BE TYPE S AND SHALL CONFORM TO ASTM C 270. ALL MASONRY WORK SHALL CONFORM TO CHAPTER 21 OF THE IBC.
- . UNLESS NOTED OTHERWISE, MINIMUM REINFORCING IN ALL 8" MASONRY WALLS SHALL BE AS FOLLOWS
- a. VERTICAL: #5 BARS IN GROUTED CELLS ADJACENT TO ALL OPENINGS GREATER THAN 24 INCHES WIDE, ON EACH SIDE OF CONTROL JOINTS, AT ENDS OF WALL, AND AT A MAXIMUM SPACING OF 32" THROUGHOUT THE WALL. AT CORNERS, PROVIDE A MINIMUM OF (4) VERTICAL BARS AT "T" JOINTS AND (3) VERTICAL BARS AT "L" JOINTS. SEE THE TYPICAL DETAIL / SCHEDULE FOR MORE INFORMATION. ALL VERTICAL REINFORCEMENT SHALL BE DOWELED INTO THE FOUNDATION WALL UNLESS SPECIFICALLY DETAILED OTHERWISE. b. HORIZONTAL: (2) #4 BARS IN 8" DEEP "H" BLOCK BOND BEAM UNITS AT 48"O.C. AND AT FLOORS,
- ROOF, BELOW OPENINGS, AND TOP OF WALL. BOND BEAMS AT ROOF SHALL SLOPE TO MATCH SLOPING ROOF. SEE THE MASONRY REINFORCING SCHEDULE FOR MASONRY BEAMS ABOVE
- 7. SEE THE MASONRY REINFORCING SCHEDULE FOR OPENINGS WHICH EXCEED 32 INCHES IN EITHER
- 8. ALL BLOCK CELLS CONTAINING REINFORCING, BOLTS, OR ANCHORS SHALL BE GROUTED SOLID. 9. PROVIDE (1) #5 (MINIMUM), IN GROUTED SPACE. ON ALL SIDES AND ADJACENT TO EVERY OPENING WHICH EXCEEDS 24" IN EITHER DIRECTION. HORIZONTAL BARS SHALL EXTEND 24" BEYOND THE CORNERS OF THE OPENING AND VERTICAL BARS SHALL EXTEND TO TOP OF WALL. VERTICAL
- REINFORCING SHALL BE PROVIDED AT ENDS, CORNERS AND EACH SIDE OF CONTROL JOINTS. SEE TYPICAL DETAILS FOR OPENINGS WHICH EXCEED 32" IN EITHER DIRECTION. 10. SOLID GROUTING OF MASONRY IS UNACCEPTABLE EXCEPT AS SPECIFICALLY NOTED ON PLANS AND
- SCHEDULES. 11. WHERE WALLS ARE NOT GROUTED SOLID, EACH GROUT POUR SHALL TERMINATE FLUSH WITH THE TOP OF THE UPPERMOST UNIT EXCEPT AT CELLS WITH VERTICAL REINFORCING WHERE GROUT SHALL BE 1-1/2" BELOW TOP OF UNIT TO PROVIDE CONSTRUCTION KEY. WHERE WALLS ARE GROUTED SOLID, EACH GROUT POUR SHALL TERMINATE 1-1/2" BELOW TOP OF UNIT.
- 12. GROUT POURS SHALL NOT EXCEED 5'-0" UNLESS HIGH LIFT GROUTING PROCEDURES ARE FOLLOWED. 13. THE USE OF HIGH LIFT GROUTING PROCEDURES REQUIRE THE APPROVAL OF THE ARCHITECT AND ENGINEER AND SHALL NOT EXCEED THE MAXIMUM HEIGHTS GIVEN IN TABLE 3.2.1 OF TMS 402-16. GROUT DEMONSTRATION PANELS, AS PRESCRIBED BY THE ARCHITECT AND ENGINEER, SHALL BE REQUIRED WHERE REQUESTED GROUTING PROCEDURES DO NOT MEET THE LIMITS OF TABLE 3.2.1. ADDITIONALLY, ALL HIGH LIFT GROUTING SHALL REQUIRE SPECIAL INSPECTION PROCEDURES NEEDED TO VERIFY GROUT PLACEMENT DURING CONSTRUCTION. DURING THE SUBMITTAL FOR APPROVAL PROCESS, SUBMITTAL SHALL INCLUDE, BUT NOT BE LIMITED TO: STATEMENT OF PROCEDURE FOR MECHANICAL VIBRATION OF HIGH LIFT GROUT; NEW MIX DESIGNS FOR HIGH SLUMP, HIGH LIFT GROUT; FOR SELF-CONSOLIDATING GROUT, SUBMIT MIX DESIGNS, SLUMP FLOW RATES, VISUAL STABILITY
- INDEX (VSI), AND QUANTITIES OF ADMIXTURES BEING USED. 14. ALL MASONRY BEAMS SHALL BE BUILT INTEGRAL WITH SUPPORT. NO TOOTHING OR DOWELING PERMITTED. UNITS WITH ONE END OPEN SHALL BE USED FOR ALL MASONRY BEAMS.
- 15. PROVIDE VERTICAL CONTROL JOINTS AT MAXIMUM SPACINGS NOTED BELOW UNLESS NOTED OTHERWISE IN THE SPECIFICATIONS AND/OR ON ARCHITECTURAL ELEVATIONS AND AT ALL CHANGES IN WALL ELEVATION AND MASONRY THICKNESS. CONTROL JOINTS SHALL NOT BE LOCATED DIRECTLY OVER OR CLOSER THAN 24" TO WALL OPENINGS (DOORS, WINDOWS, MECHANICAL OPENINGS, ETC.), OR WITHIN MASONRY JAMBS.
- REINFORCED MASONRY: 40 FT VENEER: 30 FT AND AT INTERFACE BETWEEN VENEER SUPPORTED BY FOUNDATIONS AND SUSPENDED STRUCTURAL ELEMENTS.
- 16. HORIZONTAL REINFORCEMENT SHALL TERMINATE AT EACH SIDE OF CONTROL JOINTS EXCEPT AT FLOOR AND ROOF LEVEL BOND BEAMS AND AT TOP OF PARAPET. 17. CONTROL JOINTS SHALL BE PROVIDED AT THE MASONRY SIDE OF EMBEDDED STEEL COLUMNS TO
- CONTROL CRACKING OF FACE SHELLS. 18. SUPPORT NON-BEARING, NON-STRUCTURAL WALLS AT TOP OF MASONRY AS PER TYPICAL DETAILS AT
- LOCATIONS WHERE INTERSECTING OR PERPENDICULAR WALLS ARE 12'-0" OR MORE APART OR WHERE END OF WALL OCCURS 6'-0" OR MORE FROM INTERSECTING WALL
- 19. EMBED CHANNELS AND PLATES TO BE PLACED SO AS TO CREATE FLUSH SURFACE WITH FACE OF MASONRY. FLANGES ON CHANNEL EMBEDS SHALL BE HORIZONTAL. 20. ALL VERTICAL REINFORCING SHALL BE SECURED IN PLACE PRIOR TO GROUTING USING WIRE
- POSITIONERS OR OTHER ACCEPTABLE DEVICES. REINFORCING SHALL BE SECURED AT BAR-SPLICE LOCATIONS AND AT A SPACING NOT MORE THAN 120 BAR DIAMETERS. 21. UNLESS NOTED OTHERWISE, MASONRY WALLS SHALL BE CONSTRUCTED UTILIZING COMMON
- RUNNING-BOND WITH FULLY MORTARED BED JOINTS AROUND GROUTED CELLS.\ 22. MASONRY VENEER SHALL BE ANCHORED USING THE HOHMANN AND BARNARD VENEER ANCHOR ASSEMBLY SYSTEM, OR AN APPROVED EQUAL. REGARDLESS OF BACK-UP SYSTEM, PROVIDE A CONTINUOUS HORIZONTAL 9 GAUGE WIRE AT 16"O.C. IN VENEER MORTAR JOINTS FOR ANCHOR ATTACHMENT, POSITIVE ANCHORAGE TO THE WIRE USING THE SEISMICLIP INTERLOCK SYSTEM SHALL
- BE PROVIDED TO SUPPORT NOT MORE THAN 2 SQUARE FEET OF WALL, WITH A HORIZONTAL SPACING NOT EXCEEDING 18". a. METAL STUDS; USE HOHMANN AND BARNARD HB-213 S.I.S. (SEISMICLIP INTERLOCK SYSTEM) HEAVY DUTY ANCHORS OR AN APPROVED EQUAL. THE HB-213 ASSEMBLY SHALL BE ATTACHED TO WOOD
- STUDS USING A # 12 X 2" WOOD SCREWS OR TO METAL STUDS USING #10 SCREWS. BRICK AND BLOCK WALLS; USE HOHMANN AND BARNARD 270-ML-S.I.S. (SEISMICLIP INTERLOCK SYSTEM) MIGHTY-LOK SEISMIC ANCHORS OR AN APPROVED EQUAL, AT SPACINGS NOTED ABOVE. INSTALL A 2 WIRE 9 GAUGE LADDER TYPE JOINT REINFORCEMENT AT 16"O.C. IN THE BACK-UP WALL
- FOR ANCHORAGE ATTACHMENT c. CONCRETE WALLS; USE HOHMANN AND BARNARD HB 303SV SEISMIC NOTCH DOVE TAIL ANCHOR SYSTEM OR AN APPROVED EQUAL AT SPACINGS NOTED ABOVE
- **23.** ELECTRICAL CONDUIT SHALL NOT BE PLACED IN CELLS THAT CONTAIN REBAR. CONDUIT IS ALLOWED TO PASS THROUGH REINFORCED CELLS WHEN IT OCCURS PERPENDICULAR TO THE REBAR. CONDUIT SHALL NOT CONTACT REBAR AS IT PASSES. THERE SHALL BE 1" CLEAR BETWEEN CONDUIT AND

M. STRUCTURAL DELEGATED DESIGNS AND DEFERRED SUBMITTALS

- 1. STRUCTURAL DELEGATED DESIGNS AND SUBSEQUENT DEFERRED SUBMITTALS ARE FOR ELEMENTS, PARTS, OR PORTIONS OF THE OVERALL STRUCTURAL SYSTEM THAT ARE INDICATED OR REFERRED TO ON THESE DRAWINGS AND THAT ARE CRITICAL TO THE PERFORMANCE OF THE OVERALL STRUCTURAL SYSTEM. DESIGN CRITERIA HAS BEEN PROVIDED FOR THESE ITEMS IN THE STRUCTURAL NOTES,
- 2. STRUCTURAL DEFERRED SUBMITTALS ARE COMPLETE PACKAGES TO BE SUBMITTED FOR REVIEW THAT INCLUDE DRAWINGS AND CALCULATIONS FOR ALL DELEGATED DESIGN ITEMS AND THEIR CONNECTIONS. DEFERRED SUBMITTALS SHALL BEAR THE STAMP AND SIGNATURE OF THE DESIGN PROFESSIONAL RESPONSIBLE FOR THEIR DESIGN.

3. ARW ENGINEERS WILL REVIEW STRUCTURAL DEFERRED SUBMITTALS TO VERIFY DESIGN CRITERIA IS

4. STRUCTURAL DELEGATED DESIGN COMPONENTS SHALL NOT BE INSTALLED UNTIL APPROVED BY THE BUILDING OFFICIAL. 5. STRUCTURAL DELEGATED DESIGN ITEMS REQUIRING DEFERRED SUBMITTALS INCLUDE, BUT ARE NOT

a. OPEN WEB JOISTS & GIRDERS, BRIDGING, BRACING, CONNECTIONS, AND RELATED COMPONENTS.

N. NON-STRUCTURAL DELEGATED DESIGNS AND DEFERRED SUBMITTALS

COMPLIANT WITH THE APPROVED CONSTRUCTION DOCUMENTS.

 NON-STRUCTURAL DELEGATED DESIGNS AND SUBSEQUENT DEFERRED SUBMITTALS ARE FOR ITEMS NOT INCLUDED IN THE STRUCTURAL DELEGATED DESIGN SECTION. THESE ARE ITEMS THAT ARE NOT CRITICAL TO THE OVERALL PERFORMANCE OF THE STRUCTURAL SYSTEM BUT THAT IMPART LOADS AND FORCES TO THE STRUCTURAL SYSTEM.

4. IF THE STRUCTURAL DRAWINGS INCLUDE LOADS TO ACCOMMODATE NON-STRUCTURAL ELEMENTS.

2. NON-STRUCTURAL DEFERRED SUBMITTALS SHALL BEAR THE STAMP AND SIGNATURE OF THE DESIGN PROFESSIONAL RESPONSIBLE FOR THE DESIGN. 3. ARW ENGINEERS WILL REVIEW NON-STRUCTURAL DEFERRED SUBMITTALS TO VERIFY DESIGN

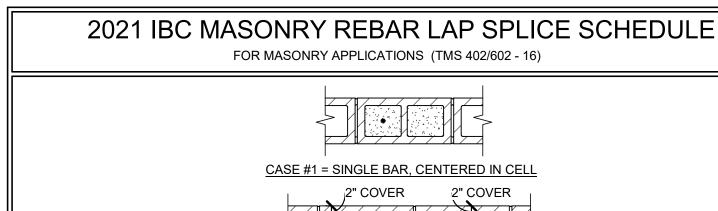
CRITERIA IS COMPLIANT WITH THE APPROVED CONSTRUCTION DOCUMENTS.

- THE CONTRACTOR SHALL SUBMIT DOCUMENTATION INDICATING THAT THE NON-STRUCTURAL ELEMENTS COMPLY WITH THE LOADING CRITERIA PROVIDED HEREIN. SUCH DOCUMENTATION SHALL BEAR THE STAMP AND SIGNATURE OF THE DESIGN PROFESSIONAL RESPONSIBLE FOR THE DESIGN. 5. WHEN THE NON-STRUCTURAL DEFERRED SUBMITTAL INDICATES THAT THE ELEMENT WILL IMPART FORCES IN EXCESS OF LOADS THAT ARE INDICATED ON THE STRUCTURAL DRAWINGS, THE CONTRACTOR SHALL SUBMIT A DETAILED GRAPHICAL REPRESENTATION OF THOSE DESIGN LOADS.
- INCLUDING MAGNITUDE, AND LOCATION. THE GRAPHIC SHALL BE ACCOMPANIED BY DOCUMENTATION INDICATING THAT THE NON-STRUCTURAL ELEMENT DESIGN COMPLIES WITH THE LOADING CRITERIA PROVIDED HEREIN. THE LETTER SHALL BEAR THE STAMP AND SIGNATURE OF THE DESIGN PROFESSIONAL RESPONSIBLE FOR THE DESIGN. 6. NON-STRUCTURAL DELEGATED DESIGN ITEMS REQUIRING DEFERRED SUBMITTALS SHALL INCLUDE,
- BUT ARE NOT LIMITED TO: a. COLD FORMED STEEL STUDS / JOISTS / HEADERS / JAMBS / TRUSSES. b. SEISMIC BRACING OF ALL ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL ITEMS WHERE REQUIRED BY THE MOST RECENT VERSION OF ASCE 7 AND THE PROJECT CONTRACT
- DOCUMENTS. c. STRUCTURAL STEEL STAIRS.

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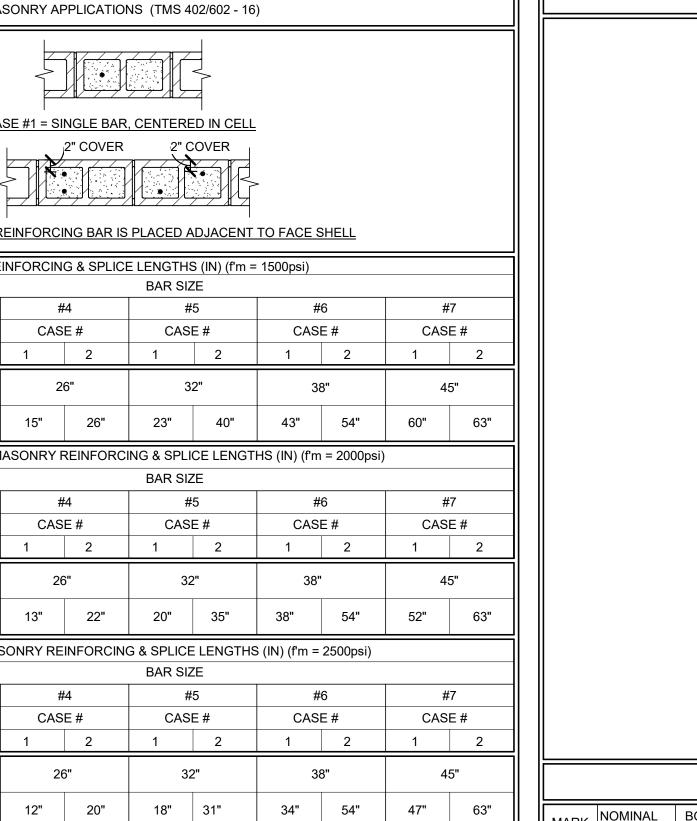


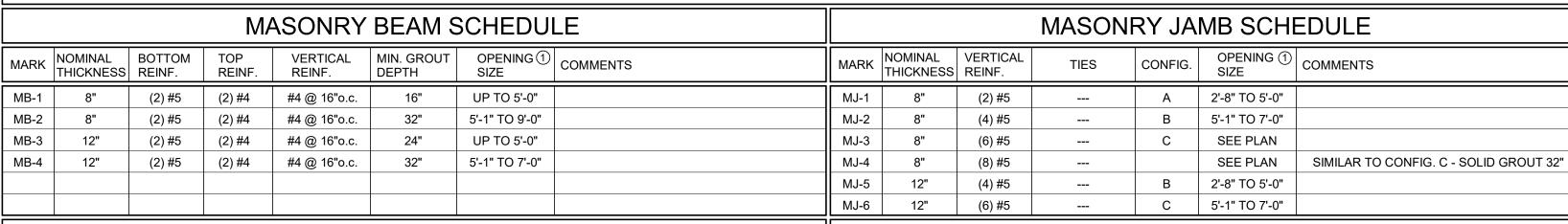
CASE #2 = WHEN REINFORCING BAR IS PLACED ADJACENT TO FACE SHELL

	GAGE WE THIER TREATMENT OF TOTAL OF THE TOTAL OF THE OF TH											
	MASONRY REINFORCING & SPLICE LENGTHS (IN) (f'm = 1500psi)											
	BAR SIZE											
BAR LOCATION	#	43	#4		#5		#6		#7			
	CASE#		CASE#		CASE#		CASE#		CASE#			
	1	2	1	2	1	2	1	2	1	2		
BEAM / WALL HORIZONTAL	19"		26"		32"		38"		45"			
WALL VERTICAL COLUMN AND JAMB	12" 15"		15"	26"	23"	40"	43"	54"	60"	63"		
	MASONRY REINFORCING & SPLICE LENGTHS (IN) (f'm = 2000psi)											
				BAR SIZE								
BAR LOCATION	#	43	#	‡ 4	#	5	#	6	#	7		
	CAS	E#	CAS	E#	CAS	E#	CAS	E#	CASI	E#		
	1	2	1	2	1	2	1	2	1	2		
BEAM / WALL HORIZONTAL	1	9"	26	6"	32	2"	38'		45	 5"		

		_							1		
	CASE#		CASE#		CASE#		CASE#		CASE#		
	1	2	1	2	1	2	1	2	1		
BEAM / WALL HORIZONTAL	19	9"	20	6"	32	2"	38'	1	4	5"	
WALL VERTICAL COLUMN AND JAMB	12"	13"	13"	22"	20"	35"	38"	54"	52"	6	
	MASONRY REINFORCING & SPLICE LENGTHS (IN) (fm = 2500psi)										
					BAR SIZE						
BAR LOCATION	#3 CASE #		#4		#5		#6		#7		
			CAS	CASE#		CASE#		CASE#		CASE#	
	1	2	1	2	1	2	1	2	1		
BEAM / WALL HORIZONTAL	1	9"	20	6"	32	2"	3	8"	4	5"	

MECHANICAL COUPLERS MAY BE USED IN LIEU OF LAP SPLICES SHOWN. SEE STRUCTURAL NOTES FOR MINIMUM COUPLER CAPACITY. WHERE MECHANICAL COUPLERS ARE USED, STAGGER ADJACENT SPLICES A MINIMUM OF 24" DEVELOPMENT LENGTHS SHALL BE INCREASED BY 50% WHERE EPOXY COATED REBAR IS USED. WHEN SPLICING BARS OF DIFFERENT SIZES, USE LAP SPLICE LENGTH OF LARGER BARS UNO.





- TYPICAL WALL BOND BEAM REINFORCING WHERE BOND BEAM LAYOUT DOES NOT

COINCIDE WITH BOTTOM OF OPENING

TYPICAL HORIZONTAL BOND BEAMS MAY BE ADJUSTED UP OR DOWN BY ONE COURSE PROVIDED THE OVERALL NUMBER OF REQUIRED BOND BEAMS ARE INSTALLED.

3. TYPICAL HORIZONTAL AND VERTICAL WALL REINFORCING NOT SHOWN FOR CLARITY, SEE PLAN AND SCHEDULE FOR TYPICAL WALL REINFORCING.

ALL BRACE FRAME AND MOMENT FRAME COLUMN FOOTINGS

TYP. FOOTING SECTION

W/ TOP & BOTTOM REINF

4. JAMB REINFORCING SHOWN IS SCHEMATIC. SEE SCHEDULE & DETAILS FOR ACTUAL JAMB REINFORCING. 5. ALL VERTICAL WALL REINFORCING SHALL BE CONTINUOUS BETWEEN THE LEVELS IN WHICH THE WALL OCCURS.

TYPICAL MASONRY / JAMB REINFORCING SCHEDULE

SLOPE BOND BEAM WHERE

DOWELS TO MATCH BOND

BEAM REINFORCING. SEE

OPENING SIZE SEE ARCH. PLANS

VERTICAL REINFORCING -

OCCURS - SEE ARCH. -

SCHEDULE FOR LAP

JAMB STEEL TO

EXTEND TO

BOND BEAM

BOND BEAM REQ'D @ ROOF

ELEVATION STEP AS REQ'D

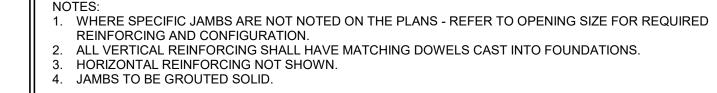
TO MATCH SLOPING ROOF

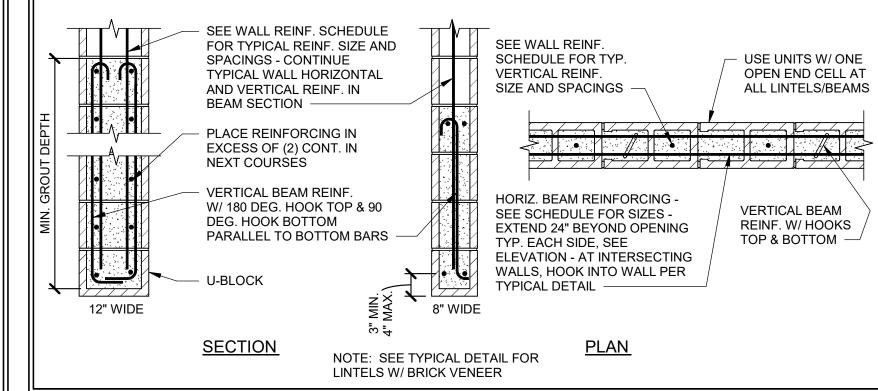
ROOF/FLOOR -

AT PARAPET -

1. WHERE SPECIFIC BEAMS ARE NOT NOTED ON THE PLANS - REFER TO OPENING SIZE FOR REQUIRED BEAM 2. FIRST VERTICAL BAR TO BE WITHIN 8" OF END OF BEAM. . SEE TYPICAL ELEVATION - VIEW OF BEAM. 4. VERTICAL REINFORCING SHALL HAVE HOOKS TOP AND BOTTOM.

1. USE OPEN-END UNITS AT INTERSECTIONS OF BEAMS AND JAMBS.





SCHEDULE		CONFIGURATION 'A'
	SOLID GROUT 24" OF JAMB VERTICA SEE SCH	AL REINFORCING - HEDULE
	SOLID GROUT	CONFIGURATION 'B'
	24" OF JAMB	CONFIGURATION 'C'
	SOLID GROUT VERTICA	L REINFORCING -

SEE SCHEDULE

SEE ARCH.

TOP OF

FOUNDATION/ FTG.

VIDTH 2'-0" 2'-6"	LENGTH	THICK	LENGTHW					
2'-0"				SSWISE R		REMARKS		
			NO.	SIZE	NO.	SIZE	SPA.	
2'-6"	CONT.	12"	(2)	#5				
	CONT.	12"	(3)	#5				
3'-0"	CONT.	12"	(3)	#5				
4'-0"	CONT.	14"	(4)	#5	(1)	#5	12"o.c.	
5'-0"	CONT.	16"	(5)	#6	(1)	#6	12"o.c.	
6'-0"	CONT.	20"	(6)	#6	(1)	#6	12"o.c.	REINFORCE TOP & BOTTO
7'-0"	CONT.	22"	(7)	#7	(1)	#7	12"o.c.	REINFORCE TOP & BOTTO
8'-0"	CONT.	24"	(8)	#8	(1)	#8	12"o.c.	REINFORCE TOP & BOTTO
3'-0"		12"	(3)		(3)			
3'-6"	3'-6"	12"	(3)	#5	(3)	#5		
4'-0"	4'-0"	12"	(4)	#5	(4)	#5		
4'-6"	4'-6"	12"	(5)	#5	(5)	#5		
5'-0"	5'-0"	14"	(5)	#5	(5)	#5		
5'-6"	5'-6"	14"	(6)	#6	(6)	#6		
6'-0"	6'-0"	16"	(6)	#6	(6)	#6		
6'-6"	6'-6"	18"	(7)	#6	(7)	#6		
7'-0"	7'-0"	18"	(7)	#6	(7)	#6		
7'-6"	7'-6"	20"	(7)	#7	(7)	#7		
8'-0"	8'-0"	22"	(8)	#7	(8)	#7		
8'-6"	8'-6"	22"	(8)	#7	(8)	#7		
9'-0"	9'-0"	24"	(9)	#7	(9)	#7		
9'-6"	9'-6"	26"	(10)	#7	(10)	#7		
10'-0"	10'-0"	26"	(10)	#8	(10)	#8		
3'-0"	6'-0"	10"	(3)	#5	(6)	#5		
6'-6"	6'-6"	24"	(7)	#3 #7	(7)	#5 #7		REINFORCE TOP AND BOTT
5 6 7 8 8 8 8 8 8	5'-0" 5'-0" 5'-0" 5'-0" 5'-0" 5'-0" 5'-6" 5'-6" 5'-6" 5'-6" 5'-6" 6'-0" 5'-6" 6'-0" 6'-6"	S'-0" CONT. S'-0" CONT. S'-0" CONT. S'-0" CONT. S'-0" 3'-0" S'-6" 3'-6" S'-6" 4'-0" S'-6" 5'-6" S'-0" 6'-0" S'-6" 7'-6" S'-6" 7'-6" S'-6" 8'-6" S'-0" 9'-6" S'-6" 9'-6"	CONT. 16" CONT. 20" CONT. 20" CONT. 22" CONT. 24" CONT. 26" CONT. CONT. 26" CONT. CONT.	S'-0" CONT. 16" (5) S'-0" CONT. 20" (6) S'-0" CONT. 22" (7) S'-0" CONT. 24" (8) S'-0" 3'-0" 12" (3) S'-6" 3'-6" 12" (4) S'-0" 4'-0" 12" (5) S'-0" 5'-0" 14" (5) S'-0" 5'-6" 14" (6) S'-0" 6'-0" 16" (6) S'-0" 6'-6" 18" (7) S'-0" 7'-6" 20" (7) S'-0" 8'-0" 22" (8) S'-6" 8'-6" 22" (8) S'-6" 9'-6" 26" (10) O'-0" 10'-0" 26" (10)	CONT. 16" (5)	G'-0" CONT. 16" (5) #6 (1) G'-0" CONT. 20" (6) #6 (1) G'-0" CONT. 22" (7) #7 (1) G'-0" CONT. 24" (8) #8 (1) G'-0" 3'-0" 12" (3) #5 (3) G'-6" 3'-6" 12" (3) #5 (3) G'-6" 3'-6" 12" (4) #5 (4) G'-6" 4'-6" 12" (5) #5 (5) G'-0" 5'-0" 14" (5) #5 (5) G'-0" 5'-0" 14" (6) #6 (6) G'-0" 6'-0" 16" (6) #6 (6) G'-6" 6'-6" 18" (7) #6 (7) G'-6" 7'-6" 20" (7) #7 (7) G'-6" 8'-6" 22" (8) #7	S'-0" CONT. 16" (5)	CONT. 16" (5)

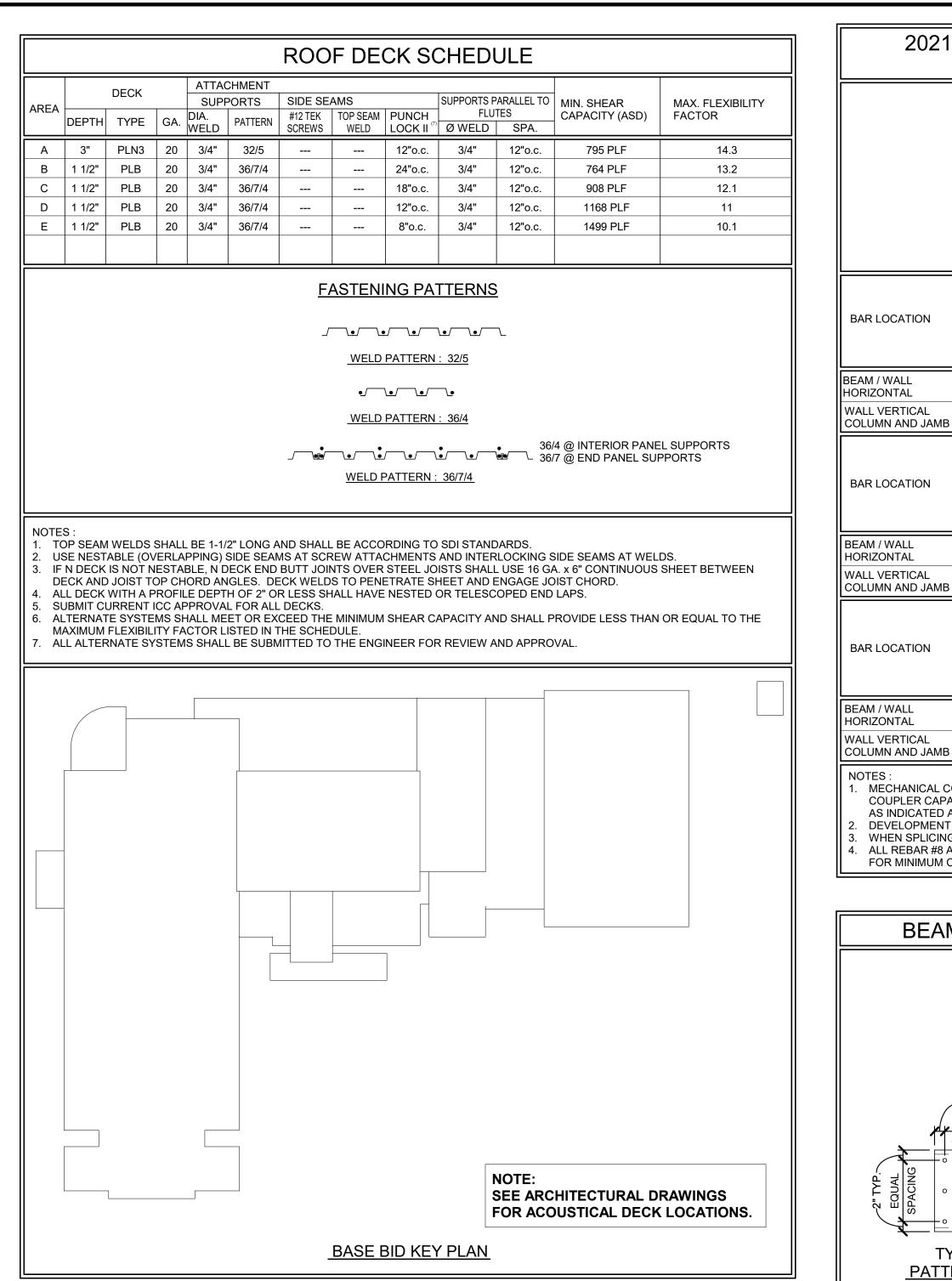
TYP. FOOTING SECTION

	MASONRY WALL SCHEDULE										
		VERT. REINF.		НОГ	RIZ. BOI	ND BEAM	1 REINF.				
MARK	THICK.	SIZE	SPACE	NO.	SIZE	SPACE	@ ROOF	@ ELEVATED FLOOR	COMMENTS		
MW-8A	8"	#5	32"o.c.	(2)	#4	48"	(2) #4				
MW-8B	8"	#5	32"o.c.	(2)	#5	32"	(2) #5	(2) #5			
MW-12A	12"	(2) #5	16"o.c.	(2)	#5	48"			SOLID GROUTED		
MW-12B	12"	(2) #5	24"o.c.	(2)	#5	48"	(4) #5	(4) #5	SOLID GROUTED		
NOTES	3:										

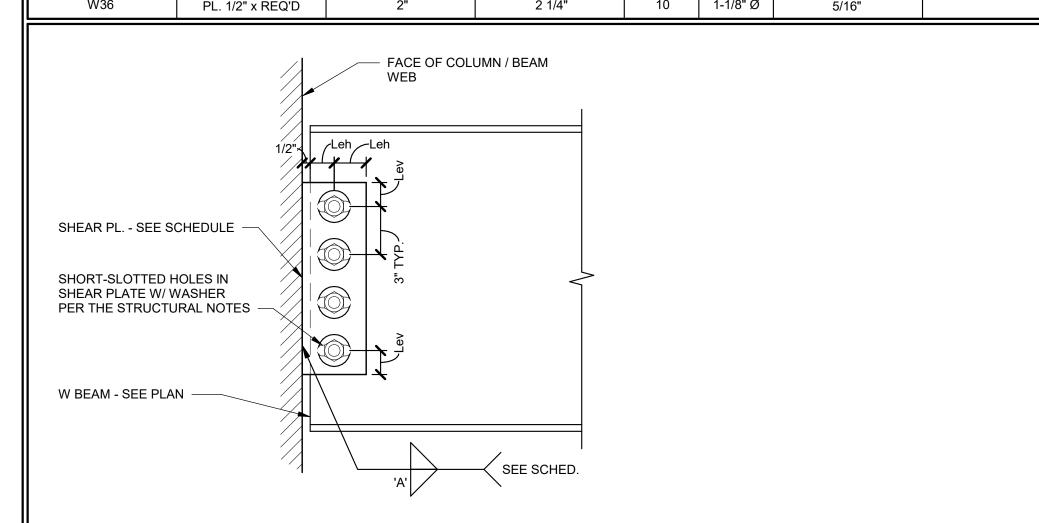
FOR ANY CMU WALLS NOT SPECIFICALLY CALLED OUT IN PLANS, USE MW-8A. VERTICAL REINFORCING TO BE @ CENTERLINE OF WALL WHERE SINGLE BAR IS NOTED IN SCHEDULE. POSITION BARS 2" FROM EACH FACE OF WALL WHERE (2) BARS ARE SPECIFIED.. SOLID GROUTING OF WALLS IS UNACCEPTABLE EXCEPT WHERE SPECIFICALLY NOTED. SEE STRUCTURAL NOTES FOR ADDITIONAL INFORMATION. 5. A BOND BEAM SHALL BE LOCATED IN THE FIRST COURSE ABOVE THE FOUNDATION IF VERTICAL DOWELS HAVE BEEN BENT TO ALIGN WITH VERTICAL CELLS, WHETHER OR NOT MASONRY WEBS HAVE BEEN CUT.

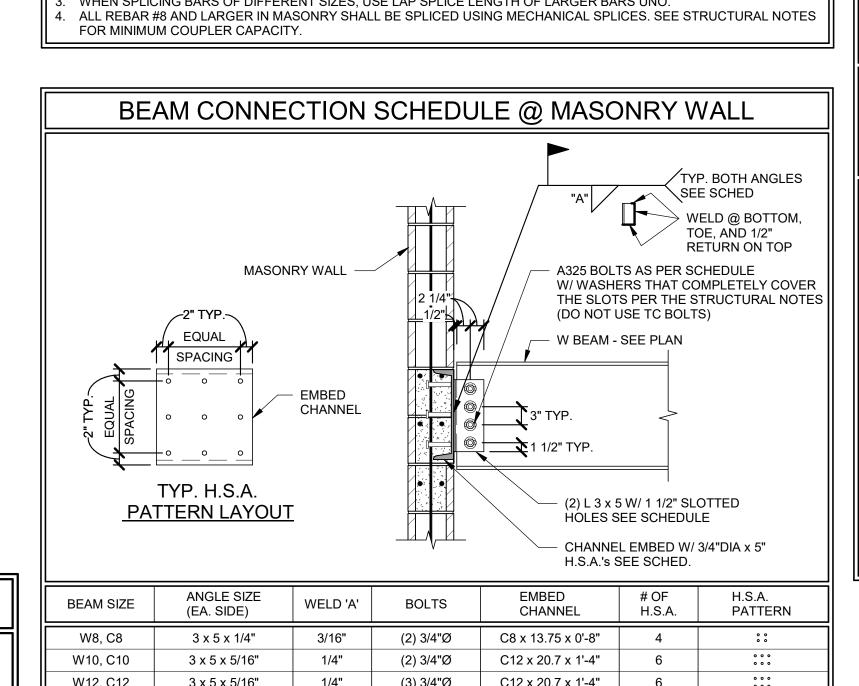
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SCHEDULES



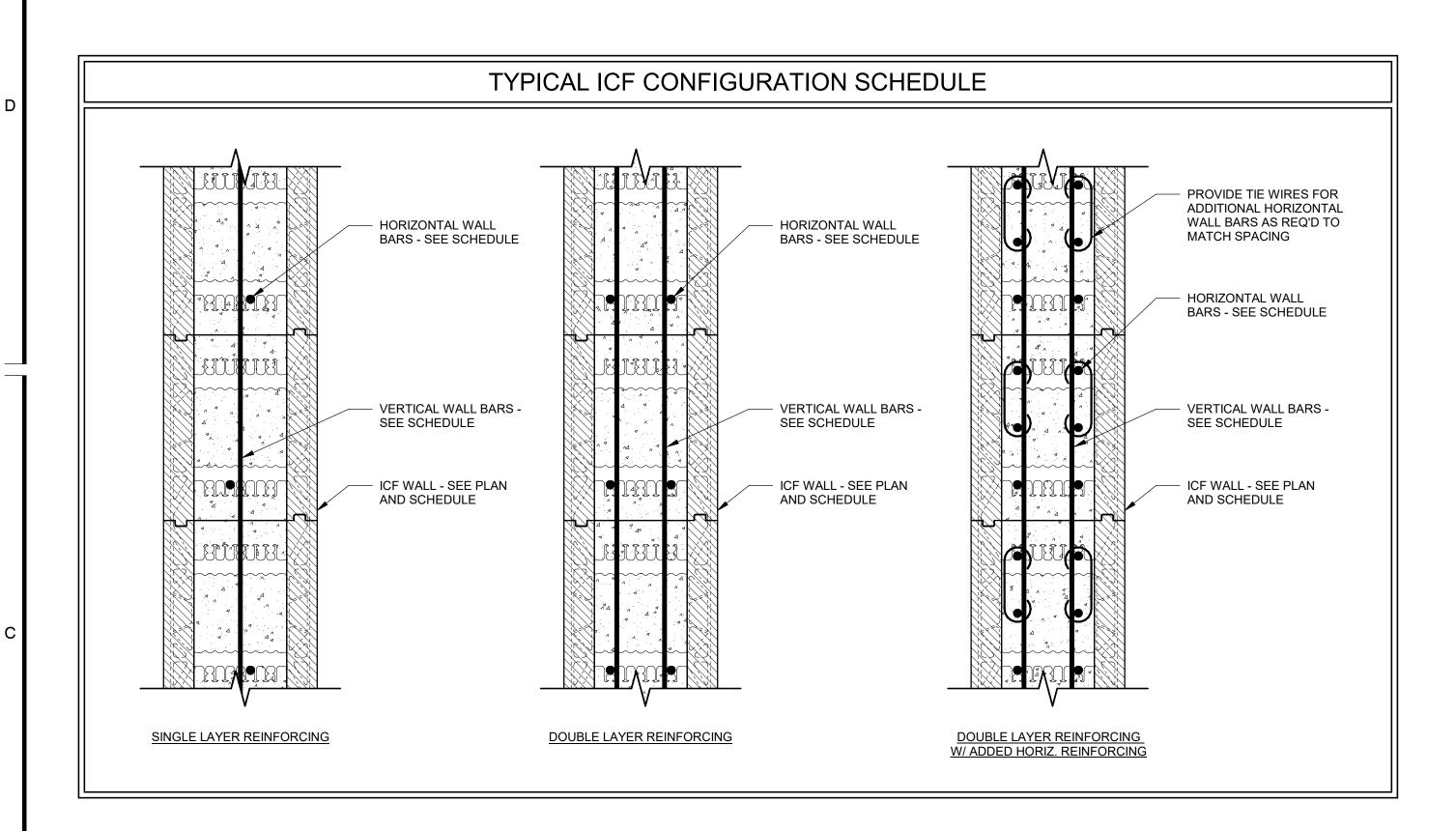
BEAM CONNECTION SCHEDULE										
	SH	EAR PLATE INFORMATION	ON	BOLTS W/ STANDARD						
BEAM DEPTH	PL. DIMENSIONS W/ SHORT-	Lev	Leh	WASHERS SLOTS	OVER	WELD 'A'	COMMENTS			
	SLOTTED HOLES			No.						
W8, W10	PL. 1/4" x REQ'D	1 1/2"	2"	2	3/4" Ø	3/16"				
W12, W14	PL. 5/16" x REQ'D	1 1/2"	2"	3	3/4" Ø	1/4"				
W16	PL. 5/16" x REQ'D	1 1/2"	2"	4	3/4" Ø	1/4"				
W18	PL. 5/16" x REQ'D	1 1/2"	2"	5	3/4" Ø	1/4"				
W21	PL. 5/16" x REQ'D	1 1/2"	2"	6	3/4" Ø	1/4"				
W24	PL. 3/8" x REQ'D	1 1/2"	2"	7	7/8" Ø	1/4"				
W27	PL. 3/8" x REQ'D	1 1/2"	2"	7	7/8" Ø	1/4"				
W30	PL. 1/2" x REQ'D	1 3/4"	2"	8	1" Ø	5/16"				
W33	PL. 1/2" x REQ'D	1 3/4"	2"	9	1" Ø	5/16"				
W36	PL. 1/2" x REQ'D	2"	2 1/4"	10	1-1/8" Ø	5/16"				

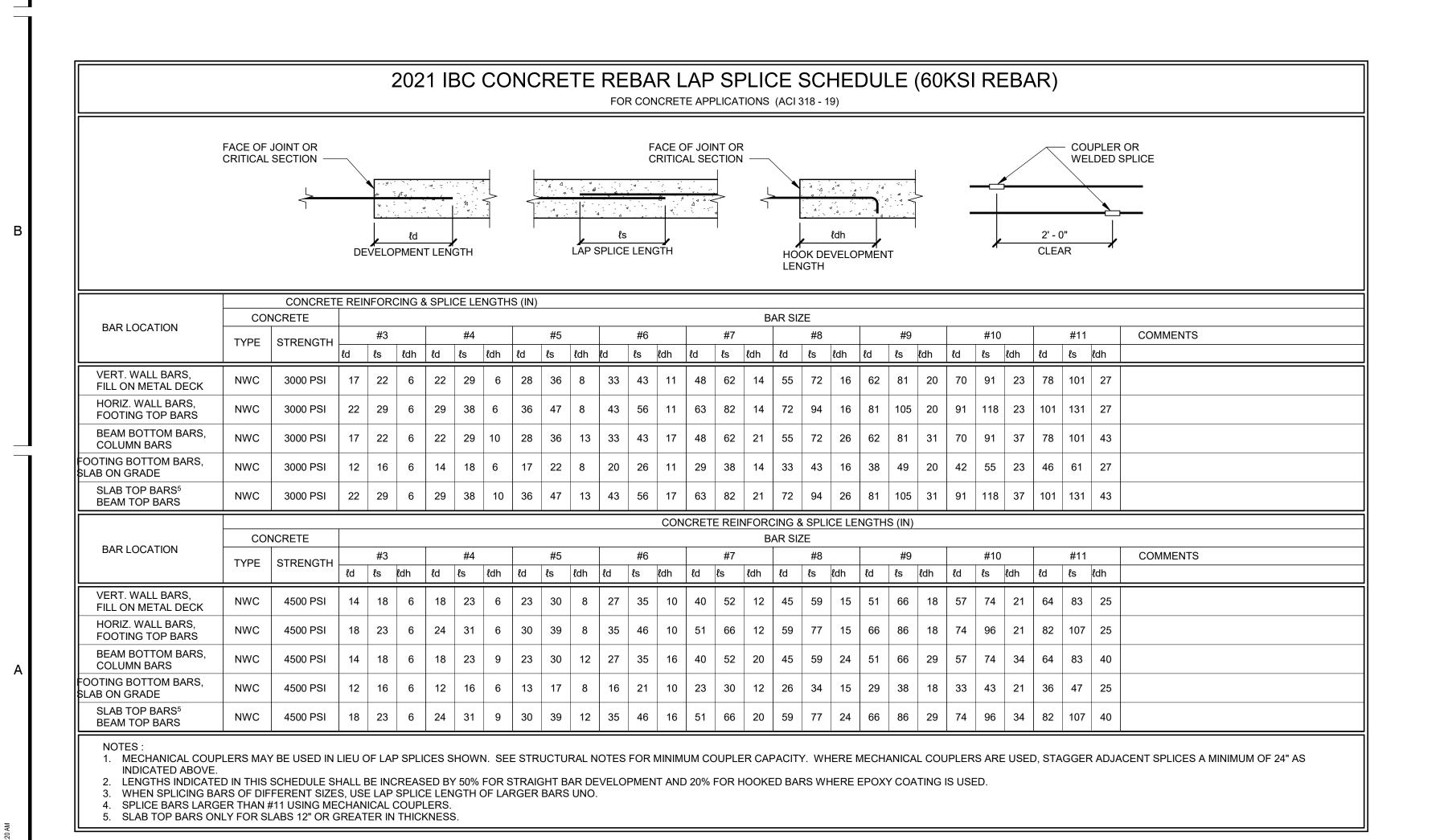


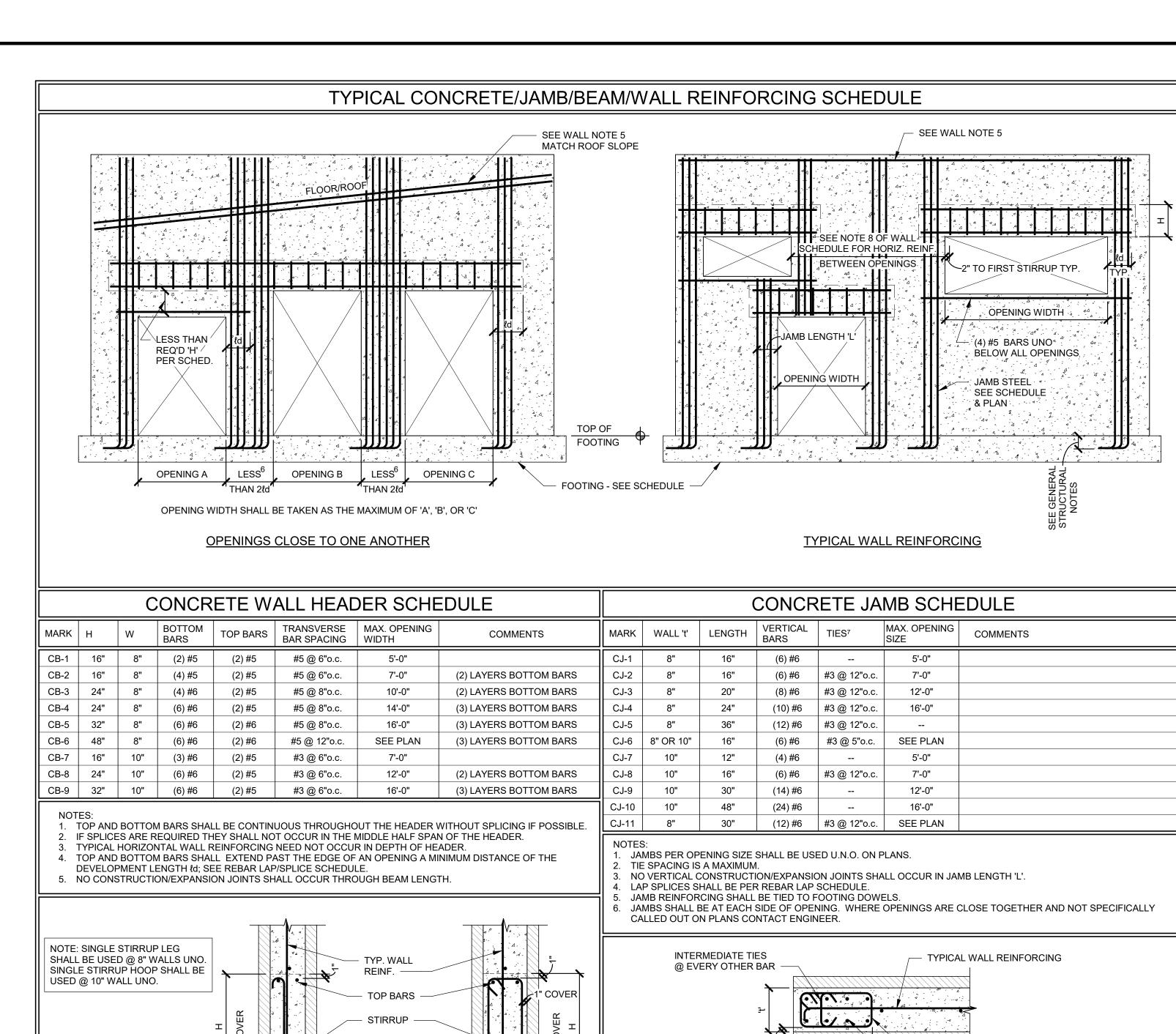


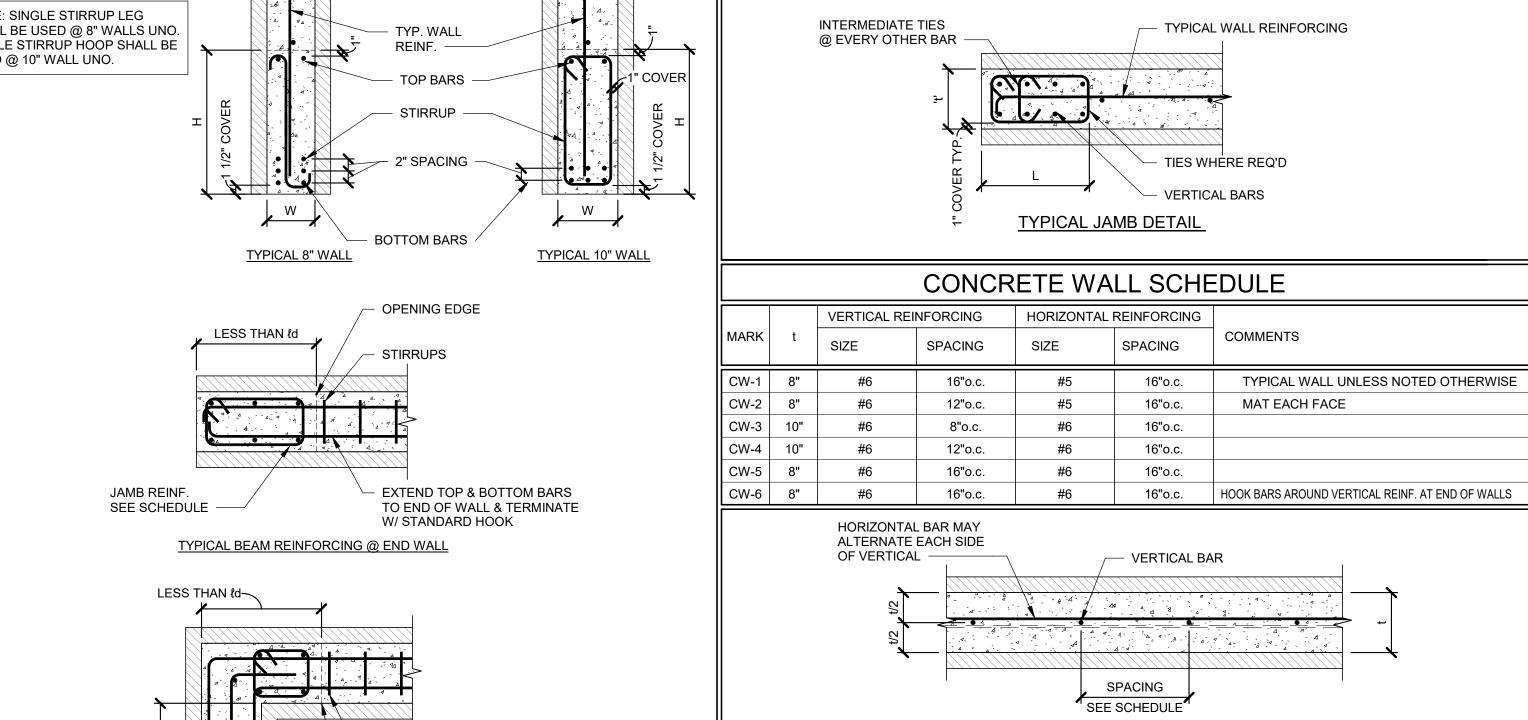
	BEAM SIZE	(EA. SIDE)	WELD A	BOLIS	CHANNEL	H.S.A.	PATTERN
il	W8, C8	3 x 5 x 1/4"	3/16"	(2) 3/4"Ø	C8 x 13.75 x 0'-8"	4	0 0
	W10, C10	3 x 5 x 5/16"	1/4"	(2) 3/4"Ø	C12 x 20.7 x 1'-4"	6	0 0 0
	W12, C12	3 x 5 x 5/16"	1/4"	(3) 3/4"Ø	C12 x 20.7 x 1'-4"	6	0 0 0
	W14	3 x 5 x 5/16"	1/4"	(3) 3/4"Ø	C15 x 33.9 x 1'-4"	9	0 0 0 0 0 0 0
il	W16	3 x 5 x 5/16"	1/4"	(4) 3/4"Ø	C15 x 33.9 x 2'-0"	12	0 0 0 0 0 0 0 0 0 0
╢	W18	3 x 5 x 5/16"	1/4"	(5) 3/4"Ø	(2) C12 x 20.7 x 2'-0"	12	0 0 0 0 0 0 0 0 0
$\ $	1. AT THE CON	NTRACTORS OPTION, ((2) C8 x 13.7 N	MAY BE SUBSTITU	JTED FOR THE C15 x 33.	.9. HSA PATT	ERN SHALL BE

RECONFIGURED & EQUALLY DISTRIBUTED BETWEEN THE TWO CHANNELS. DOUBLE CHANNELS SHALL BE WELDED TOGETHER WITH A 2" STITCH WELD EVERY 4" AT THE FLANGE TOE.









- OPENING EDGE

TYPICAL BEAM REINFORCING @ WALL CORNER

TYPICAL WALL REINFORCING

I. VERTICAL BARS IN SINGLE MAT WALLS SHALL BE CENTERED IN WALL. 2. VERTICAL BARS IN DOUBLE MAT WALLS SHALL BE PLACED TIGHT AGAINST HORIZONTAL BARS AS SHOWN AND HELD IN PLACE WHILE PLACING CONCRETE. 3. TYPICAL VERTICAL WALL REINFORCING NEED NOT EXTEND INTO JAMB.

MAT EACH FACE

SINGLE MAT IN WALL

SPACING

SEE SCHEDULE

- HORIZONTAL BAR

VERTICAL BAR

4. TYPICAL HORIZONTAL WALL REINFORCING SHALL EXTEND INTO JAMB BARS AS SHOWN.

5. PROVIDE (4) #5 BARS x CONT. AT ALL FLOOR AND ROOF ELEVATIONS TYPICAL UNLESS NOTED OTHERWISE. 6. U.N.O. USÈ CW-1 FOR ALL WALLS.

8. SPACE HORIZONTAL REINFORCING @ 8"o.c. FOR WALL SEGMENTS THAT ARE LESS THAN 4'-0" IN LENGTH.

7. PROVIDE (2) #5 BARS x CONT. @ TOP OF WALL WHERE WALL EXTENDS PAST ROOF.

SCHEDULES

MIDD

archite

sign

a

(5) 3/4"Ø

(6) 7/8"Ø

(7) 7/8"Ø

15" x 1'-8"

15" x 2'-0"

15" x 2'-0"

12

12

					STRUCTUR	AL STEF
						STABLISHE
INSPECTION TASKS PRIOR TO WELDING (TABLE N5.4-1)	FABRICA QUALITY CO	NTROL	SPECIAL INS QUALITY ASS	SURANCE	NOTES	- TABLIOTIL
WELDER QUALIFICATION RECORDS AND CONTINUITY RECORDS	•			•		
WELDING PROCEDURE SPECIFICATIONS (WPSs) AVAILABLE	•		•			
MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE	•		•		PERIODIC - OBSERVE THESE ITEMS ON A RANDOM BAS OPERATIONS NEED NOT BE DELAYED PENDING THESE	
MATERIAL IDENTIFICATION (TYPE / GRADE) WELDER IDENTIFICATION SYSTEM ¹		•		•	INSPECTIONS. CONTINUOUS - PERFORM THESE TASKS FOR EACH WE	LDED JOINT
FIT-UP OF GROOVE WELDS (INCLUDING JOINT GEOMETRY)				•	OR MEMBER. QUALITY CONTROL (QC) SHALL BE PROVIDED BY THE F.	
* JOINT PREPARATION					AND ERECTOR.	
* DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL)					QUALITY ASSURANCE (QA) SHALL BE PROVIDED BY OTH REQUIRED BY THE AUTHORITY HAVING JURISDICTION (AHJ),
* CLEANLINESS (CONDITION OF STEEL SURFACES)				•	APPLICABLE BUILDING CODE (ABC), PURCHASER, OWN ENGINEER OF RECORD (EOR). NONDESTRUCTIVE TEST	
* TACKING (TACK WELD QUALITY AND LOCATION)					SHALL BE PERFORMED BY THE AGENCY OR FIRM RESP FOR QUALITY ASSURANCE, EXCEPT AS PERMITTED IN	
* BACKING TYPE AND FIT (IF APPLICABLE)					ACCORDANCE WITH SECTION N6.	NDDANCE
FIT-UP OF CJP GROOVE WELDS OFHSS T-, Y-, AND K-JOINTS WITHOUT BACKING (INCLUDING JOINT GEOMETRY)					QC AND QA INSPECTORS SHALL BE QUALIFIED IN ACCO WITH AISC 360-16 CHAPTER N4. NONDESTRUCTIVE TESTING PERSONNEL SHALL BE QU	
* JOINT PREPARATIONS	•			•	ACCORDANCE WITH AISC 360-16 CHAPTER N4.3. NONDESTRUCTIVE TESTING OF WELDED JOINTS SHALL	COMPLY
* DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL)					WITH AISC 360-16 CHAPTER N5.5a AND b. OBSERVATION OF WELDING OPERATIONS AND VISUAL	INSPECTION
* CLEANLINESS (CONDITION OF STEEL SURFACES) * TACKING (TACK WELD QUALITY AND LOCATION)					OF IN-PROCESS AND COMPLETED WELDS SHALL BE TH METHOD TO CONFIRM THAT THE MATERIALS, PROCEDU	IE PRIMARY
CONFIGURATION AND FINISH OF ACCESS HOLES				•	WORKMANSHIP ARE IN CONFORMANCE WITH THE CON	STRUCTION
FIT-UP OF FILLET WELDS					DOCUMENTS. FOR STRUCTURAL STEEL, ALL PROVISIO D1.1 / D1.1M STRUCTURAL WELDING CODE - STEEL FOR	
* DIMENSIONS (ALIGNMENT, GAPS AT ROOT)					STATICALLY LOADED STRUCTURES SHALL APPLY. THERMALLY CUT SURFACES OF ACCESS HOLES SHALL	
* CLEANLINESS (CONDITION OF STEEL SURFACES)					BY QA USING MT OR PT, WHEN THE FLANGE THICKNES: 2 IN. (50mm) FOR ROLLED SHAPES, OR WHEN THE WEB	S EXCEEDS
* TACKING (TACK WELD QUALITY AND LOCATION)					EXCEEDS 2 IN. (50mm) FOR BUILT-UP SHAPES. ANY CRA	ACK SHALL
CHECK WELDING EQUIPMENT		•			BE DEEMED UNACCEPTABLE REGARDLESS OF SIZE OR WHEN REQUIRED BY APPENDIX 3, TABLE A-3.1, WELDED	O JOINTS
¹ THE FABRICATOR OR ERECTOR, AS APPLICABLE, SHALL MAINTAIN A SYSTEM JOINT OR MEMBER CAN BE IDENTIFIED. STAMPS, IF USED, SHALL BE THE LO			IO HAS WELDED) A	REQUIRING WELD SOUNDNESS TO BE ESTABLISHED BY RADIOGRAPHICS OR ULTRASONIC INSPECTION SHALL BY QA AS PRESCRIBED. REDUCTION IN THE RATE OF U	BE TESTED
INSPECTION TASKS DURING WELDING (TABLE N5.4-2)	CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC	PROHIBITED. 1. REDUCTION OF RATE OF ULTRASONIC TESTING - THE F	RATE OF UT
CONTROL AND HANDLING OF WELDING CONSUMABLES					IS ONLY PERMITTED TO BE REDUCED IF APPROVED BY AND THE AHJ PER AISC 360-16 CHAPTER N5.5e.	
* PACKAGING		•		•	2. FOR STRUCTURES IN RISK CATEGORY II, WHERE THE II	
* EXPOSURE CONTROL		_		_	FOR UT IS 10%, THE NDT RATE FOR AN INDIVIDUAL WEL WELDING OPERATOR SHALL BE INCREASED TO 100% S	
NO WELDING OVER CRACKED TACK WELDS ENVIRONMENTAL CONDITIONS				•	REJECT RATE, THE NUMBER OF WELDS CONTAINING UNACCEPTABLE DEFECTS DIVIDED BY THE NUMBER OF	- WELDS
* WIND SPEED WITHIN LIMITS				•	COMPLETED, EXCEEDS 5% OF THE WELDS TESTED FOR WELDER OR WELDING OPERATOR. A SAMPLING OF AT	R THE
* PRECIPITATION AND TEMPERATURE					COMPLETED WELDS FOR A JOB SHALL BE MADE PRIOR	R TO
WPS FOLLOWED					IMPLEMENTING SUCH AN INCREASE. WHEN THE REJECTHE WELDER OR WELDING OPERATOR, AFTER A SAMP	LING OF AT
* SETTINGS ON WELDING EQUIPMENT					LEAST 40 COMPLETED WELDS, HAS FALLEN TO 5% OR I RATE OF UT SHALL BE RETURNED TO 10%. FOR EVALU	
* TRAVEL SPEED					REJECT RATE OF CONTINUOUS WELDS OVER 3 FT (1M) WHERE THE EFFECTIVE THROAT IS 1 IN. (25mm) OR LES	
* SELECTED WELDING MATERIALS		•		•	IN. (300mm) INCREMENT OR FRACTION THEREOF SHALL CONSIDERED AS ONE WELD. FOR EVALUATING THE RE	BE
* SHIELDING GAS TYPE / FLOW RATE * PREHEAT APPLIED					ON CONTINUOUS WELDS OVER 3 FT (1M) IN LENGTH WI	HERE THE
* INTERPASS TEMPERATURE MAINTAINED (MIN. / MAX)					EFFECTIVE THROAT IS GREATER THAN 1 IN. (25mm), EA (150mm) OF LENGTH OR FRACTION THEREOF SHALL BE	
* PROPER POSITION (F, V, H, OH)					CONSIDERED ON WELD. 3. ALL NDT PERFORMED SHALL BE DOCUMENTED. FOR S	HOP
WELDING TECHNIQUES					FABRICATION, THE NDT REPORT SHALL IDENTIFY THE TWELD BY PIECE MARK AND LOCATION IN THE PIECE. FOR	TESTED
* INTERPASS AND FINAL CLEANING				•	WORK, THE NDT REPORT SHALL IDENTIFY THE TESTED	WELD BY
* EACH PASS WITHIN PROFILE LIMITATIONS					LOCATION IN THE STRUCTURE, PIECE MARK, AND LOCATION IN THE STRUCTURE, PIECE MARK, AND LOCATION OF I	NDT, THE
* EACH PASS MEETS QUALITY REQUIREMENTS PLACEMENT AND INSTALLATION OF STEEL HEADED STUD ANCHORS					NDT RECORD SHALL INDICATE THE LOCATION OF THE IT THE BASIS OF REJECTION	
INSPECTION TASKS AFTER WELDING (TABLE N5.4-3)	CONTINUOUS	DEBIODIC	CONTINUOUS	BEBIODIC	 DEMAND CRITICAL WELDS SHALL MEET THE PROVISIO AISC 341-16 AND WELDING METHODS, PROCEDURES AN 	
WELDS CLEANED	CONTINUOUS		CONTINUOUS		CONTROL SHALL COMPLY WITH AWS D1.1 AND THE FOI a. ARC STRIKES, GOUGES AND OTHER IMPERFECTIO	LLOWING:
SIZE, LENGTH AND LOCATION OF WELDS	•	•	•	•	OR ADJACENT TO THE JOINT, SHALL BE REPAIRED	
WELDS MEET VISUAL ACCEPTANCE CRITERIA			•		REMOVED. b. PREHEAT AND INTER-PASS REQUIREMENTS AS OL	JTLINED IN
* CRACK PROHIBITION					SECTION 3.5. c. UNREPAIRED CRACKS, GOUGES, AND NOTCHES W	/ILL NOT BE
* WELD / BASE-METAL FUSION					PERMITTED IN THE JOINT AREA. d. USE ELECTRODES WITH CHARPY V-NOTCH ABSOR	RBED
* CRATER CROSS SECTION	•		•		ENERGY EQUAL TO OR GREATER THAN 20 FT-LBS DEGREES FAHRENHEIT UNDER AWS A5 CLASSIFIC	AT 20
* WELD PROFILES					METHODS, AND 40 FT-LBS AT 70 DEGREES FAHREN	NHEIT USING
* WELD SIZE * UNDERCUT					TEST PROCEDURES PRESCRIBED IN APPENDIX X O ACCEPTABLE ELECTRODES INCLUDE E70TG-K2, E7	
* POROSITY						
ARC STRIKES	•		•			
K-AREA ¹	•		•			
WELD ACCESS HOLES IN ROLLED HEAVY SHAPES AND BUILT-UP HEAVY SHAPES ²	•		•			
BACKING REMOVED AND WELD TABS REMOVED (IF REQUIRED)	•		•			
REPAIR ACTIVITIES	•		•			
DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT OR MEMBER	•		•			
NO PROHIBITED WELDS HAVE BEEN ADDED WITHOUT THE APPROVAL OF THE EOR		•		•		

1WHEN WELDING OF DOUBLER PLATES, CONTINUITY PLATES OR STIFFENERS HAS BEEN PERFORMED IN THE K-AREA.

²AFTER ROLLED HEAVY SHAPES (SEE SECTION A3.1c) AND BUILT-UP HEAVY SHAPES (SEE SECTION A3.1d) ARE WELDED,

VISUALLY INSPECT THE WEB K-AREA FOR CRACKS WITHIN 3 IN. (75mm) OF THE WELD)

VISUALLY INSPECT THE WELD ACCESS HOLE FOR CRACKS.

NOTES	INSPECTION TASKS PRIOR TO BOLTING (TABLE N5.6-1)	CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC	
	MANUFACTURER'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS		•	•		1.
	FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS		•		•	
ODIC - OBSERVE THESE ITEMS ON A RANDOM BASIS. RATIONS NEED NOT BE DELAYED PENDING THESE ECTIONS.	PROPER FASTENERS SELECTED FOR THE JOINT DETAIL (GRADE, TYPE, BOLT LENGTH IF THREADS ARE TO BE EXCLUDED FROM SHEAR PLANE)		•		•	2.
TINUOUS - PERFORM THESE TASKS FOR EACH WELDED JOINT IEMBER.	PROPER BOLTING PROCEDURES SELECTED FOR JOINT DETAIL		•		•	J.
LITY CONTROL (QC) SHALL BE PROVIDED BY THE FABRICATOR ERECTOR. LITY ASSURANCE (QA) SHALL BE PROVIDED BY OTHERS WHEN	CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FAYING SURFACE CONDITION AND HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQUIREMENTS		•		•	4.
JIRED BY THE AUTHORITY HAVING JURISDICTION (AHJ), ICABLE BUILDING CODE (ABC), PURCHASER, OWNER, OR NEER OF RECORD (EOR). NONDESTRUCTIVE TESTING (NDT)	PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL OBSERVED AND DOCUMENTED FOR FASTENER ASSEMBLIES AND METHODS USED	•			•	
L BE PERFORMED BY THE AGENCY OR FIRM RESPONSIBLE OUALITY ASSURANCE, EXCEPT AS PERMITTED IN DRDANCE WITH SECTION N6.	PROPER STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS AND OTHER FASTENER COMPONENTS		•		•	5.
ND QA INSPECTORS SHALL BE QUALIFIED IN ACCORDANCE AISC 360-16 CHAPTER N4.	INSPECTION TASKS DURING BOLTING (TABLE N5.6-2)	CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC	
DESTRUCTIVE TESTING PERSONNEL SHALL BE QUALIFIED IN DRDANCE WITH AISC 360-16 CHAPTER N4.3. DESTRUCTIVE TESTING OF WELDED JOINTS SHALL COMPLY	FASTENER ASSEMBLIES, PLACED IN ALL HOLES AND WASHERS (IF REQUIRED) ARE POSITIONED AS REQUIRED		•		•	6.
AISC 360-16 CHAPTER N5.5a AND b. ERVATION OF WELDING OPERATIONS AND VISUAL INSPECTION I-PROCESS AND COMPLETED WELDS SHALL BE THE PRIMARY	JOINT BROUGHT TO THE SNUG-TIGHT CONDITION PRIOR TO THE PRETENSIONING OPERATION		•		•	
HOD TO CONFIRM THAT THE MATERIALS, PROCEDURES AND KMANSHIP ARE IN CONFORMANCE WITH THE CONSTRUCTION	FASTENER COMPONENT NOT TURNED BY THE WRENCH PREVENTED FROM ROTATING		•		•	
JMENTS. FOR STRUCTURAL STEEL, ALL PROVISIONS OF AWS D1.1M STRUCTURAL WELDING CODE - STEEL FOR ICALLY LOADED STRUCTURES SHALL APPLY. RMALLY CUT SURFACES OF ACCESS HOLES SHALL BE TESTED	FASTENERS ARE PRETENSIONED IN ACCORDANCE WITH THE RCSC SPECIFICATION, PROGRESSING SYSTEMATICALLY FROM THE MOST RIGID POINT TOWARD THE FREE EDGES		•		•	7.
A USING MT OR PT, WHEN THE FLANGE THICKNESS EXCEEDS (50mm) FOR ROLLED SHAPES, OR WHEN THE WEB THICKNESS	INSPECTION TASKS AFTER BOLTING (TABLE N5.6-3)	CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC	
EDS 2 IN. (50mm) FOR BUILT-UP SHAPES. ANY CRACK SHALL EEMED UNACCEPTABLE REGARDLESS OF SIZE OR LOCATION.	DOCUMENT ACCEPTANCE OR REJECTION OF BOLTED CONNECTIONS	•		•		8.

STRUCTURAL STEEL SPECIAL INSPECTION SCHEDULE

ESTABLISHED PER 2021 IBC SECTION 1705.2.1

I. PERIODIC - OBSERVE THESE ITEMS ON A RANDOM BASIS. OPERATIONS NEED NOT BE DELAYED PENDING THESE 2. CONTINUOUS - PERFORM THESE TASKS FOR EACH BOLTED CONNECTION. 3. QUALITY CONTROL (QC) SHALL BE PROVIDED BY THE FABRICATOR 4. QUALITY ASSURANCE (QA) SHALL BE PROVIDED BY OTHERS WHEN REQUIRED BY THE AUTHORITY HAVING JURISDICTION (AHJ), APPLICABLE BUILDING CODE (ABC), PURCHASER, OWNER, OR ENGINEER OF RECORD (EOR). NONDESTRUCTIVE TESTING (NDT) SHALL BE PERFORMED BY THE AGENCY OR FIRM RESPONSIBLE FOR QUALITY ASSURANCE, EXCEPT AS PERMITTED IN ACCORDANCE WITH SECTION N7. 5. FOR SNUG-TIGHT JOINTS, PRE-INSTALLATION VERIFICATION TESTING AS SPECIFIED IN TABLE N5.6-1 AND MONITORING OF THE INSTALLATION PROCEDURES AS SPECIFIED IN TABLE N5.6-2 ARE NOT APPLICABLE. THE QCI AND QAI NEED NOT BE PRESENT DURING THE INSTALLATION OF FASTENERS IN SNUG-TIGHT JOINTS. 6. FOR PRETENSIONED JOINTS AND SLIP-CRITICAL JOINTS, WHEN THE INSTALLER IS USING THE TURN-OF-NUT METHOD WITH MATCHMARKING TECHNIQUES, THE DIRECT-TENSION-INDICATOR METHOD, OR THE TWIST-OFF-TYPE TENSION CONTROL BOLT METHOD, MONITORING OF BOLT PRETENSIONING PROCEDURES SHALL BE AS SPECIFIED IN TABLE N5.6-2. THE QCI AND QAI NEED NOT BE PRESENT DURING THE INSTALLATION OF FASTENERS WHEN THESE METHODS ARE USED BY THE INSTALLER. Y. FOR PRETENSIONED JOINTS AND SLIP-CRITICAL JOINTS, WHEN THE INSTALLER IS USING THE CALIBRATED WRENCH METHOD OR THE TURN-OF-NUT METHOD WITHOUT MATCHMARKING, MONITORING OF BOLT PRETENSIONING PROCEDURES SHALL BE AS SPECIFIED IN TABLE N5.6-2. THE QCI AND QAI SHALL BE ENGAGED IN THEIR ASSIGNED INSPECTION DUTIES DURING INSTALLATION OF FASTENERS WHEN THESE METHODS ARE USED BY THE INSTALLER. OBSERVATION OF BOLTING OPERATIONS SHALL BE THE PRIMARY METHOD USED TO CONFIRM THAT THE MATERIALS, PROCEDURES AND WORKMANSHIP INCORPORATED IN CONSTRUCTION ARE IN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS AND THE

PROVISIONS OF THE RCSC SPECIFICATION.

NOTES

GENERAL STEEL SPECIAL INSPECTION NOTES:

QUALITY ASSURANCE (QA) INSPECTION OF FABRICATED ITEMS SHALL BE MADE AT THE FABRICATOR'S PLANT. THE QUALITY ASSURANCE INSPECTOR (QAI) SHALL SCHEDULE THIS WORK TO MINIMIZE INTERRUPTION TO THE WORK OF THE FABRICATOR. QA INSPECTION OF THE ERECTED STEEL SYSTEM SHALL BE MADE AT THE PROJECT SITE. THE QAI SHALL SCHEDULE THIS WORK TO MINIMIZE INTERRUPTION TO THE WORK OF THE ERECTOR. WHERE A TASK IS NOTED TO BE PERFORMED BY BOTH QC AND QA, IT IS PERMITTED TO COORDINATE THE INSPECTION FUNCTION BETWEEN THE QCI AND QAI SO THAT THE INSPECTION FUNCTIONS ARE

W18x

W24x

3 x 4 x 5/16 x 1'-3

3 x 4 x 3/8 x 1'-6"

3 x 4 x 3/8 x 1'-9"

1/4"

- PERFORMED BY ONLY ONE PARTY. WHERE QA RELIES UPON INSPECTION FUNCTIONS PERFORMED BY QC, THE APPROVAL OF THE ENGINEER OF RECORD AND THE AUTHORITY HAVING JURISDICTION IS THE FABRICATOR'S QCI SHALL INSPECT THE FABRICATED STEEL TO VERIFY COMPLIANCE WITH THE DETAILS SHOWN ON THE SHOP DRAWINGS, SUCH AS PROPER APPLICATION OF JOINT DETAILS AT EACH CONNECTION. THE ERECTOR'S QCI SHALL INSPECT THE ERECTED STEEL FRAME TO VERIFY COMPLIANCE WITH THE DETAILS SHOWN ON THE ERECTION DRAWINGS, SUCH AS BRACES, STIFFENERS, MEMBER LOCATIONS AND PROPER APPLICATION OF JOINT DETAILS AT EACH CONNECTION.
- THE QAI SHALL BE ON THE PREMISES FOR INSPECTION DURING THE PLACEMENT OF ANCHOR RODS AND OTHER EMBEDMENTS SUPPORTING STRUCTURAL STEEL FOR COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS. AS A MINIMUM, THE DIAMETER, GRADE, TYPE AND LENGTH OF THE ANCHOR ROD OR EMBEDDED ITEM, AND THE EXTENT OR DEPTH OF EMBEDMENT INTO THE CONCRETE, SHALL BE VERIFIED PRIOR TO PLACEMENT OF THE CONCRETE.
- THE QAI SHALL INSPECT THE FABRICATED STEEL OR ERECTED STEEL FRAME, AS APPROPRIATE, TO VERIFY COMPLIANCE WITH THE DETAILS SHOWN ON THE CONSTRUCTION DOCUMENTS, SUCH AS BRACES, STIFFENERS, MEMBER LOCATIONS AND PROPER APPLICATION OF JOINT DETAILS AT EACH CONNECTION. QUALITY ASSURANCE (QA) INSPECTIONS, EXCEPT NONDESTRUCTIVE TESTING (NDT), MAY BE WAIVED WHEN THE WORK IS PERFORMED IN A FABRICATING SHOP OR BY AN ERECTOR APPROVED BY THE AUTHORITY HAVING JURISDICTION (AHJ) TO PERFORM THE WORK WITHOUT QA. NDT OF WELDS COMPLETED IN AN APPROVED FABRICATOR'S SHOP MAY BE PERFORMED BY THAT FABRICATOR WHEN APPROVED BY THE AHJ. WHEN THE FABRICATOR PERFORMS THE NDT, THE QA AGENCY SHALL REVIEW THE FABRICATOR'S NDT REPORTS.
- AT COMPLETION OF FABRICATION, THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE AHJ STATING THAT THE MATERIALS SUPPLIED AND WORK PERFORMED BY THE FABRICATOR ARE IN ACCORDANCE WITH THE CONSTRUCTION DOCUMENTS. AT COMPLETION OF ERECTION, THE APPROVED ERECTOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE AHJ STATING THAT THE MATERIALS SUPPLIED AND WORK PERFORMED BY THE ERECTOR ARE IN ACCORDANCE WITH THE CONSTRUCTION DOCUMENTS.
- IDENTIFICATION AND REJECTION OF MATERIAL OR WORKMANSHIP THAT IS NOT IN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS, SHALL BE PERMITTED AT ANY TIME DURING THE PROGRESS OF THE WORK. HOWEVER, THIS PROVISION SHALL NOT RELIEVE THE OWNER OR THE INSPECTOR OF THE OBLIGATION FOR TIMELY, IN-SEQUENCE INSPECTIONS. NONCONFORMING MATERIAL AND WORKMANSHIP SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE FABRICATOR OR ERECTOR, AS APPLICABLE.
-). NONCONFORMING MATERIAL OR WORKMANSHIP SHALL BE BROUGHT INTO CONFORMANCE, OR MADE SUITABLE FOR ITS INTENDED PURPOSE AS DETERMINED BY THE ENGINEER OF RECORD. . CONCURRENT WITH THE SUBMITTAL OF SUCH REPORTS TO THE AHJ, EOR OR OWNER, THE QA AGENCY SHALL SUBMIT TO THE FABRICATOR AND ERECTOR: (1) NONCONFORMANCE REPORTS (2) REPORTS OF REPAIR, REPLACEMENT OR ACCEPTANCE OF NONCONFORMING ITEMS.

		F:	STABLISHED PER 2021 IBC SE	CTION 110 AND CHAPTER 17			
ITEM	CONTINUOUS ³		REFERENCE	COMMENTS			
PRE-FAB CONSTRUCTION (IBC 1704.2)	CONTINUOUS	T ENGELO	REFERENCE NOTES P1 & P2	P1. SPECIAL INSPECTION IS NOT REQUIRED WHERE THE WORK IS DONE ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROTO PERFORM SUCH WORK WITHOUT SPECIAL INSPECTION, PROVIDED THE FABRICATOR COMPLIES WITH IBC. P2. INSPECTION FOR PREFABRICATED CONSTRUCTION SHALL BE THE SAME AS IF THE MATERIAL USED IN THE CONSTRUCTION TOOK PON SITE. SPECIAL INSPECTION WILL NOT BE REQUIRED DURING PREFABRICATION IF THE APPROVED AGENCY CERTIFIES THE CONSTRUCTION AND FURNISHES EVIDENCE OF COMPLIANCE. (SEE NOTE 2).			
OPEN-WEB STEEL JOISTS AND JOIST							
GIRDERS (IBC 1705.2.3)							
INSTALLATION OF OPEN-WEB JOISTS AND JOIST GIRDERS END CONNECTIONS - WELDED OR BOLTED		•	SJI SPECIFICATIONS LISTED IN SECTION 2207.1				
BRIDGING - HORIZONTAL OR DIAGONAL			COLOR EQUITORIORO EIGLES IN GEOTION 2201.1				
STANDARD BRIDGING		•					
BRIDGING THAT DIFFERS FROM THE SJI SPECIFICATIONS LISTED IN SECTION 2207.1		•	SJI SPECIFICATIONS LISTED IN SECTION 2207.1				
CONCRETE CONSTRUCTION (IBC 1705.3) REINFORCING STEEL PLACEMENT		•	SEE IBC TABLE 1705.3 - REF. NOTE C1	C 1. SPECIAL INSPECTION IS NOT REQUIRED FOR CONC. ISOLATED SPREAD FOOTINGS, CONTINUOUS FOOTINGS, NON-STRUCTURAL SLIF FOUNDATION WALLS, PATIOS, DRIVEWAYS, AND SIDEWALKS PROVIDED THE REQUIREMENTS OF IBC 1705.3 ARE MET. C 2. PERIODIC SPECIAL INSPECTION IS ALLOWED FOR VERIFICATION OF THE WELDABILITY OF REINFORCING STEEL RESISTING FLEXURA			
WELDING OF REINFORCING STEEL	•	•	REFERENCE NOTE C2	AXIAL FORCES IN INTERMEDIATE AND SPECIAL MOMENT FRAMES, BOUNDARY ELEMENTS OF SPECIAL REINFORCED CONCRETE SH			
ANCHORS CAST IN CONCRETE	•			WALLS, AND SHEAR REINFORCEMENT. PERIODIC SPECIAL INSPECTION IS ALLOWED FOR WELDING OF OTHER ASTM A 706 REINFOR STEEL NOT INCLUDED IN THE CONTINUOUS SPECIAL INSPECTION REQUIREMENTS NOTED ABOVE.			
VERIFYING REQUIRED DESIGN MIX		•		C 3. PERFORM AIR, SLUMP AND TEMP. TESTS WHEN CONCRETE SAMPLES ARE CAST. C 4. PERIODIC SPECIAL INSPECTION IS REQUIRED FOR VERIFICATION OF IN-SITU CONCRETE STRENGTH PRIOR TO STRESSING OF TEND			
CONCRETE PLACEMENT / SAMPLING	•		REFERENCE NOTE C3	POST-TENSIONED CONCRETE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.			
CURING TEMPERATURE / TECHNIQUES CONCRETE AND SHOTCRETE PLACEMENT / APPLICATION TECHNIQUES	•	•		C 5. EPOXY AND EXPANSION ANCHORS INTO MASONRY OR CONCRETE MAY BE USED ONLY WHEN APPROVED BY ARCHITECT. AND/OR ENGINEER USING AN APPROVED PRODUCT WITH CURRENT PUBLISHED ICC RESEARCH REPORT NUMBERS. COORDINATE CONTINUOUS/PERIODIC SPECIAL INSPECTION REQUIREMENTS WITH ICC REPORT AND ACI 318: 17.8.2.4. C 6. CONTINUOUS SPECIAL INSPECTION IS REQUIRED FOR PRECAST CONCRETE DIAPHRAGM CONNECTIONS OR REINFORCEMENT AT 3.			
VERIFICATION OF IN-SITU STRENGTH		•	REFERENCE NOTE C4	CLASSIFIED AS MODERATE OR HIGH DEFORMABILITY ELEMENTS IN STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY C, D,			
POST-INSTALLED ANCHOR PLACEMENT		•	REFERENCE NOTE C5	C 7. PERIODIC SPECIAL INSPECTION IS REQUIRED FOR THE INSTALLATION TOLERANCES OF PRECAST CONCRETE DIAPHRAGM CONNEC			
FORMWORK		•	REFERENCE NOTE C8	C 8. PERIODIC SPECIAL INSPECTION IS REQUIRED FOR FORMWORK SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER E FORMED.			
MASONRY CONSTRUCTION (IBC 1705.4)			SEE TMS 402/ACI 550 TABLE 1.19.2 (NON-ESSENTIAL)	PERIODIC SPECIAL INSPECTION IS ALLOWED FOR VERIFICATION OF THE WELDABILITY OF REINFORCING STEEL OTHER THAN ASTM A 706 IN ACCORDANCE WITH ANSI / AWS D1.4. CONTINUOUS SPECIAL INSPECTION IS REQUIRED FOR REINFORCING STEEL RESISTING			
AS MASONRY CONSTRUCTION BEGINS, VERIFY:			,	FLEXURAL AND AXIAL FORCES IN INTERMEDIATE AND SPECIAL MOMENT FRAMES, BOUNDARY ELEMENTS OF SPECIAL REINFORCEI CONCRETE SHEAR WALLS, AND SHEAR REINFORCEMENT. PERIODIC SPECIAL INSPECTION IS ALLOWED FOR WELDING OF OTHER			
SITE PREPARED MORTAR		•		ASTM A 706 REINFORCING STEEL NOT INCLUDED IN THE CONTINUOUS SPECIAL INSPECTION REQUIREMENTS NOTED ABOVE.			
MORTAR JOINTS		•		M2. CONTINUOUS SPECIAL INSPECTION IS REQUIRED FOR ESSENTIAL FACILITIES (TMS 602-16/ACI 530.1 TABLE 3). M3. EPOXY AND EXPANSION ANCHORS INTO MASONRY OR CONCRETE MAY BE USED ONLY WHEN APPROVED BY ARCHITECT AND/OR			
REINFORCEMENT / CONNECTORS		•		ENGINEER USING AN APPROVED PRODUCT WITH CURRENT PUBLISHED ICC RESEARCH REPORT NUMBERS. COORDINATE CONTINUOUS/PERIODIC SPECIAL INSPECTION REQUIREMENTS WITH ICC REPORT.			
PRE-STRESSING TECHNIQUES GRADE & SIZE OF TENDONS & ANCHORAGES		•		CONTINUOSOS/I ENIODIO DI ESIME INCI ESTICIVILE WITH IOCI NEI CIVI.			
INSPECTION SHALL VERIFY: SIZE & LOCATION OF STRUCTURAL ELEMENTS		•					
TYPE, SIZE, & LOCATION OF STRUCTURAL ELEMENTS TYPE, SIZE, & LOCATION OF ANCHORS		•	REFERENCE NOTE M2	\dashv			
SIZE, GRADE & TYPE OF REINFORCEMENT		•	INCI ENCINCE NOTE WZ				
WELDING OF REINFORCING BARS	•		REFERENCE NOTE M1				
HOT OR COLD WEATHER PROTECTION MEASUREMENT OF PRE-STRESSING FORCE	-	•	REFERENCE NOTE M2				
PRIOR TO GROUTING, VERIFY:							
CLEAN GROUT SPACE		•	REFERENCE NOTE M2				
PLACEMENT OF REINFORCEMENT CONNECTORS, TENDONS AND ANCHORS.		•					
PROPORTIONS OF SITE PREPARED GROUT CONSTRUCTION OF MORTAR JOINTS		•					
GROUT PLACEMENT	•						
PREPARATION OF TEST SPECIMENS / PRISMS	•						
COMPLIANCE W/ CONST. DOCS. / SUBMITTALS EPOXY / EXPANSION ANCHOR PLACEMENT	_	•	REFERENCE NOTE M3				
VERIFICATION OF fm AND faac		•	INCI LINCHINOL INOTE IVIO				
SELF CONSOLIDATING GROUT: VERIFY SLUMP FLOW AND VSI	•						
SOILS (IBC 1705.6)			REFERENCE NOTE F1	F 1. SPECIAL INSPECTION OF SOILS SHALL REFERENCE THE APPROVED GEOTECHNICAL REPORT TO DETERMINE COMPLIANCE.			
VERIFY ADEQUATE MATERIALS BELOW FOOTINGS		•	REFERENCE NOTE F1	F 2. WHERE GEOTECHNICAL REPORT IS NOT PROVIDED SPECIAL INSPECTIONS ARE REQUIRED TO VERIFY THAT THE IN-PLACE DRY			
EXCAVATIONS EXTEND TO PROPER DEPTH AND REACH PROPER MATERIAL		•	REFERENCE NOTE F2	DENSITY OF THE COMPACTED FILL IS NOT LESS THAN 90 PERCENT OF THE MAXIMUM DRY DENSITY AT OPTIMUM MOISTURE CONTENT DETERMINED IN ACCORDANCE WITH ASTM D1557. F 3. CONTINUOUS SPECIAL INSPECTION IS REQUIRED DURING FILL PLACEMENT. VERIFY USE OF PROPER MATERIALS AND PROCEDURES IN ACCORDANCE WITH THE PROVISIONS OF THE APPROVED CECTECHNICAL PERPORT. VERIFY DENSITIES AND			
CLASSIFY & TEST CONTROLLED FILL MATERIALS		•	REFERENCE NOTE F2	PROCEDURES IN ACCORDANCE WITH THE PROVISIONS OF THE APPROVED GEOTECHNICAL REPORT. VERIFY DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF COMPACTED FILL.			
FILL MATERIAL AND PLACEMENT	•		REFERENCE NOTE F3				
PROPERLY PREPARED SITE AND SUB-GRADE PRIOR TO FILL.		•	REFERENCE NOTE F1				

THE ITEMS MARKED WITH A "O" IN THE SPECIAL INSPECTION SCHEDULE SHALL BE INSPECTED IN ACCORDANCE WITH IBC CHAPTER 17 BY A CERTIFIED SPECIAL INSPECTOR FROM AN ESTABLISHED TESTING AGENCY. FOR MATERIAL SAMPLING AND TESTING REQUIREMENTS, REFER TO THE MATERIAL SAMPLING AND TESTING SECTION, THE PROJECT SPECIFICATIONS, AND THE SPECIFIC GENERAL NOTES SECTIONS. THE TESTING AGENCY SHALL SEND COPIES OF ALL STRUCTURAL TESTING AND INSPECTION REPORTS DIRECTLY TO THE ARCHITECT, ENGINEER, CONTRACTOR, AND BUILDING OFFICIAL. ANY ITEMS WHICH FAIL TO COMPLY WITH THE APPROVED CONSTRUCTION DOCUMENTS SHALL IMMEDIATELY BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL, ARCHITECT, AND ENGINEER PRIOR TO COMPLETION OF THAT PHASE OF WORK. SPECIAL INSPECTION TESTING REQUIREMENTS APPLY EQUALLY TO ALL BIDDER DESIGNED COMPONENTS. ANY CONSTRUCTION OR MATERIAL THAT HAS FAILED INSPECTION SHALL BE SUBJECT TO REMOVAL AND REPLACEMENT.

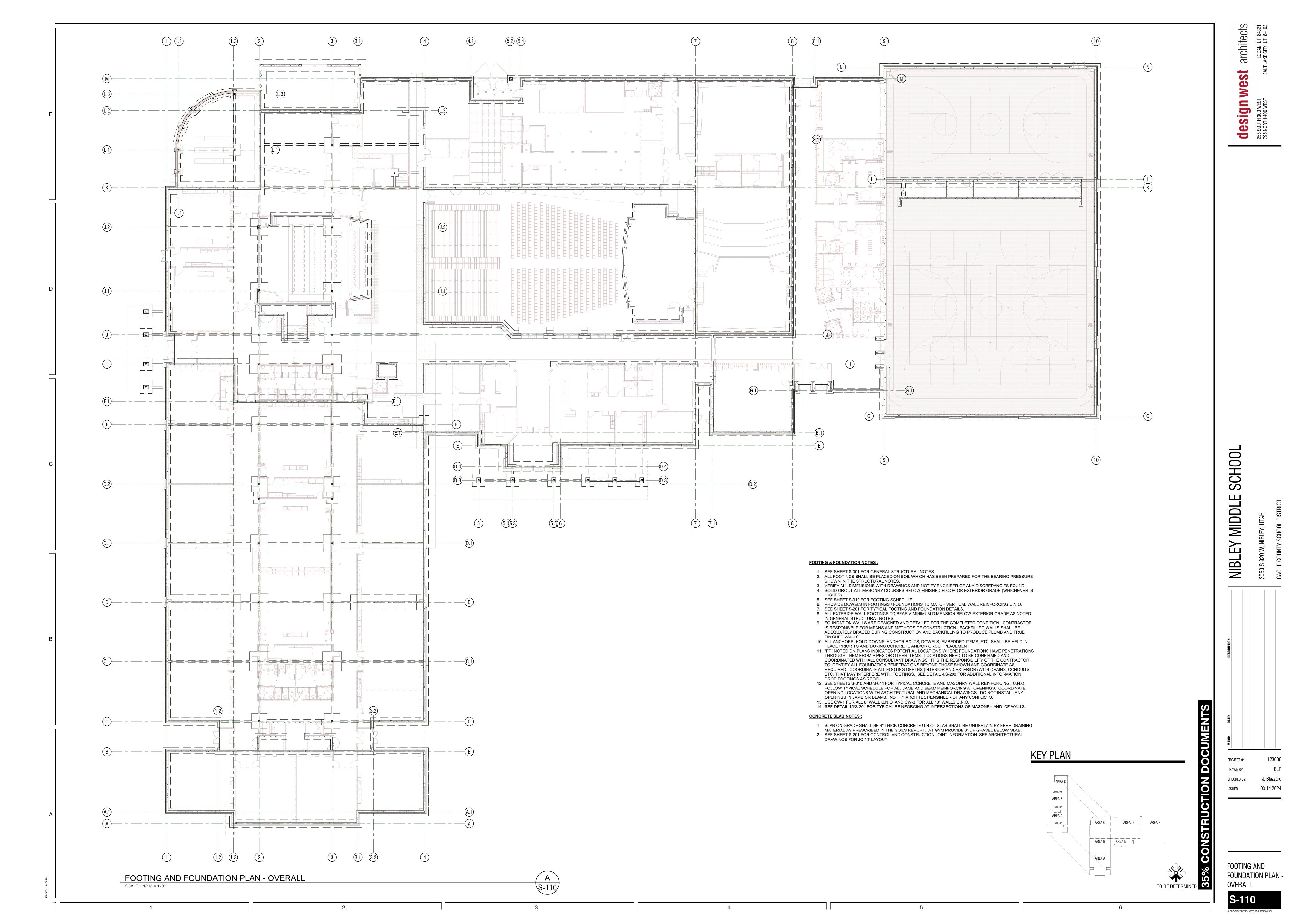
CONTINUOUS SPECIAL INSPECTION MEANS THE FULL-TIME OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED. PERIODIC SPECIAL INSPECTION MEANS THE PART-TIME OR INTERMITTENT OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK HAS BEEN OR IS BEING PERFORMED AND AT THE COMPLETION OF THE WORK. (IBC SECTION 202)

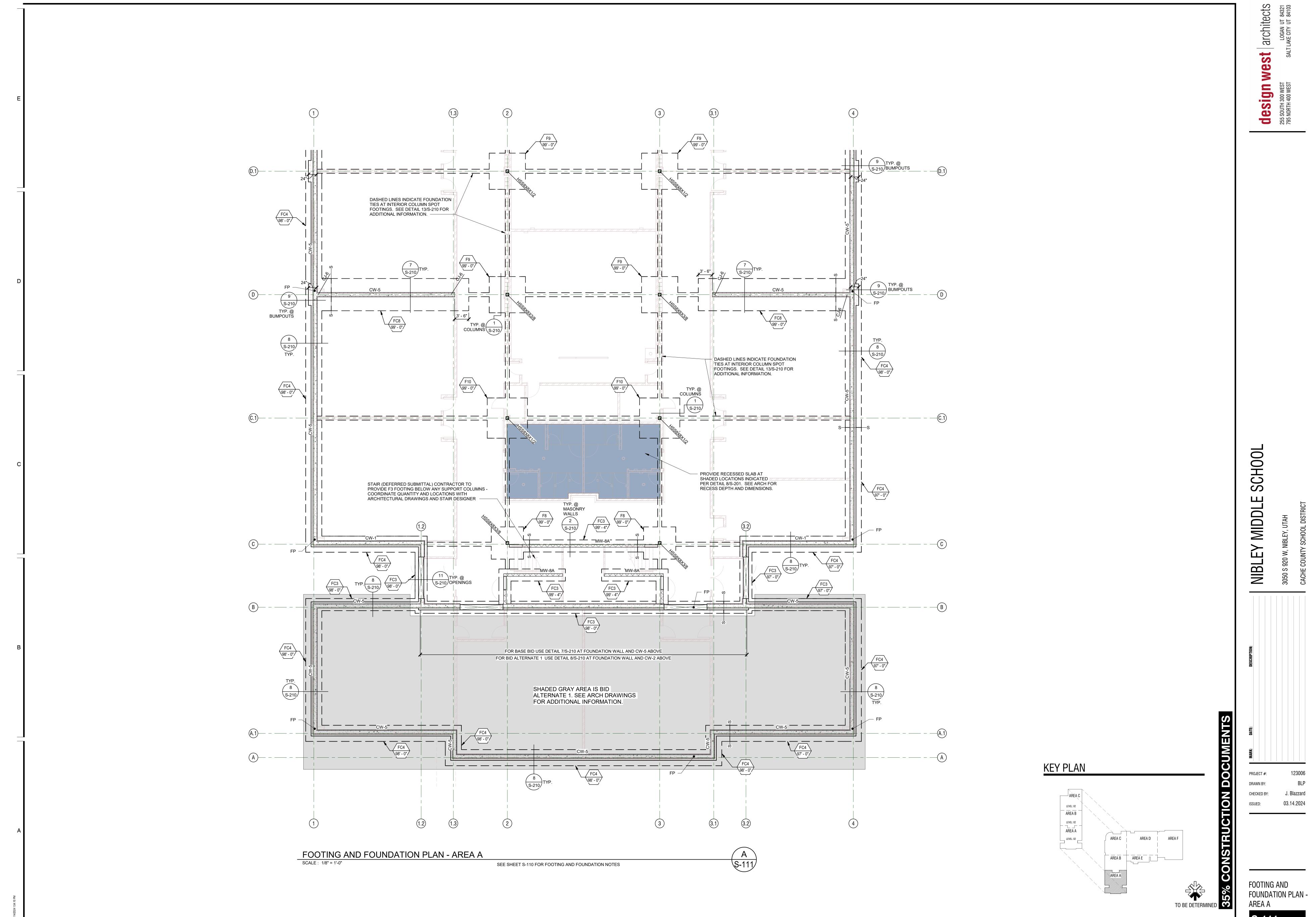
COLD-FORMED STEEL DECK SPECIAL INSPECTION SCHEDULE
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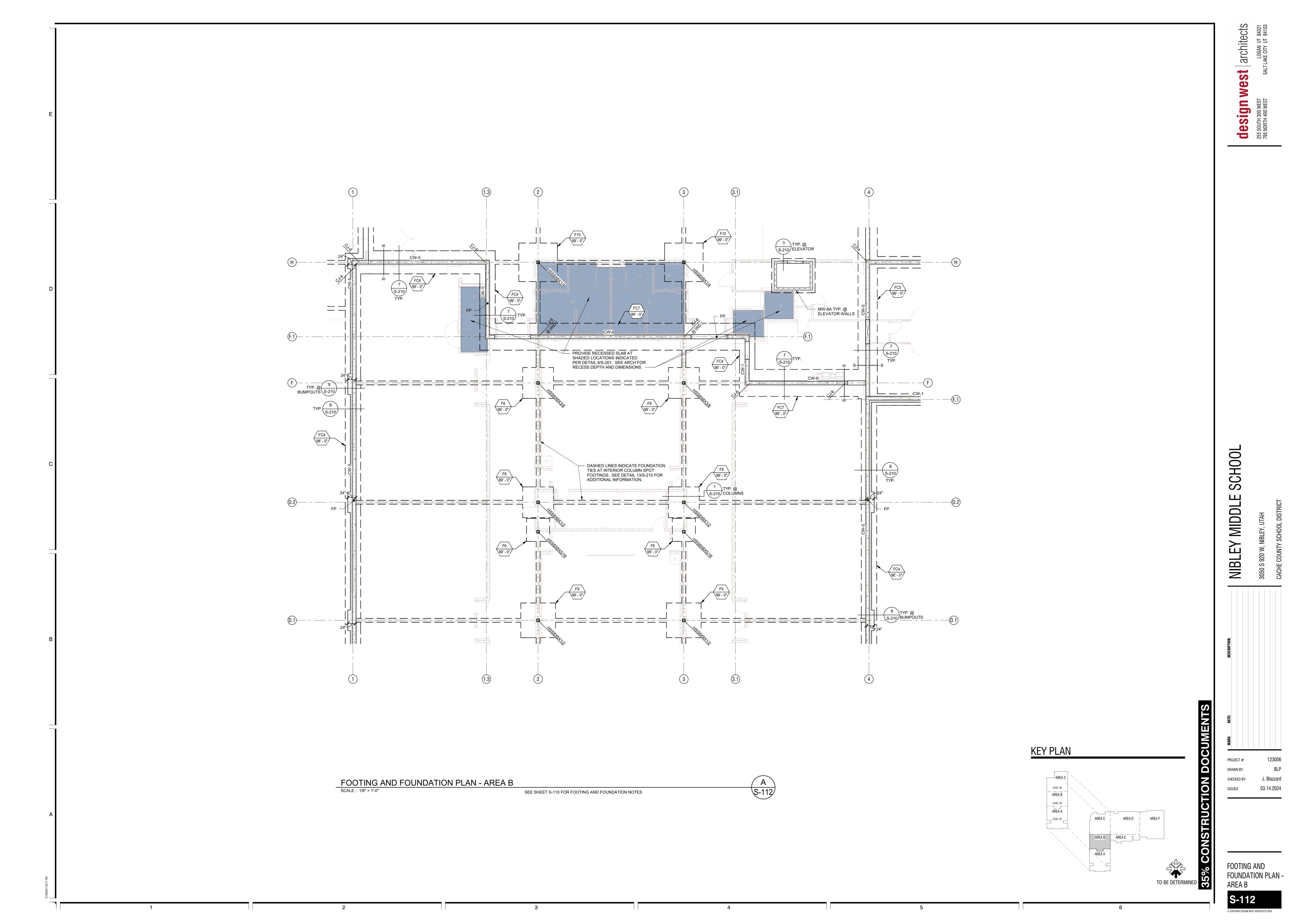
ESTABLIS	SHED PER 2021	I IBC SEC	TION 1705.2	2.2 AND S	SDI QA/QC	
INSPECTION TASKS PRIOR TO DECK PLACEMENT (TABLE 1.1)	INSTAI QUALITY O CONTINUOUS	ONTROL	SPECIAL INSPECTOR QUALITY ASSURANCE		<u>NOTES</u>	
		PERIODIC	CONTINUOUS	PERIODIC	1. PERIODIC - INSPECT THESE ITEMS ON AN INTERMITTENT BASIS. OPERATIONS NEE	
VERIFY COMPLIANCE OF MATERIALS (DECK AND ALL DECK ACCESSORIES) WITH CONSTRUCTION DOCUMENTS, INCLUDING PROFILES, MATERIAL PROPERTIES, AND BASE METAL THICKNESS	•		•		NOT BE DELAYED PENDING THESE INSPECTIONS. FREQUENCY OF OBSERVATIONS SHALL BE ADEQUATE TO CONFIRM THAT THE WORK HAS BEEN PERFORMED IN ACCORDANCE WITH THE APPLICABLE DOCUMENTS. ADDITIONAL INSPECTIONS	
DOCUMENT ACCEPTANCE OR REJECTION OF DECK AND DECK ACCESSORIES	•		•		SHALL BE PERFORMED TO DETERMINE THE EXTENT OF NON-CONFORMANCE.	
INSPECTION TASKS AFTER DECK PLACEMENT (TABLE 1.2)	CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC	 CONTINUOUS - PERFORM THESE TASKS PRIOR TO FINAL ACCEPTANCE FOR EACH ITEM OR ELEMENT. 	
VERIFY COMPLIANCE OF DECK AND ALL DECK ACCESSORIES INSTALLATION WITH CONSTRUCTION DOCUMENTS	•		•		3. WITHIN THE LISTED TASKS, "DOCUMENT" SHALL MEAN THE INSPECTOR SHALL PREPARE REPORTS OR OTHER APPROPRIATE WRITTEN DOCUMENTATION INDICATING THAT THE WORK HAS OR HAS NOT BEEN PERFORMED IN ACCORDANC WITH THE CONSTRUCTION DOCUMENTS.	
VERIFY DECK MATERIALS ARE REPRESENTED BY THE MILL CERTIFICATIONS THAT COMPLY WITH THE CONSTRUCTION DOCUMENTS	•		•		WITH THE CONSTRUCTION DOCUMENTS.	
DOCUMENT ACCEPTANCE OR REJECTION OF INSTALLATION OF DECK AND DECK ACCESSORIES	•		•			
INSPECTION TASKS PRIOR TO WELDING (TABLE 1.3)	CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC		
WELDING PROCEDURE SPECIFICATIONS (WPSs) AVAILABLE		•		•		
MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE		•		•		
MATERIAL IDENTIFICATION (TYPE/GRADE)		•		•		
CHECK WELDING EQUIPMENT		•		•		
INSPECTION TASKS DURING WELDING (TABLE 1.4)	CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC		
USE OF QUALIFIED WELDERS						
CONTROL AND HANDLING OF WELDING CONSUMABLES		•		•		
ENVIRONMENTAL CONDITIONS (WIND SPEED, MOISTURE, TEMPERATURE)		•		•		
WPS FOLLOWED		•		•		
INSPECTION TASKS AFTER WELDING (TABLE 1.5)	CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC		
VERIFY SIZE AND LOCATIONS OF WELDS, INCLUDING SUPPORT, SIDELAP, AND PERIMETER WELDS	•		•			
WELDS MEET VISUAL ACCEPTANCE CRITERIA	•		•			
VERIFY REPAIR ACTIVITIES	•		•			
DOCUMENT ACCEPTANCE OR REJECTION OF WELDS	•		•			
INSPECTION TASKS PRIOR TO MECHANICAL FASTENING (TABLE 1.6)	CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC		
MANUFACTURER INSTALLATION INSTRUCTIONS AVAILABLE FOR MECHANICAL FASTENERS		•		•		
PROPER TOOLS AVAILABLE FOR FASTENER INSTALLATION		•		•		
PROPER STORAGE FOR MECHANICAL FASTENERS		•		•		
INSPECTION TASKS DURING MECHANICAL FASTENING (TABLE 1.7)	CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC		
FASTENERS ARE POSITIONED AS REQUIRED		•		•		
FASTENERS ARE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS		•		•		
INSPECTION TASKS AFTER MECHANICAL FASTENING (TABLE 1.8)	CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC		
CHECK SPACING, TYPE, AND INSTALLATION OF SUPPORT FASTENERS	•		•			
CHECK SPACING, TYPE, AND INSTALLATION OF SIDELAP FASTENERS	•		•			
CHECK SPACING, TYPE, AND INSTALLATION OF PERIMETER FASTENERS	•		•			
VERIFY REPAIR ACTIVITIES	•		•			
DOCUMENT ACCEPTANCE OR REJECTION OF MECHANICAL FASTENERS	•		•			

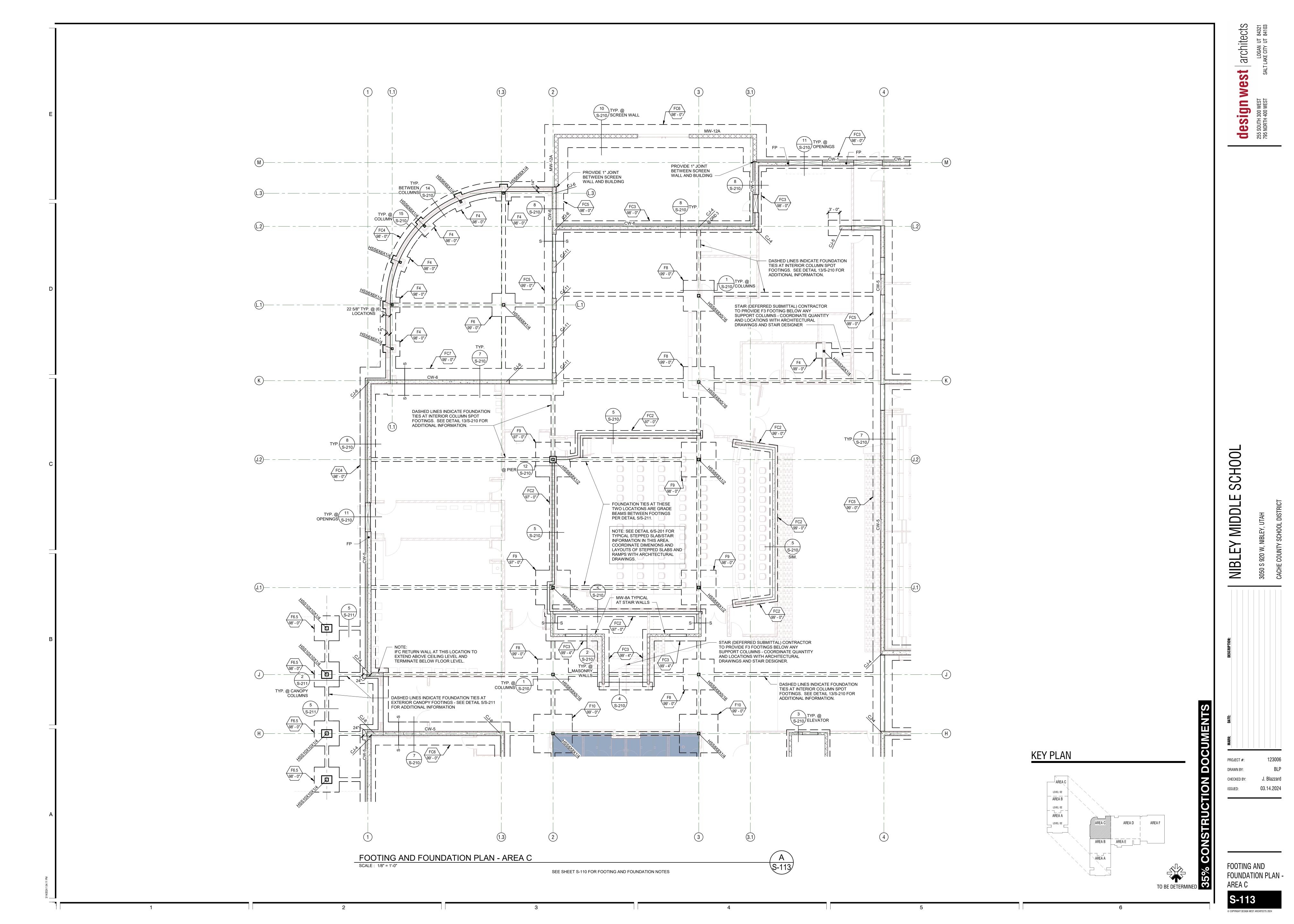
GENERAL STEEL DECK SPECIAL INSPECTION NOTES:

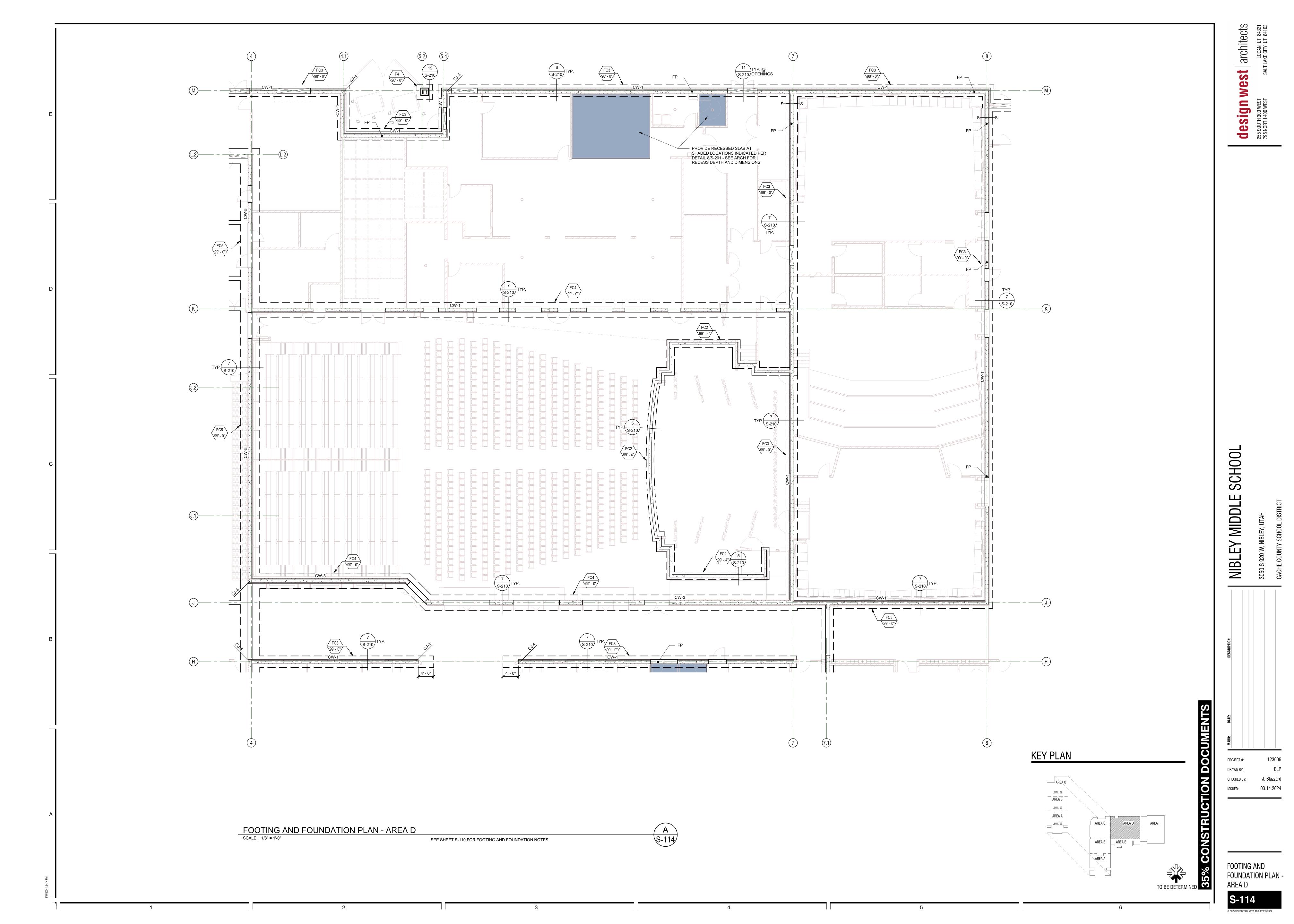
- FOR QUALITY CONTROL INSPECTION, THE CONSTRUCTION DOCUMENTS, INSTALLATION DRAWINGS, SHOP DRAWINGS, DESIGN DOCUMENTS AND THE APPLICABLE REFERENCED STANDARDS SHALL BE UTILIZED. QUALITY ASSURANCE INSPECTION OF THE DECK SHALL BE MADE AT THE PROJECT SITE. THE OWNER'S DESIGNATED REPRESENTATIVE FOR CONSTRUCTION SHALL SCHEDULE THIS WORK WITH THE QUALITY ASSURANCE INSPECTOR (QAI) AND
- THE INSTALLER TO MINIMIZE INTERRUPTIONS TO THE WORK OF THE INSTALLER. 4. THE QAI SHALL REVIEW THE MATERIALS TEST REPORTS AND CERTIFICATIONS LISTED IN SECTION 2.2 OF SDI QA/QC FOR COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS.
- 5. QUALITY ASSURANCE TASKS SHALL BE PERFORMED BY THE QAI. 6. WHERE A TASK IS TO BE PERFORMED BY BOTH QA AND QC, IT SHALL BE PERMITTED TO COORDINATE INSPECTION FUNCTIONS BETWEEN THE QCI AND QAI SO THAT THE INSPECTIONS ARE PERFORMED BY ONLY ONE PARTY WHEN APPROVED
- IN ADVANCE BY THE OWNER, DESIGNER, AND AHJ. WHEN QA TASKS ARE PERFORMED ONLY BY THE QCI, EACH INSPECTION IS TO BE DOCUMENTED IN A REPORT AND THE QAI SHALL PERIODICALLY REVIEW THE WORK OF THE QCI AT AN INTERVAL ACCEPTABLE TO THE OWNER, DESIGNER, AND THE AHJ.
- . IN THE EVENT THAT THE REQUIREMENTS OF THE CONSTRUCTION DOCUMENTS CONFLICT WITH THE INSTALLATION DRAWINGS OR SHOP DRAWINGS, THE REQUIREMENTS OF THE CONSTRUCTION DOCUMENTS SHALL GOVERN. 8. IDENTIFICATION AND REJECTION OF MATERIALS AND WORKMANSHIP NOT IN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS SHALL BE PERMITTED AT ANY TIME DURING PROGRESS OF OR FOLLOWING THE COMPLETION OF THE WORK. HOWEVER, THIS PROVISION SHALL NOT RELIEVE THE OWNER OR THE INSPECTOR OF THE OBLIGATION FOR TIMELY, IN-SEQUENCE INSPECTIONS. NONCOMFORMING MATERIAL OR WORKMANSHIP SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE OWNERS DESIGNATED REPRESENTATIVE FOR CONSTRUCTION AND THE DECK INSTALLER. NONCONFORMING MATERIAL OR WORKMANSHIP SHALL BE BROUGHT IN CONFORMANCE, OR MADE SUITABLE FOR ITS INTENDED PURPOSE AS DETERMINED BY THE DESIGNER.

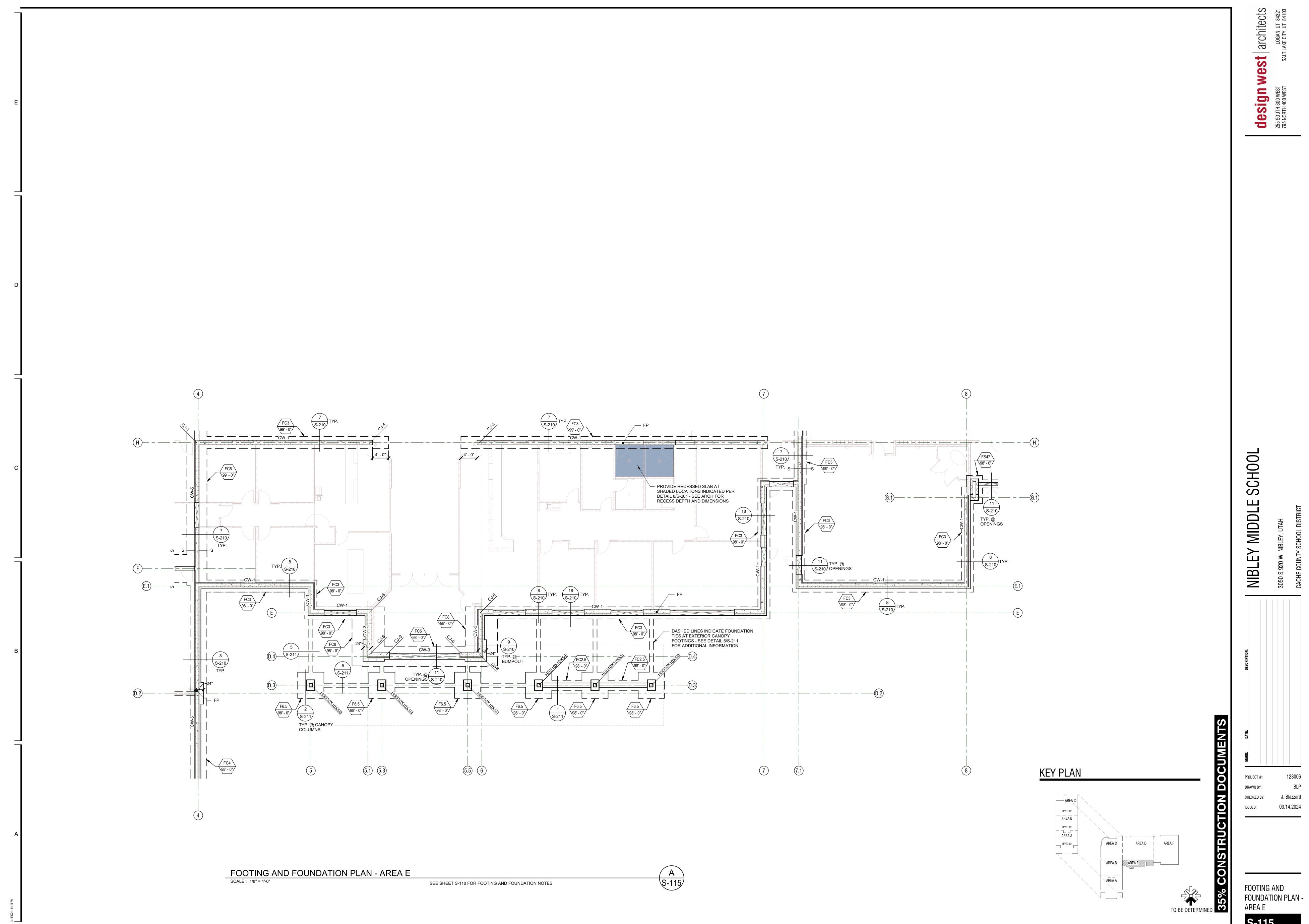


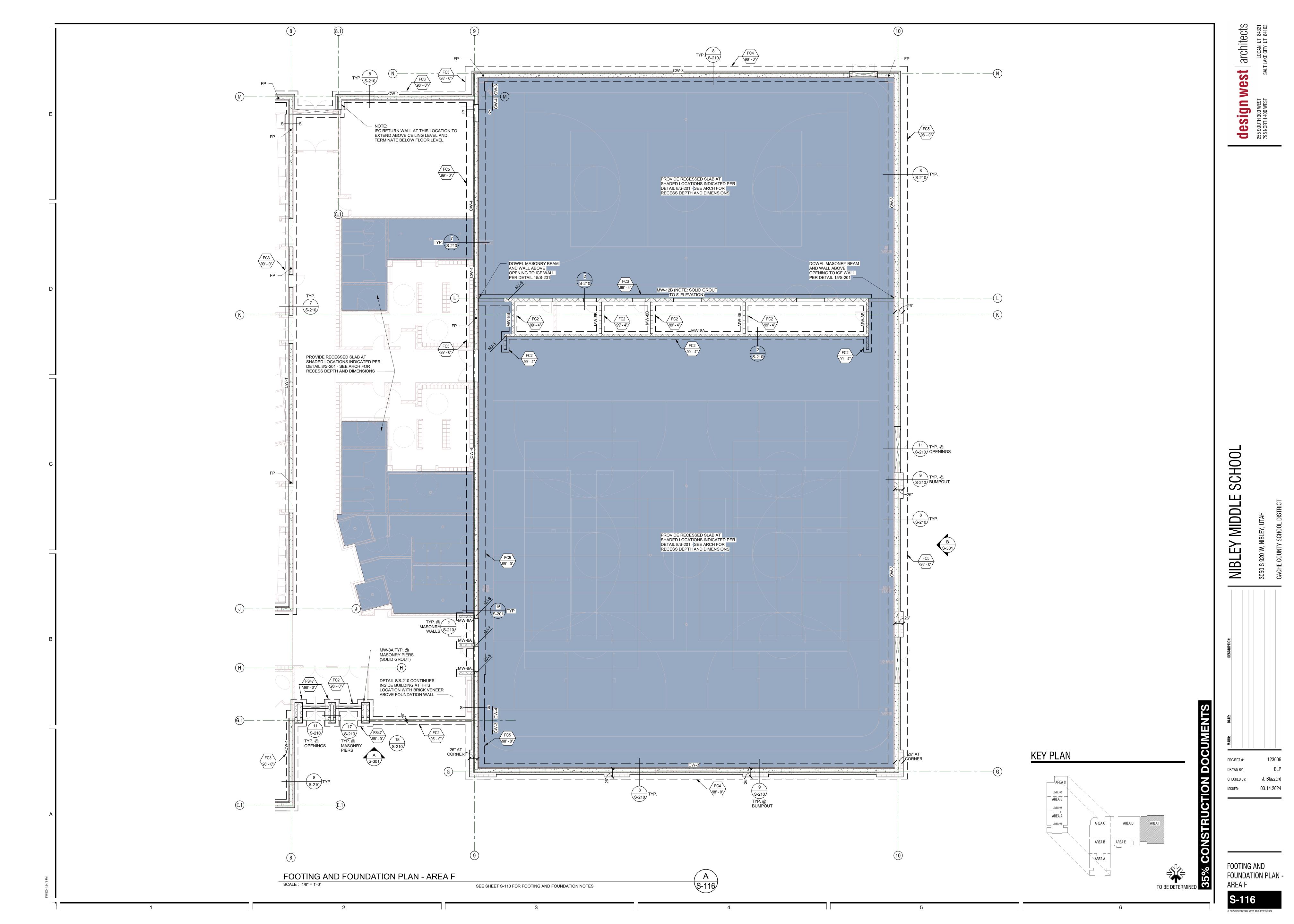


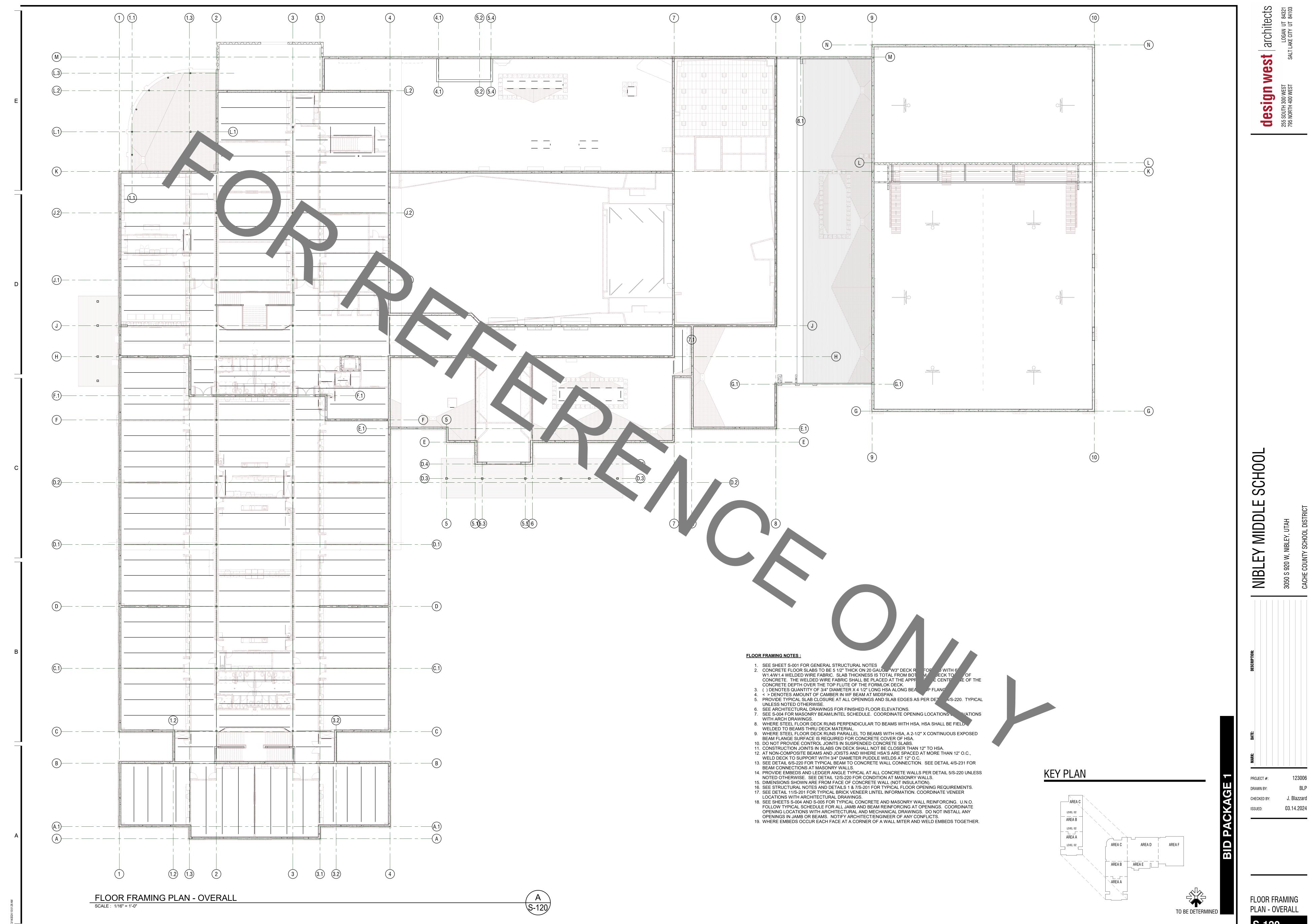


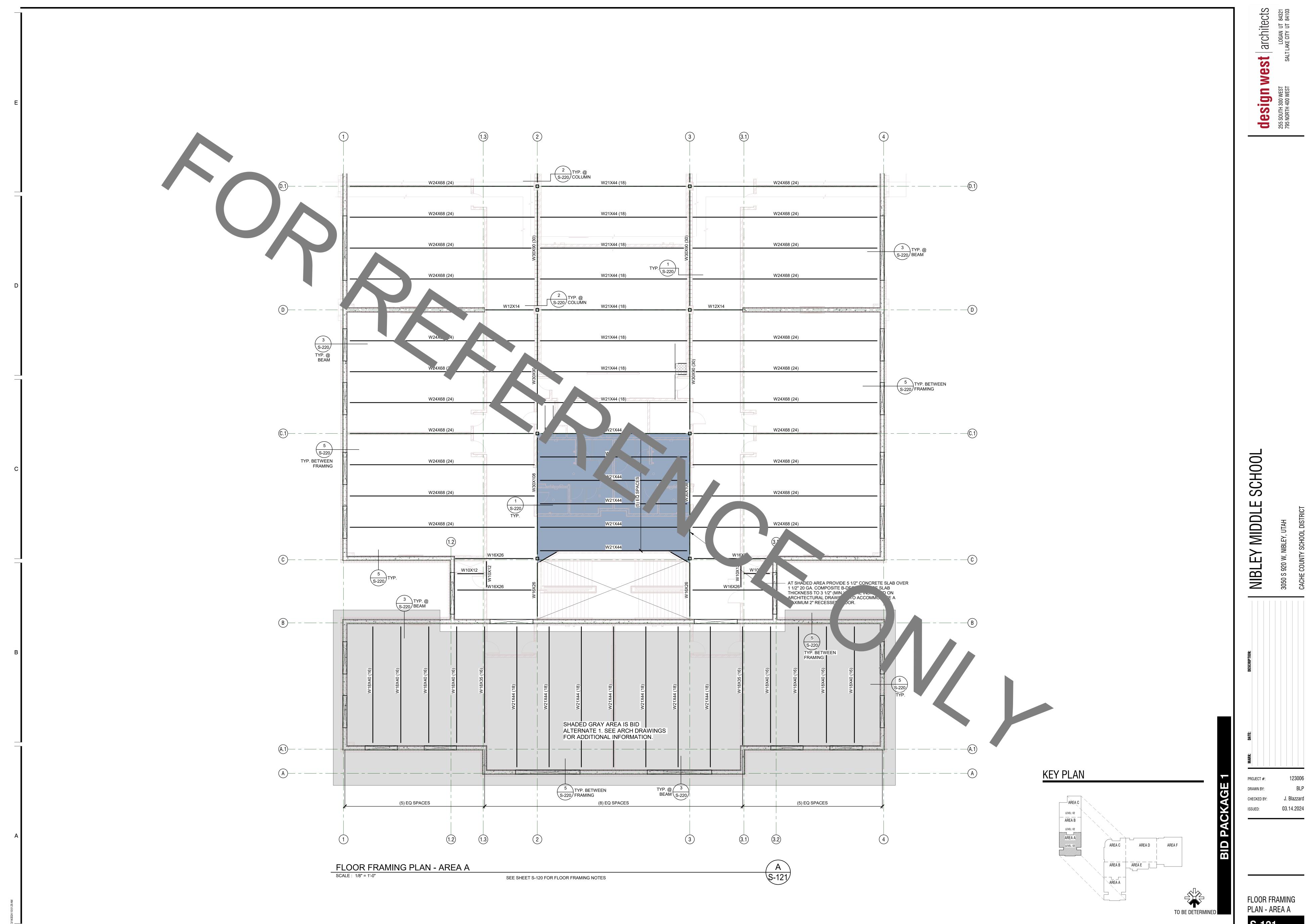


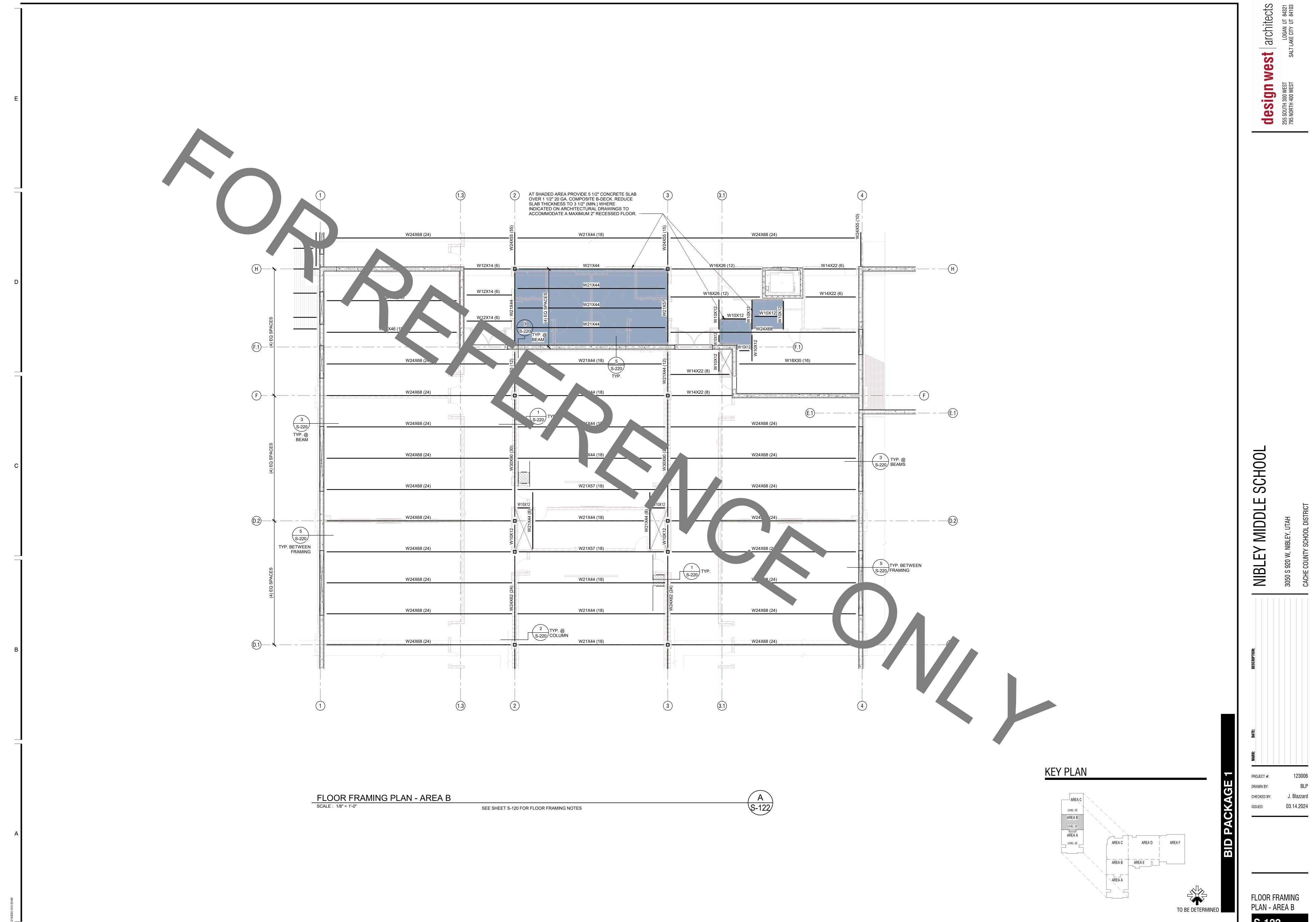


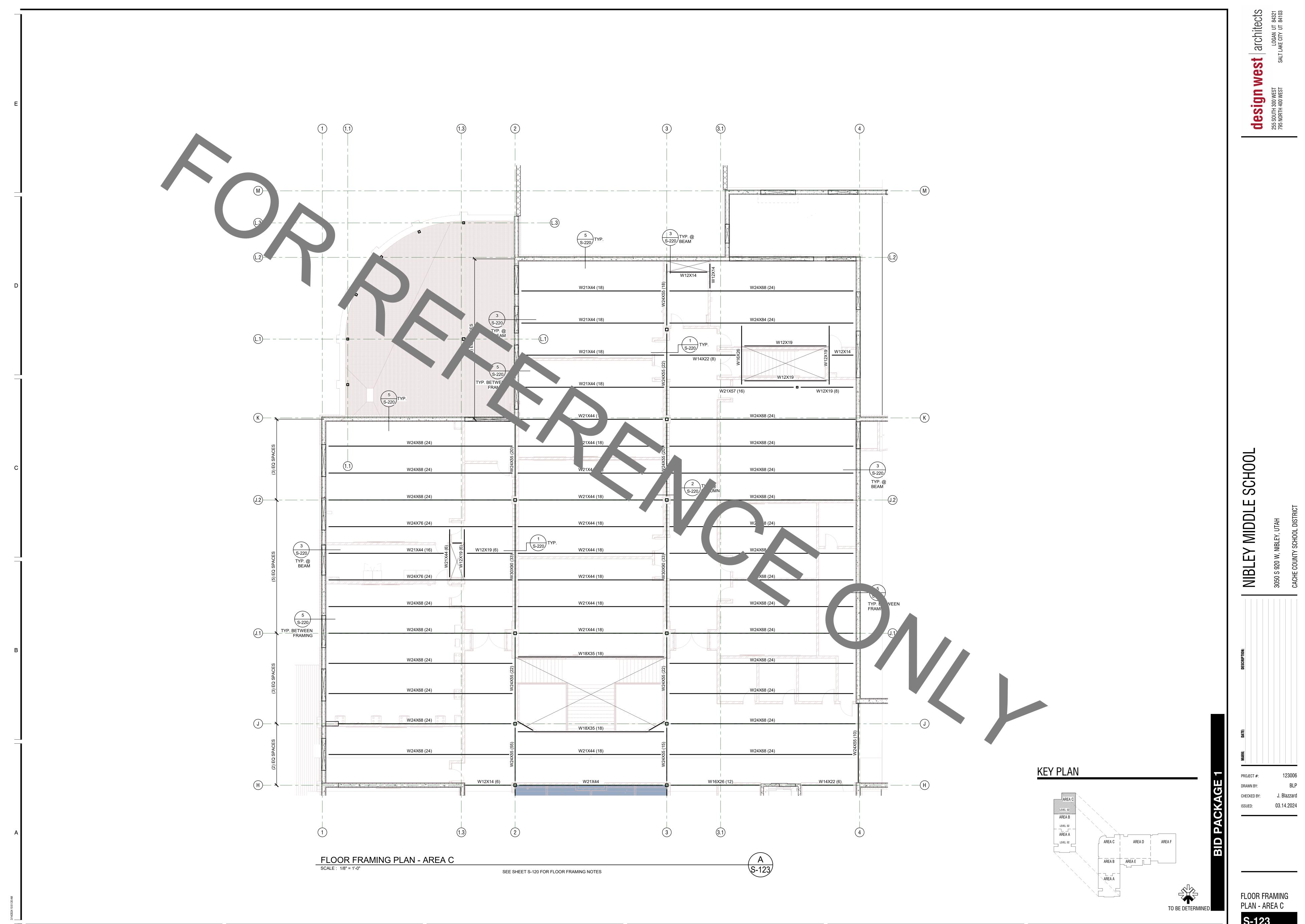


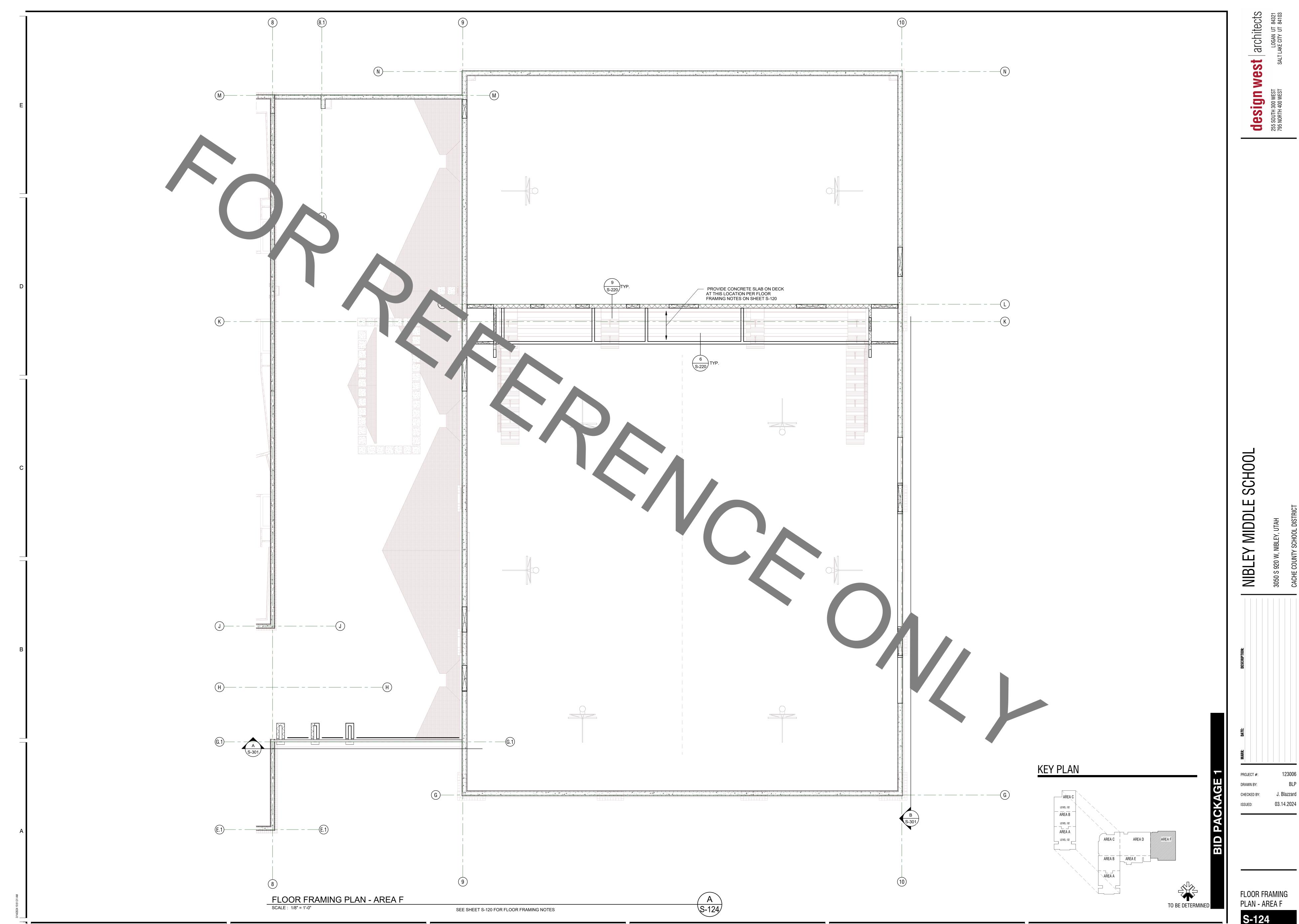


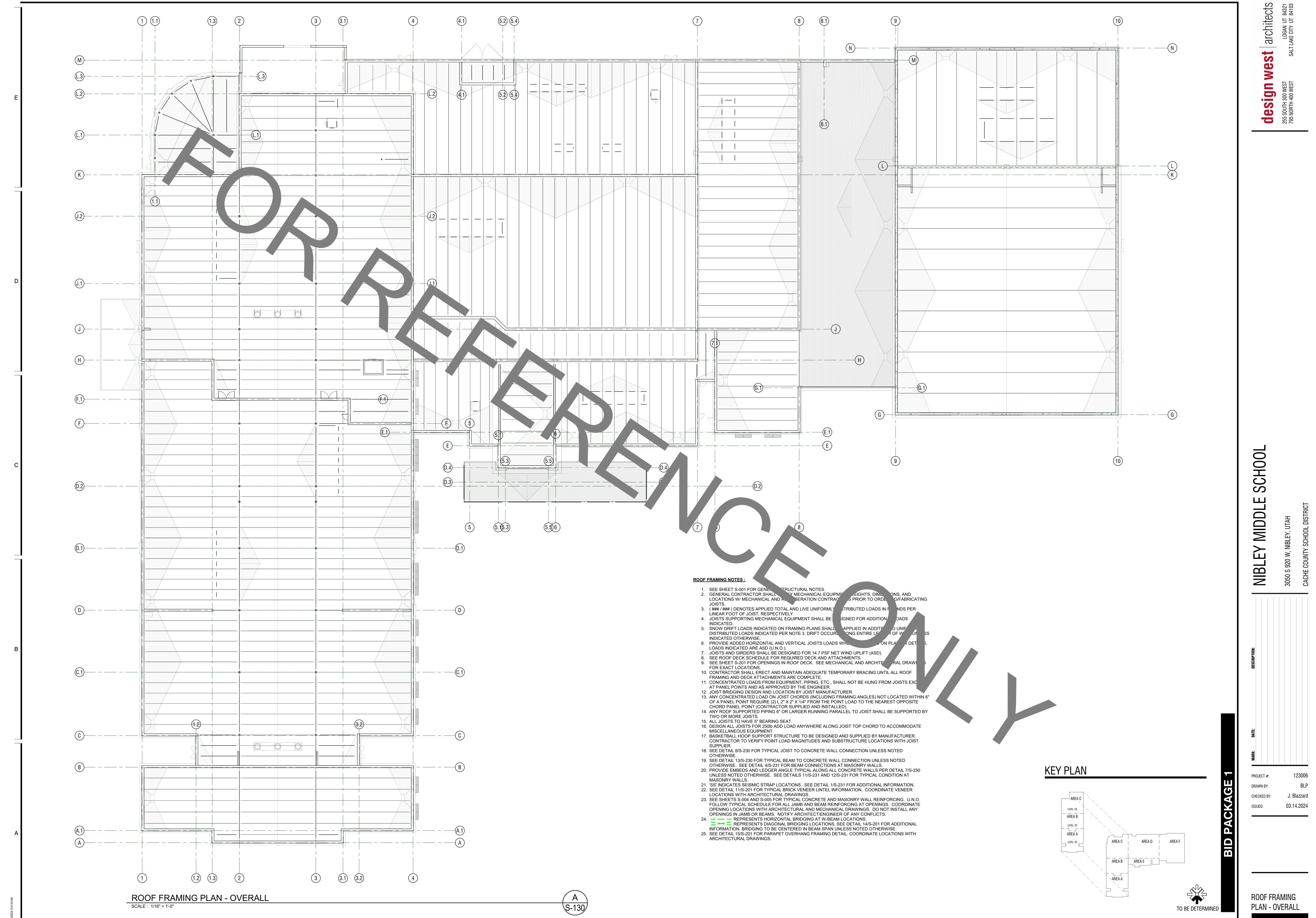


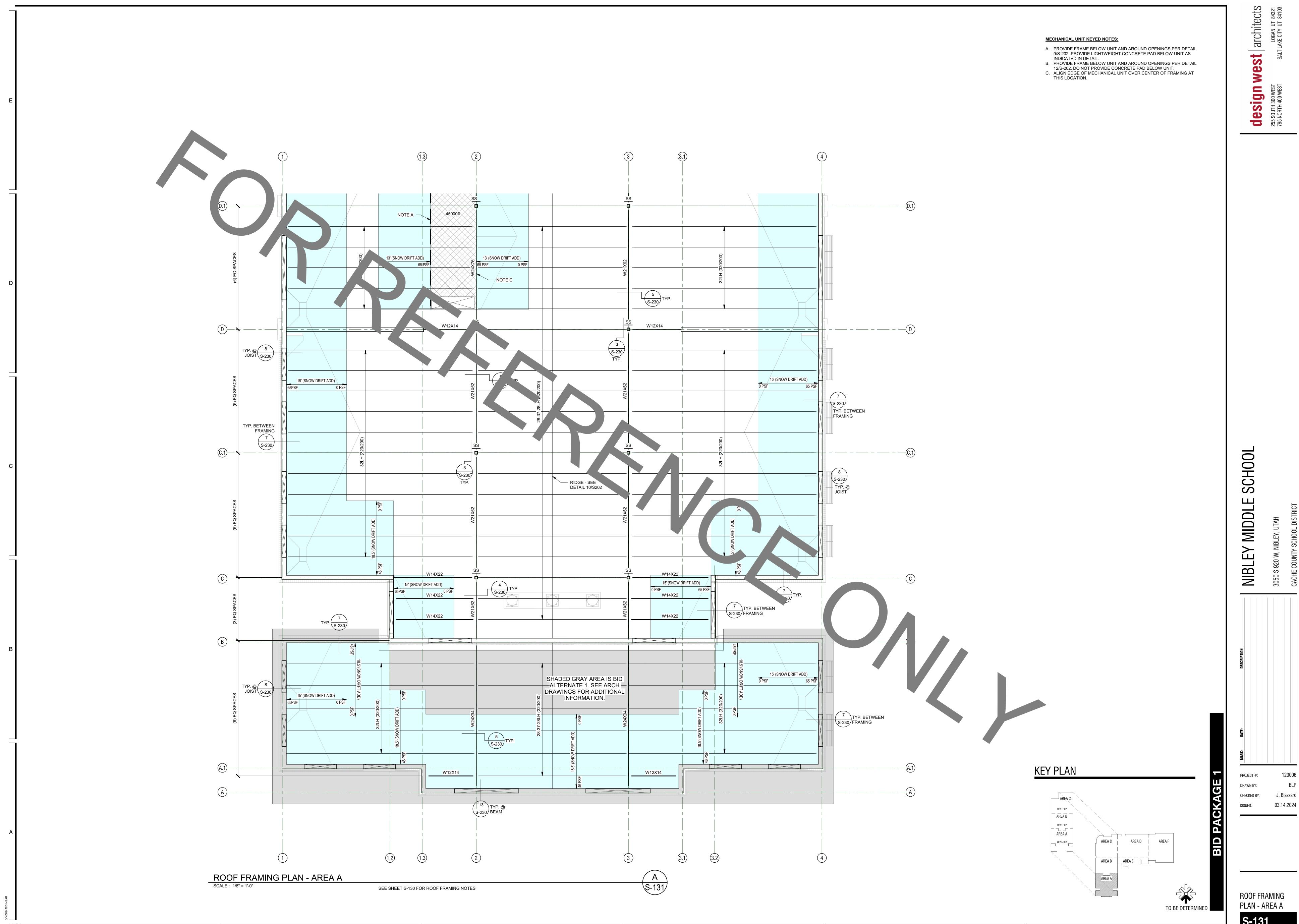


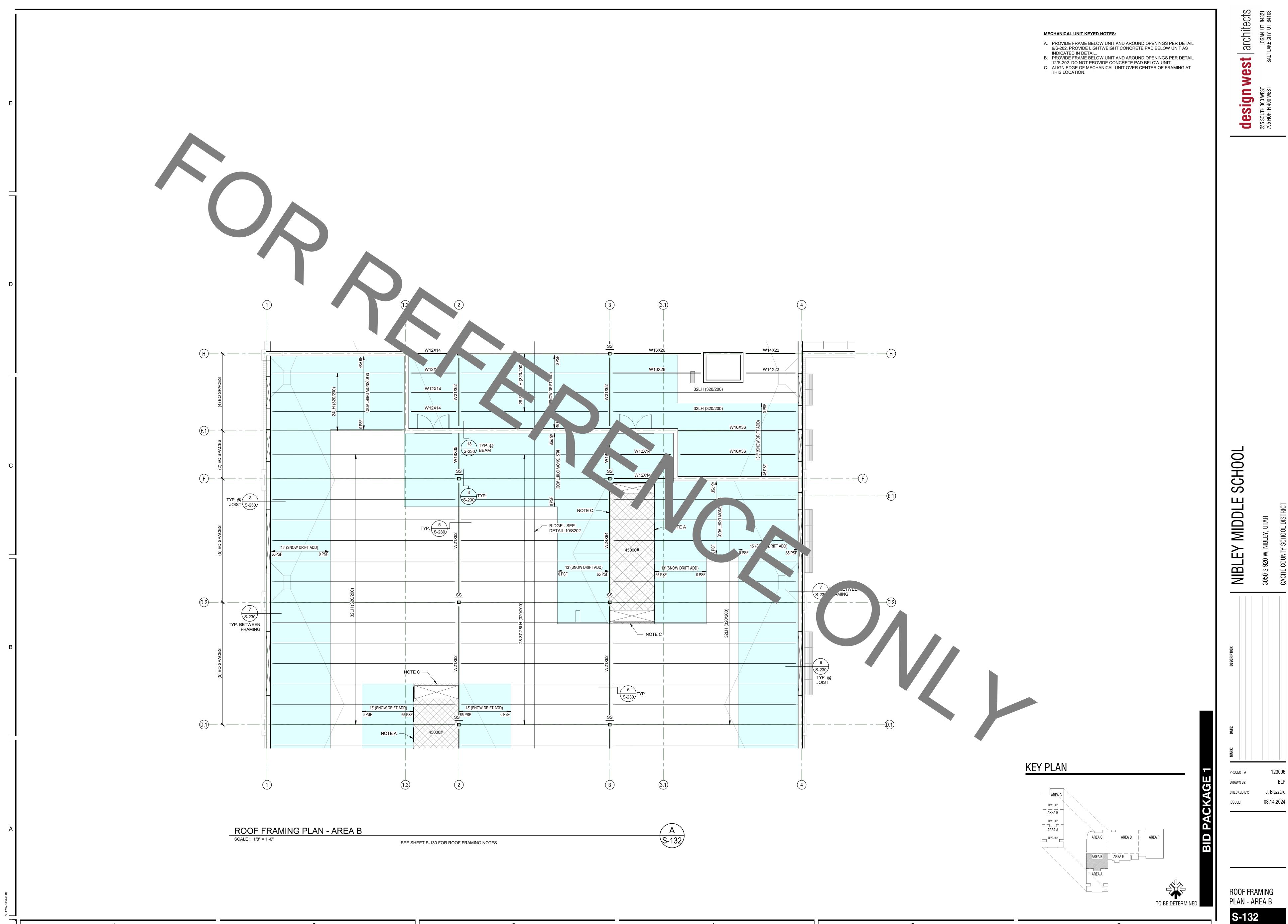




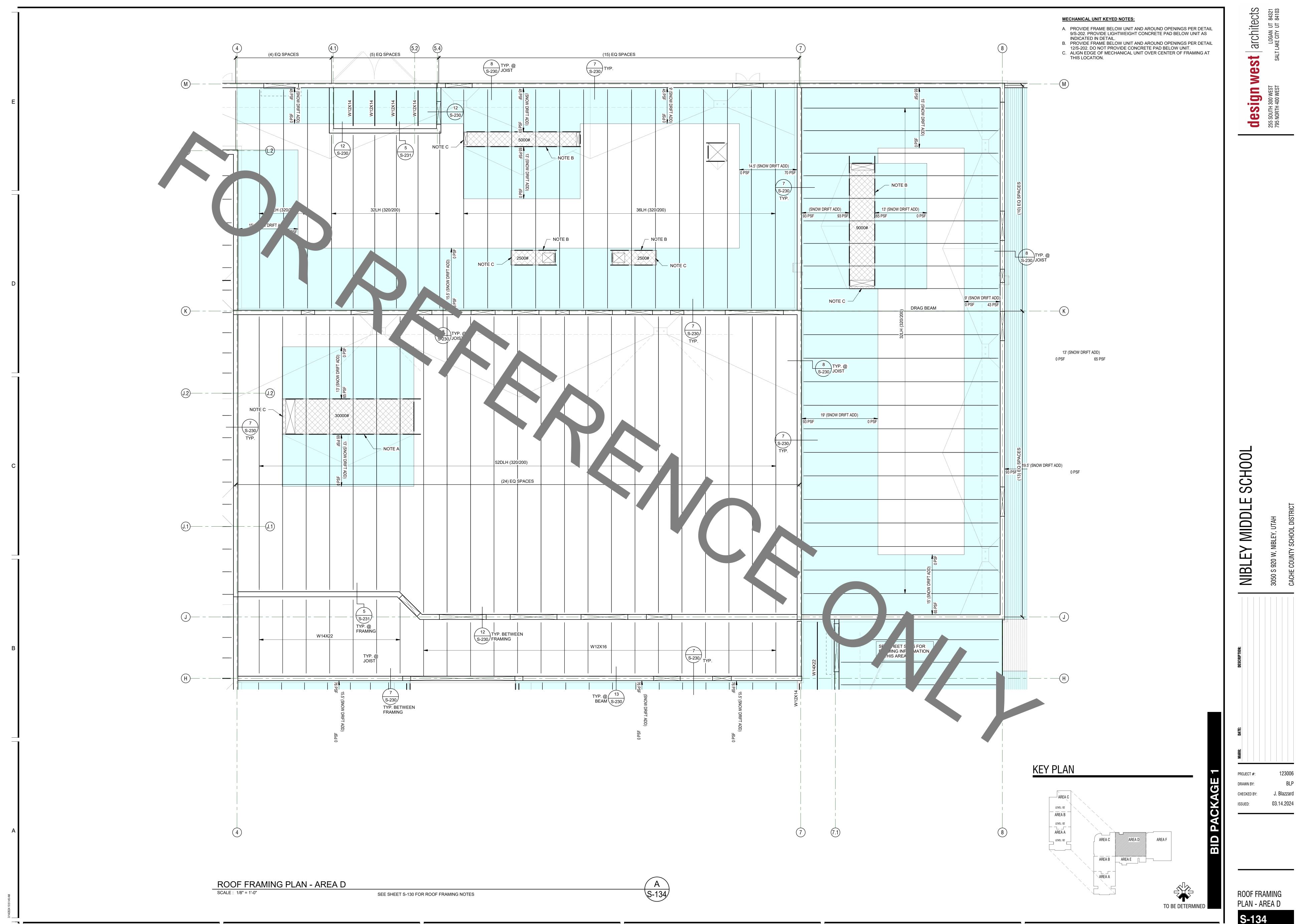


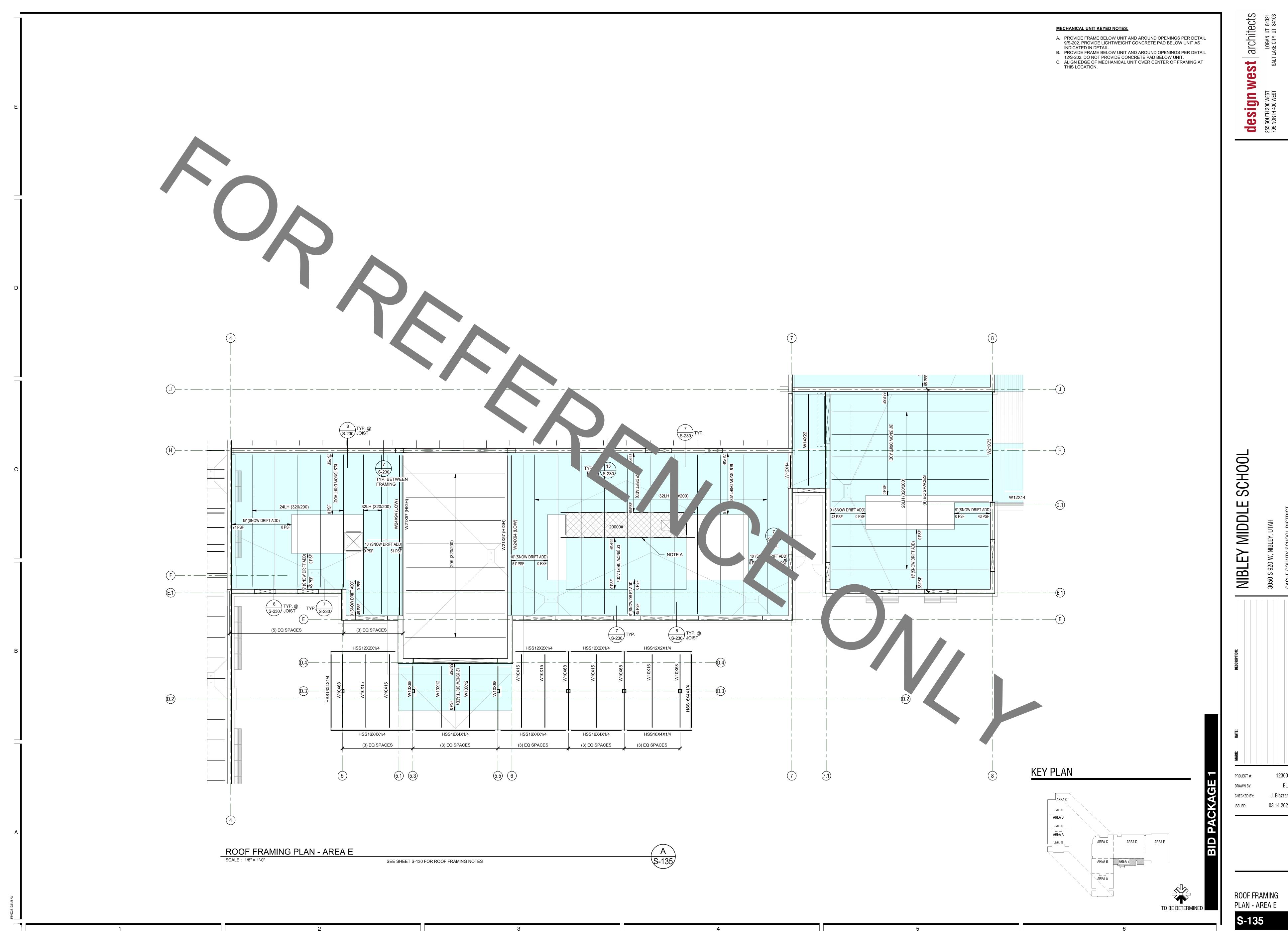




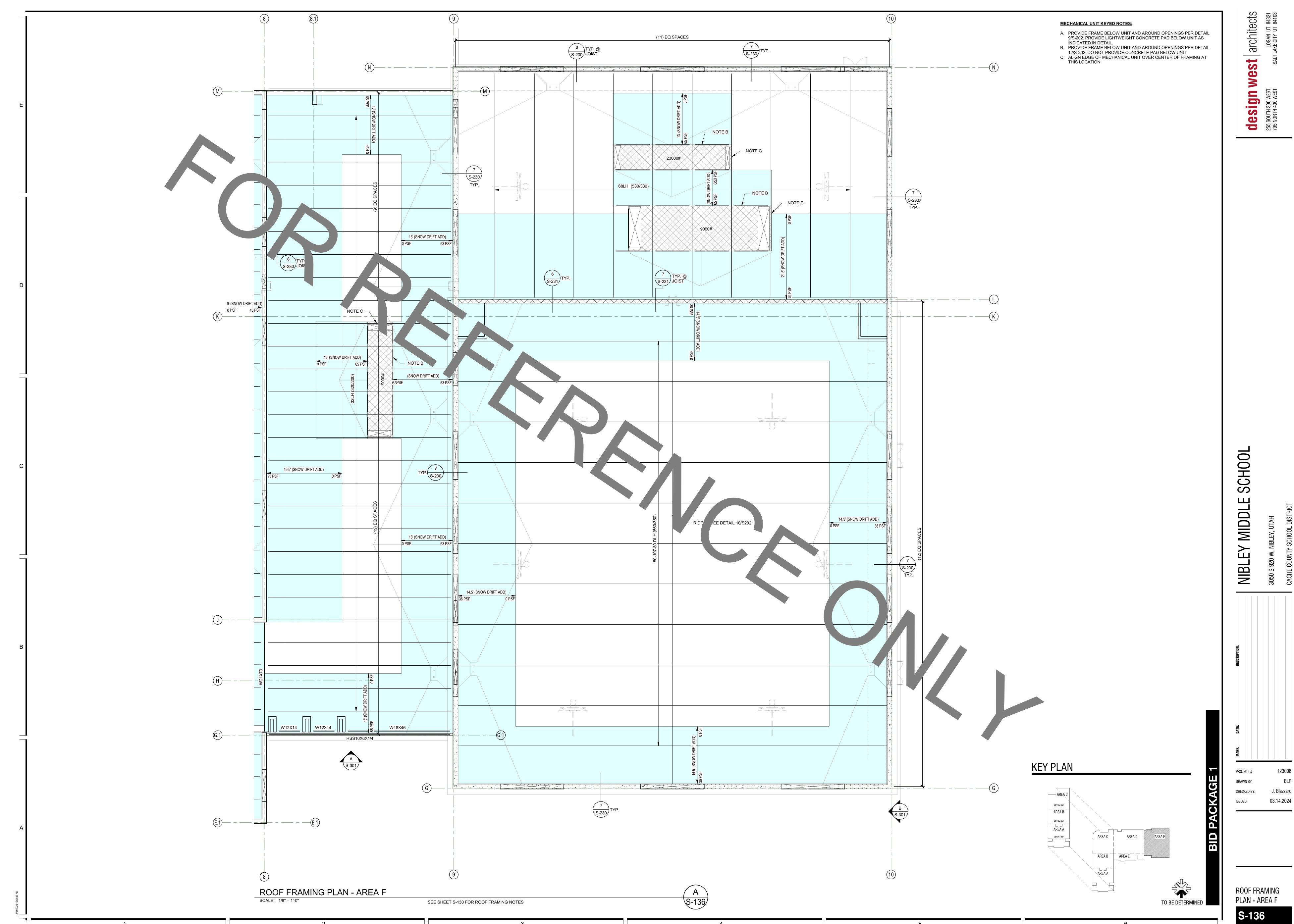








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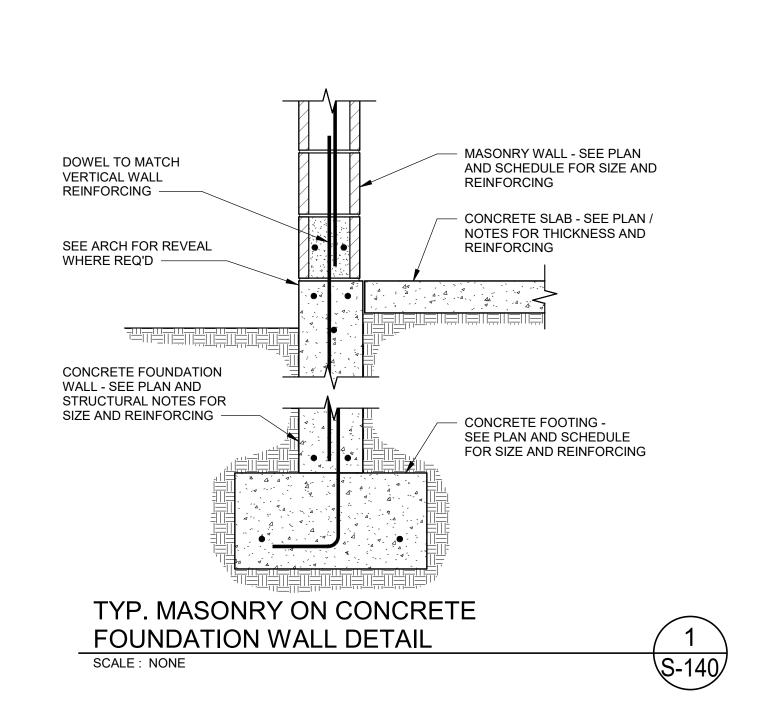
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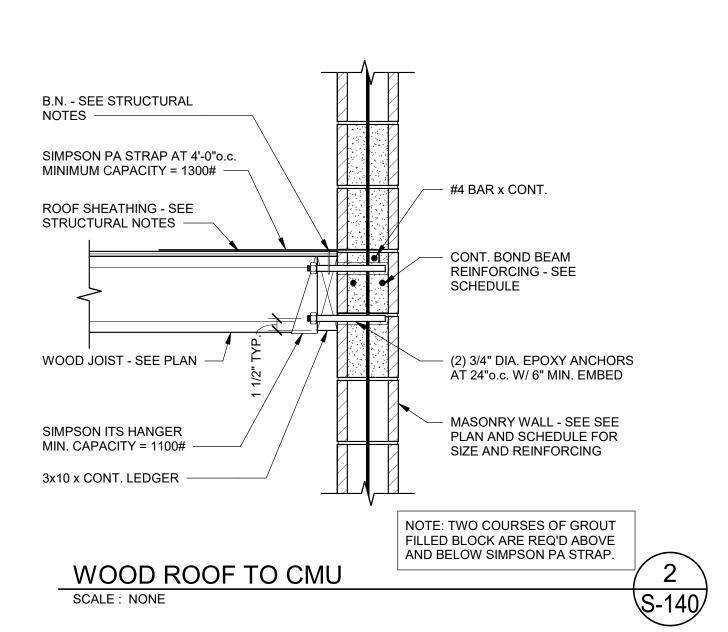
1. SEE SHEET S-001 FOR GENERAL STRUCTURAL NOTES. 2. SEE SHEET S-110 FOR FOOTING AND FOUNDATION NOTES.

WOOD ROOF NOTES

NOTES

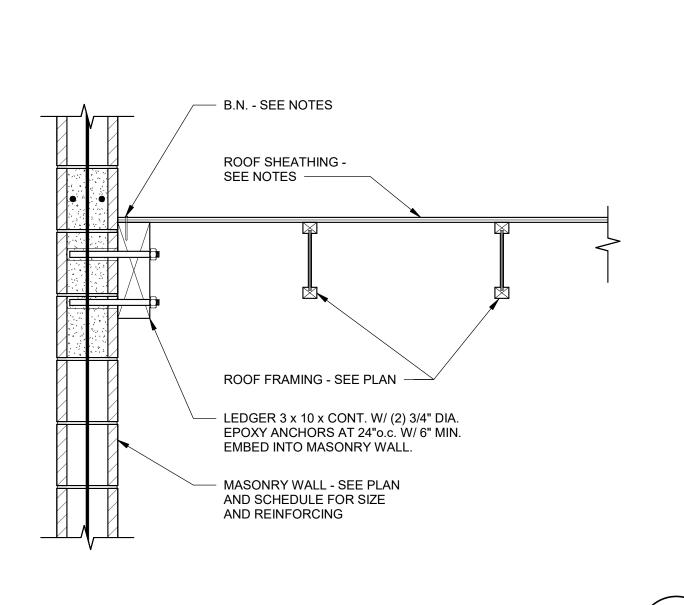
- 1. WOOD GRADES (UNLESS NOTED OTHERWISE) a. ALL FRAMING LUMBER SHALL BE DOUGLAS FIR/LARCH CLEARLY MARKED WITH A STAMP BY WWPA APPROVED AGENCY AND SHALL BE GRADED AS FOLLOWS: 1. HORIZONTAL MEMBERS: JOISTS & RAFTERS: NO. 2, BEAMS & STRINGERS: NO. 2. b. ALL WOOD "I" JOISTS AND BRIDGING SHALL BE FURNISHED BY TRUS-JOIST
- CORPORATION OR APPROVED EQUAL. 2. SHEATHING SHALL BE APA RATED SHEATHING, EXPOSURE I, EXTERIOR GLUE AND PANEL INDEX RATING AS NOTED BELOW UNLESS NOTED OTHERWISE: LOCATION THICKNESS PANEL INDEX
- ROOFS: 19/32" 32/16 3. INDIVIDUAL PIECES OF SHEATHING AT ROOF SHALL NOT BE SMALLER THAN 24" IN EITHER DIRECTION AND SHALL SPAN A MINIMUM OF TWO FRAMING SPACES, UNO.
- 4. CONNECTIONS, FASTENERS, AND ADHESIVE a. ALL BOLTS THRU WOOD SHALL BE ASTM A307 AND SHALL HAVE HARDENED
- WASHERS UNDER ASTM A563 HEAVY HEX NUT AND BOLT HEADS. b. UNLESS NOTED OTHERWISE, 10d COMMON (0.148) NAILS SHALL BE USED TO FASTEN ALL PLYWOOD ROOF SHEATHING TO SUPPORTING TRUSSES, JOISTS, LEDGERS OR BLOCKING AS FOLLOWS: 1. BOUNDARY NAILING "BN": 4"O.C. AT ALL BEARING WALLS, SHEAR WALLS,
- BLOCKING, AND WHERE OTHERWISE INDICATED IN THE STRUCTURAL DRAWINGS. 2. PANEL EDGE NAILING "EN": 6"O.C. AT ALL OTHER PLYWOOD PANEL EDGES. 3. PANEL FIELD NAILING "FN": 12"O.C. AT INTERIOR SUPPORTS IN FIELD OF PANEL. EXCEPT WHERE NOTED OTHERWISE, THE NUMBER AND SIZE OF NAILS CONNECTING WOOD MEMBERS SHALL NOT BE LESS THAN THAT SET FORTH IN IBC <u>TABLE 2304.10.1</u>. CONNECTIONS FOR MULTIPLE PIECES OF ENGINEERED LUMBER PIECES SHALL BE IN ACCORDANCE WITH THE MANUFACTURERS
- SPECIFICATIONS. c. UNLESS NOTED OTHERWISE, ALL NAILS SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES: COMMON SHANK HEAD LENGTH MIN. PENETRATION NAIL SIZE DIAMETER DIAMETER INTO SUPPORT MEMBER
 - 0.113" 0.266" 1.25" 0.131" 0.281" 2-1/2" 1.375" 0.148" 0.312" 1.50" 0.148" 0.312" 3-1/4" 0.162" 0.344"
- d. ALL FRAMING ANCHORS, POST CAPS, HOLD DOWNS, COLUMN BASES ETC. TO BE PROVIDED BY SIMPSON OR APPROVED EQUAL AND SHALL BE ATTACHED IN ACCORDANCE WITH MANUFACTURER'S PUBLISHED DATA, UNLESS NOTED OTHERWISE.
- e. FASTENERS CONNECTED TO OR IN CONTACT WITH PRESERVATIVE-TREATED AND/OR FIRE-RETARDANT-TREATED WOOD (EXCEPT FOR TIMBERSTRAND LSL TREATED LUMBER AND BORATE BASED TREATMENTS) SHALL BE OF G-185 HOT-DIP GALVANIZED STEEL OR 304 OR 316 STAINLESS STEEL. STAINLESS STEEL AND GALVANIZED STEEL SHALL NEVER BE USED IN CONTACT WITH EACH OTHER.





WOOD ROOF TO MASONRY SCALE: NONE

S-140



SPORTS STORAGE FOOTING, FDN, AND **ROOF FRAMING**

2 S-140 TYP.

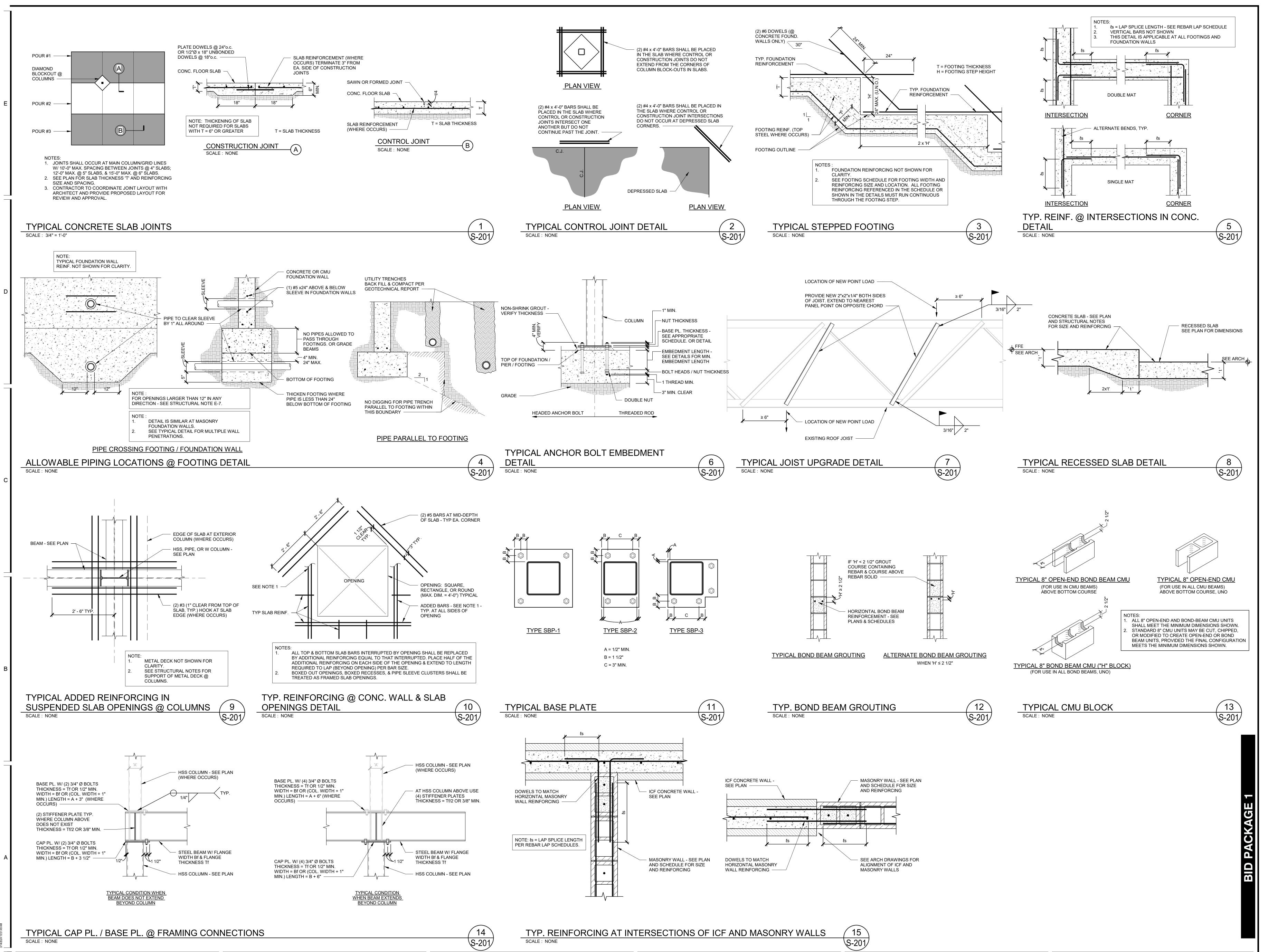
S-140

FOOTING AND FOUNDATION PLAN

ROOF FRAMING PLAN

SCALE: 1/4" = 1'-0"

OPENINGS



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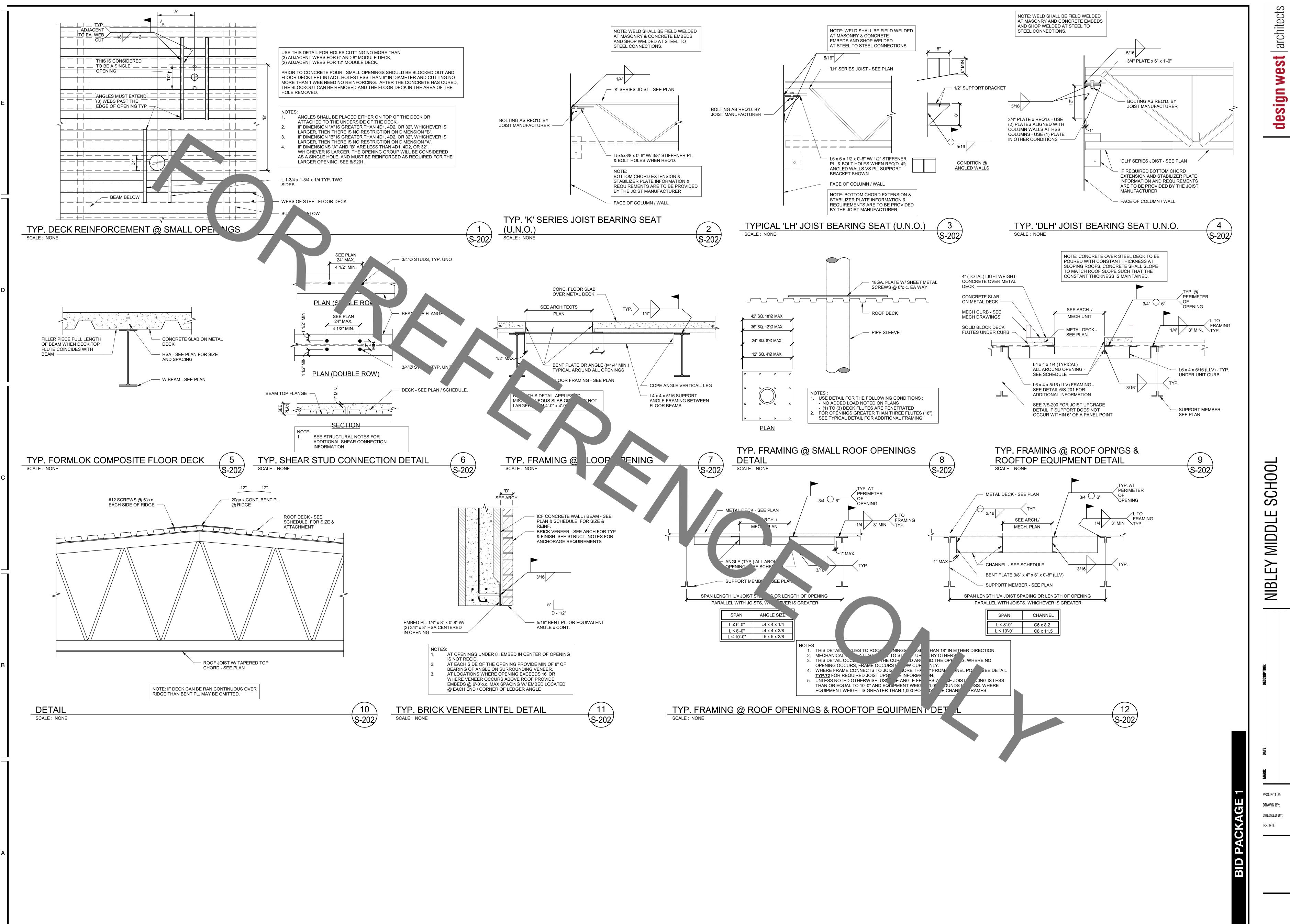
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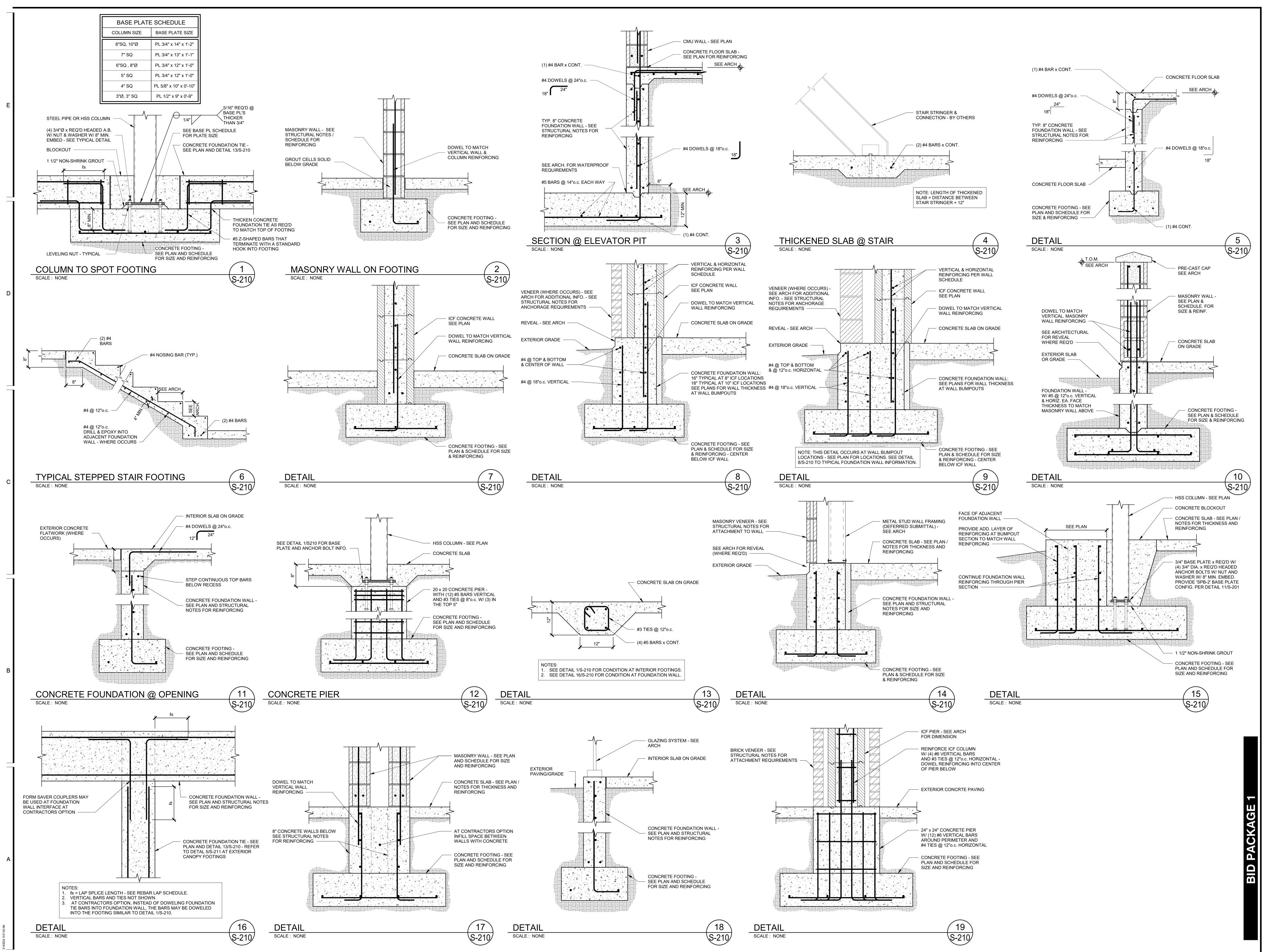
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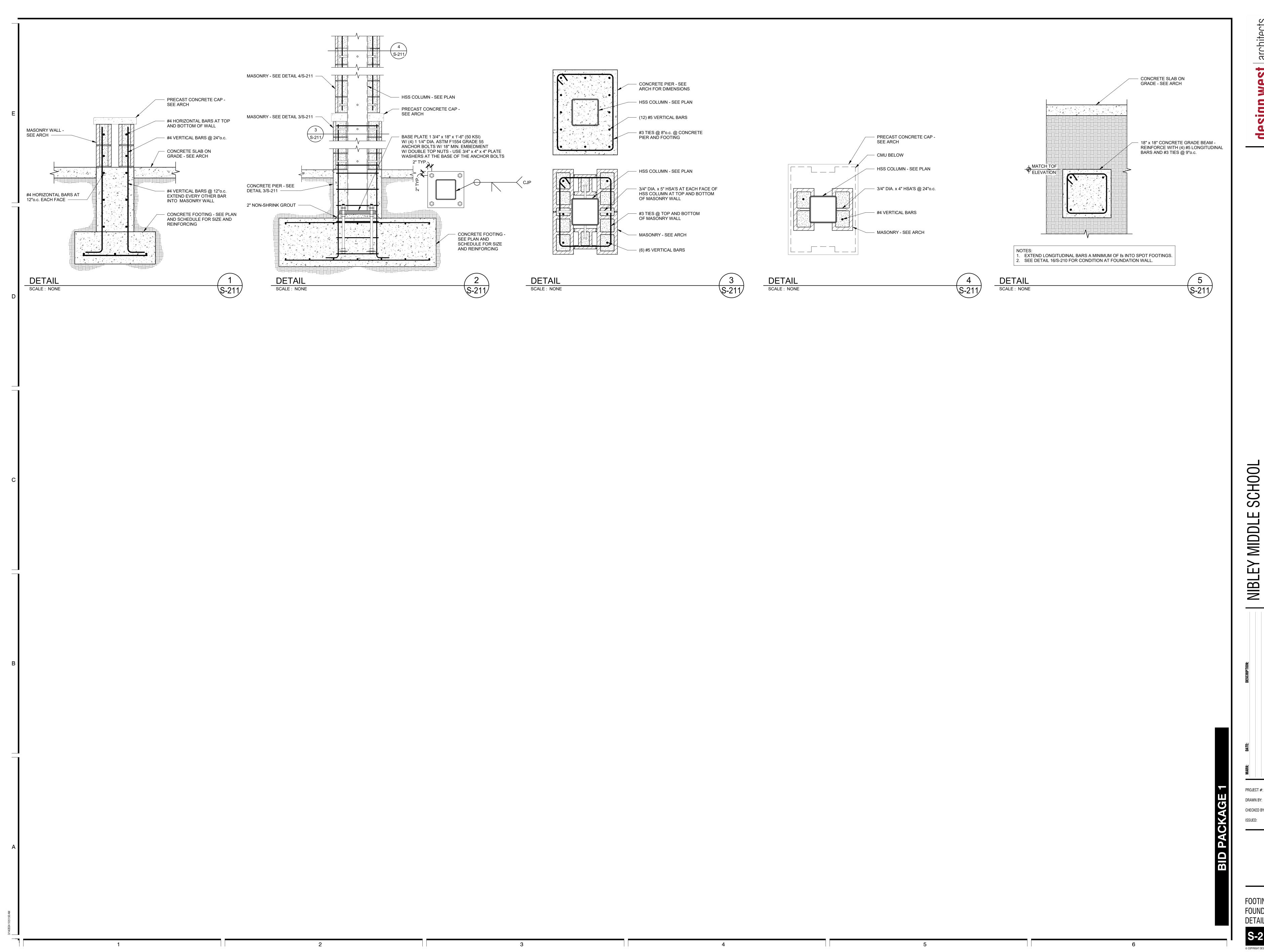
TYPICAL DETAILS





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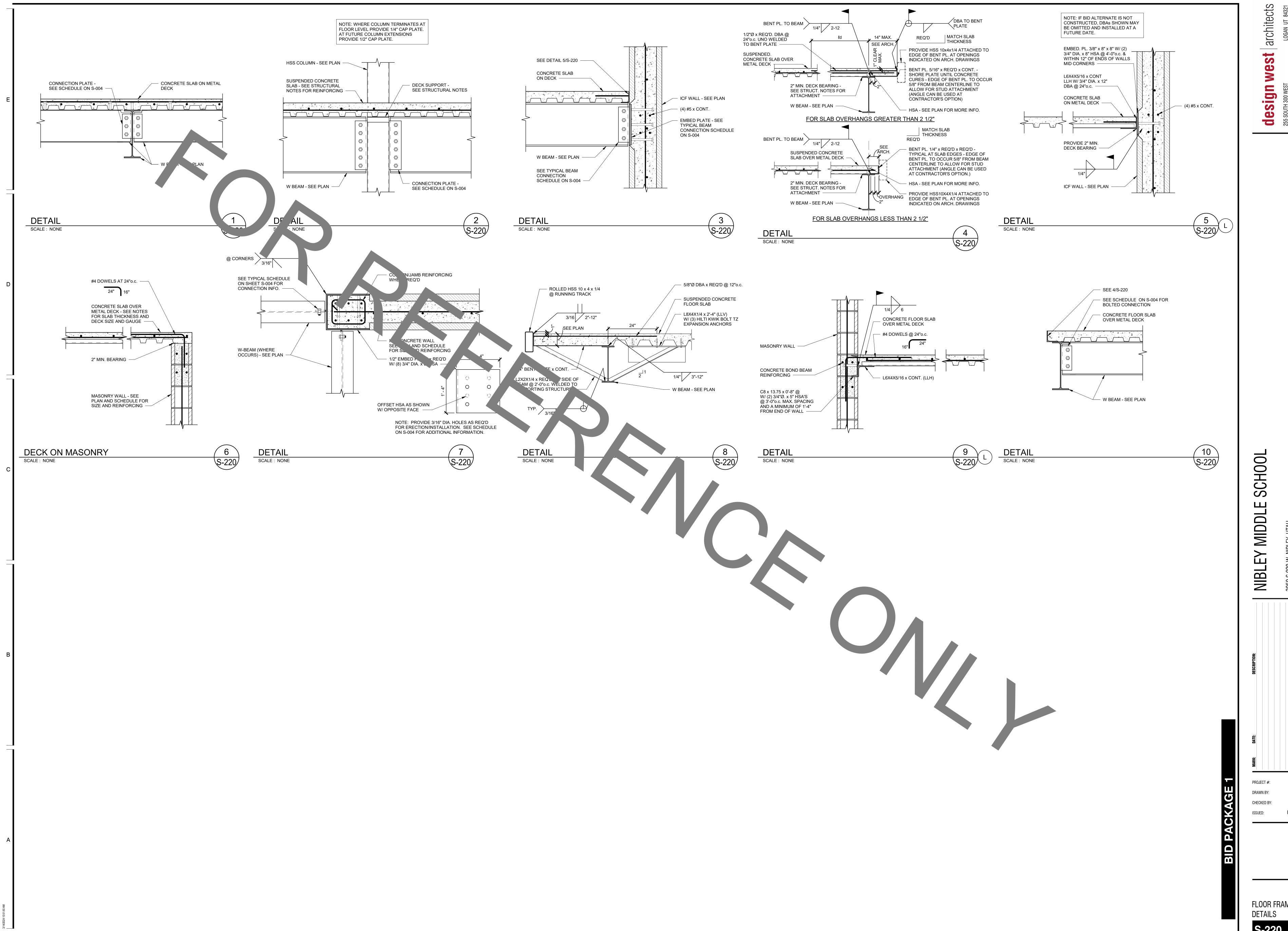
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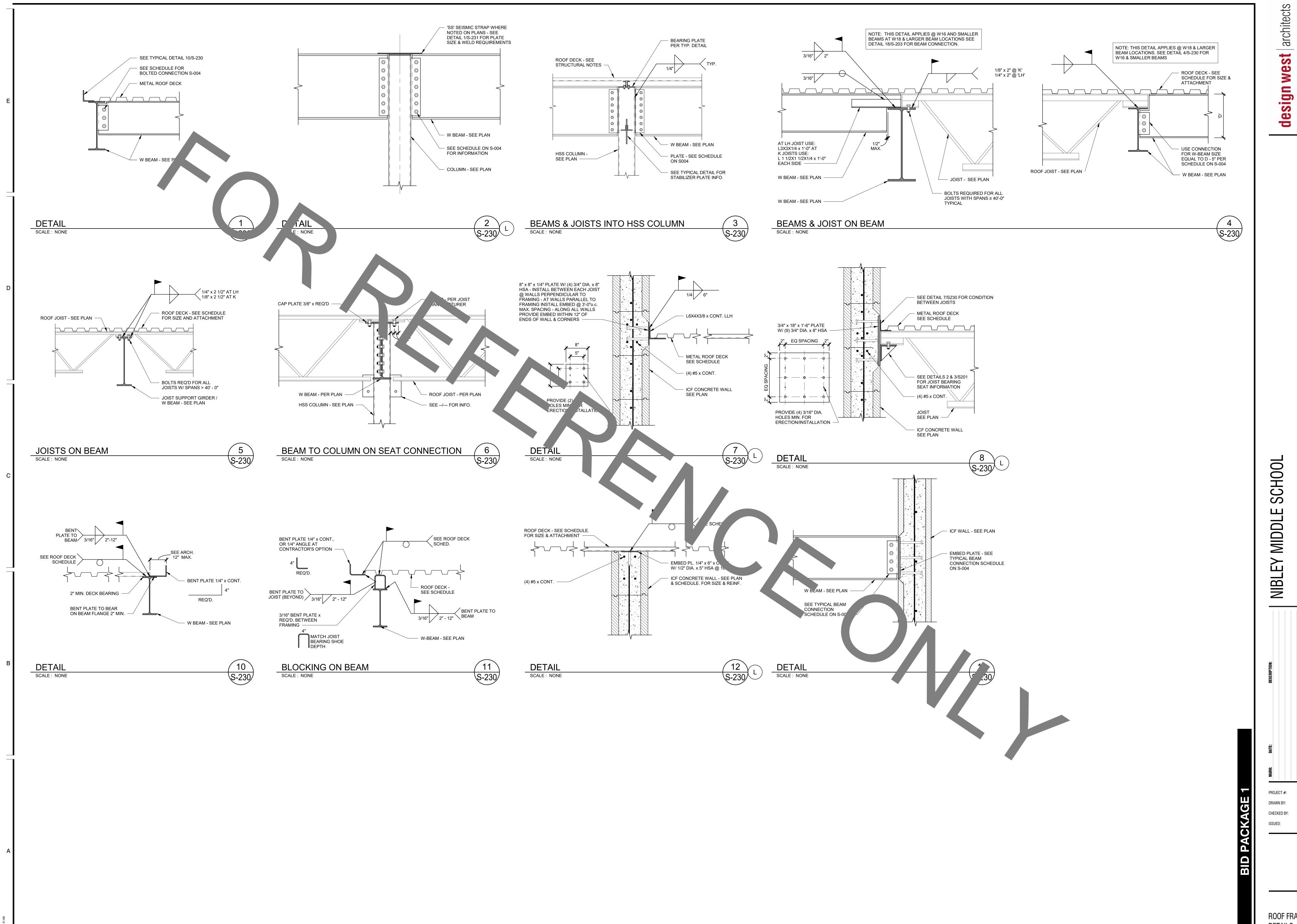


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SUED: 03.14.20

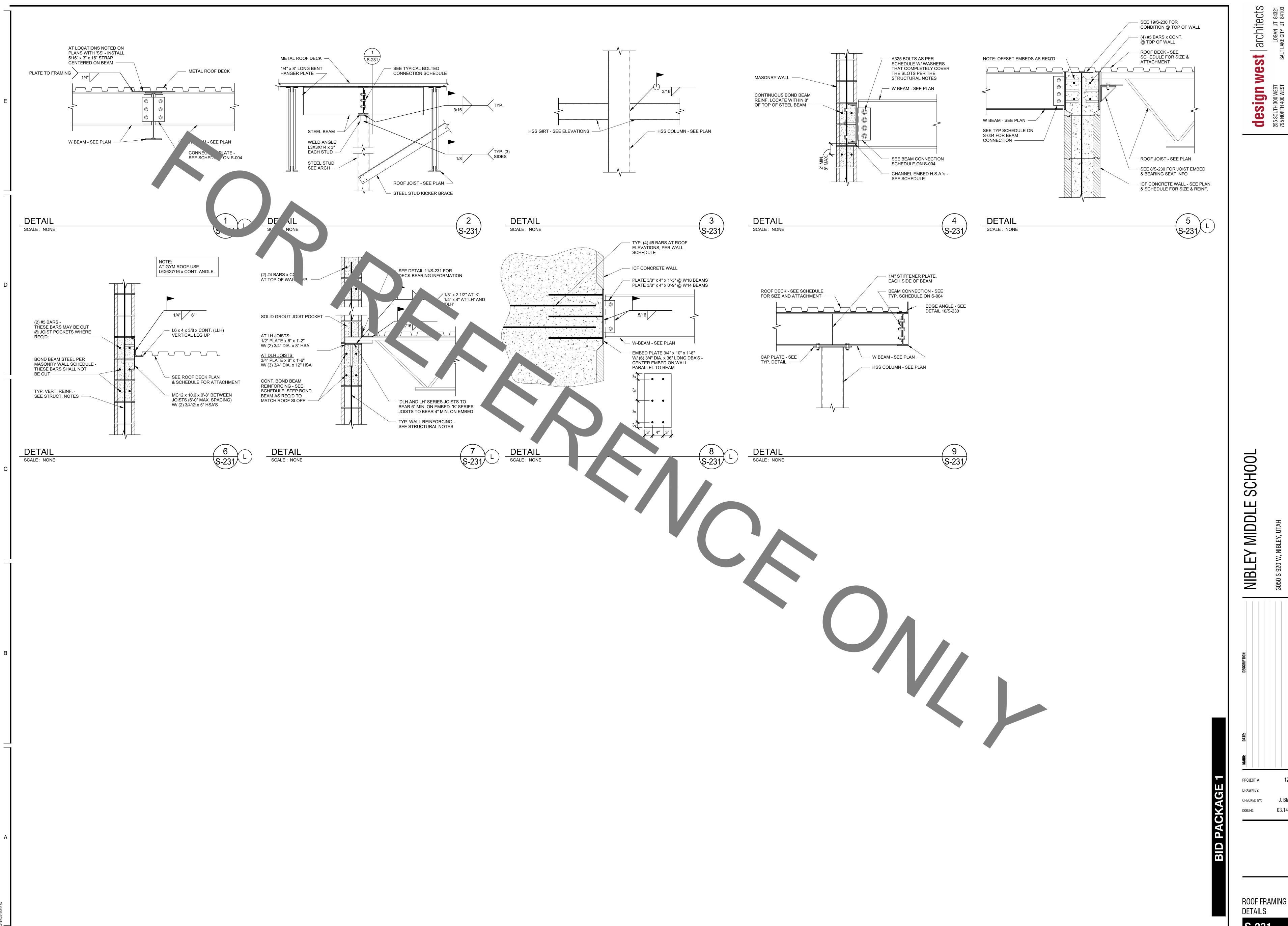
FOOTING &
FOUNDATION
DETAILS





ROOF FRAMING DETAILS

LOGAN UT LAKE CITY UT



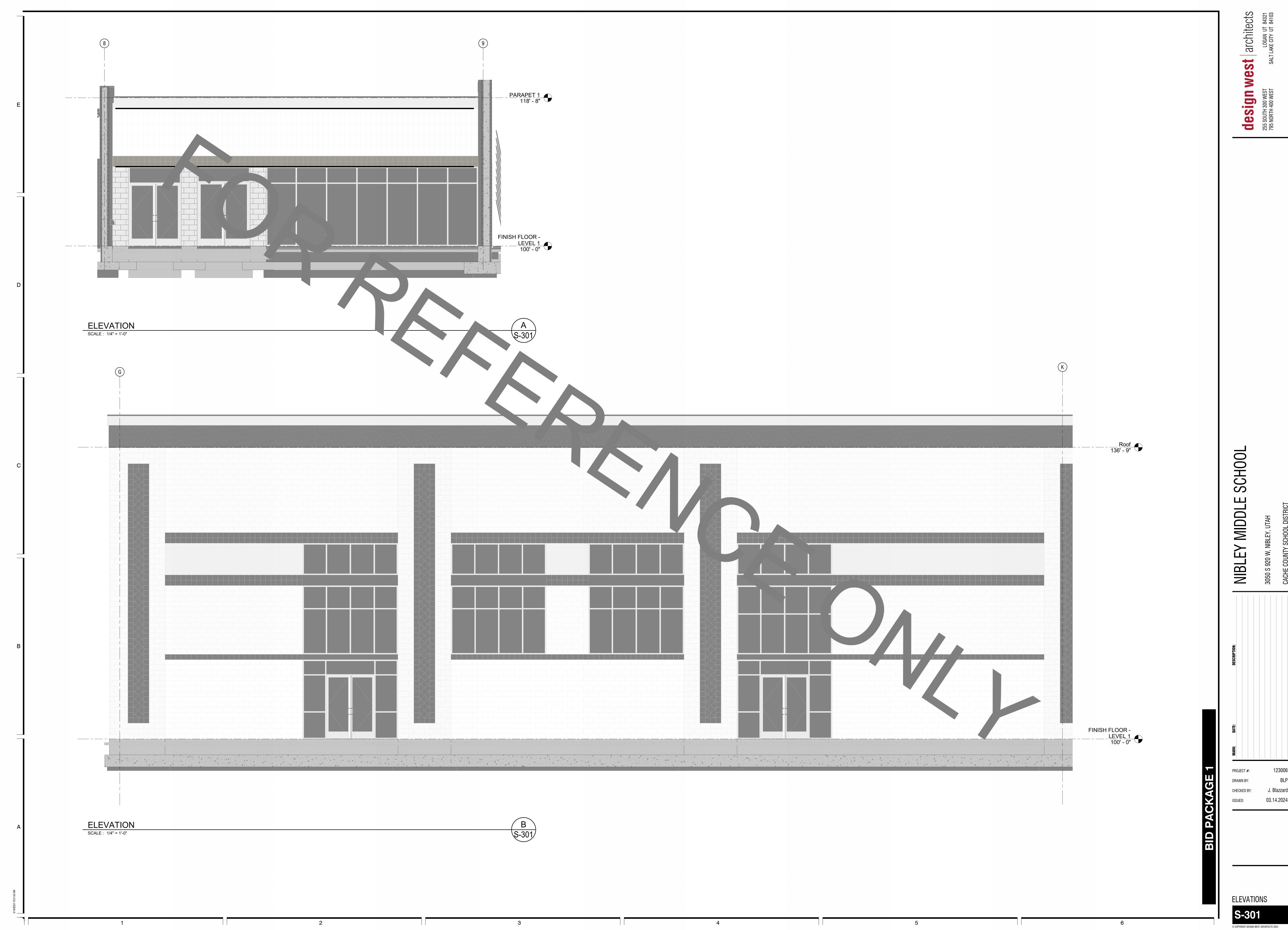
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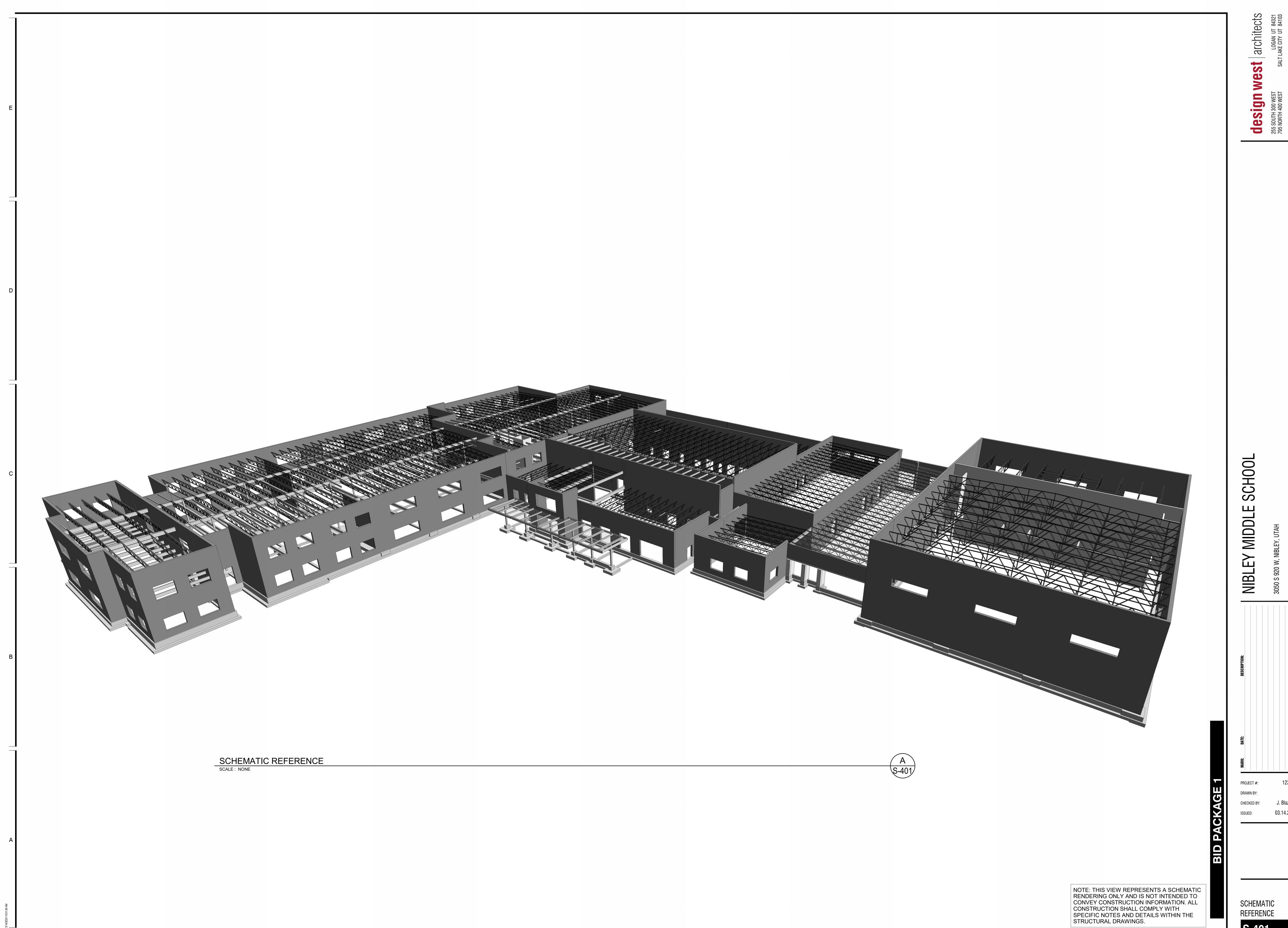
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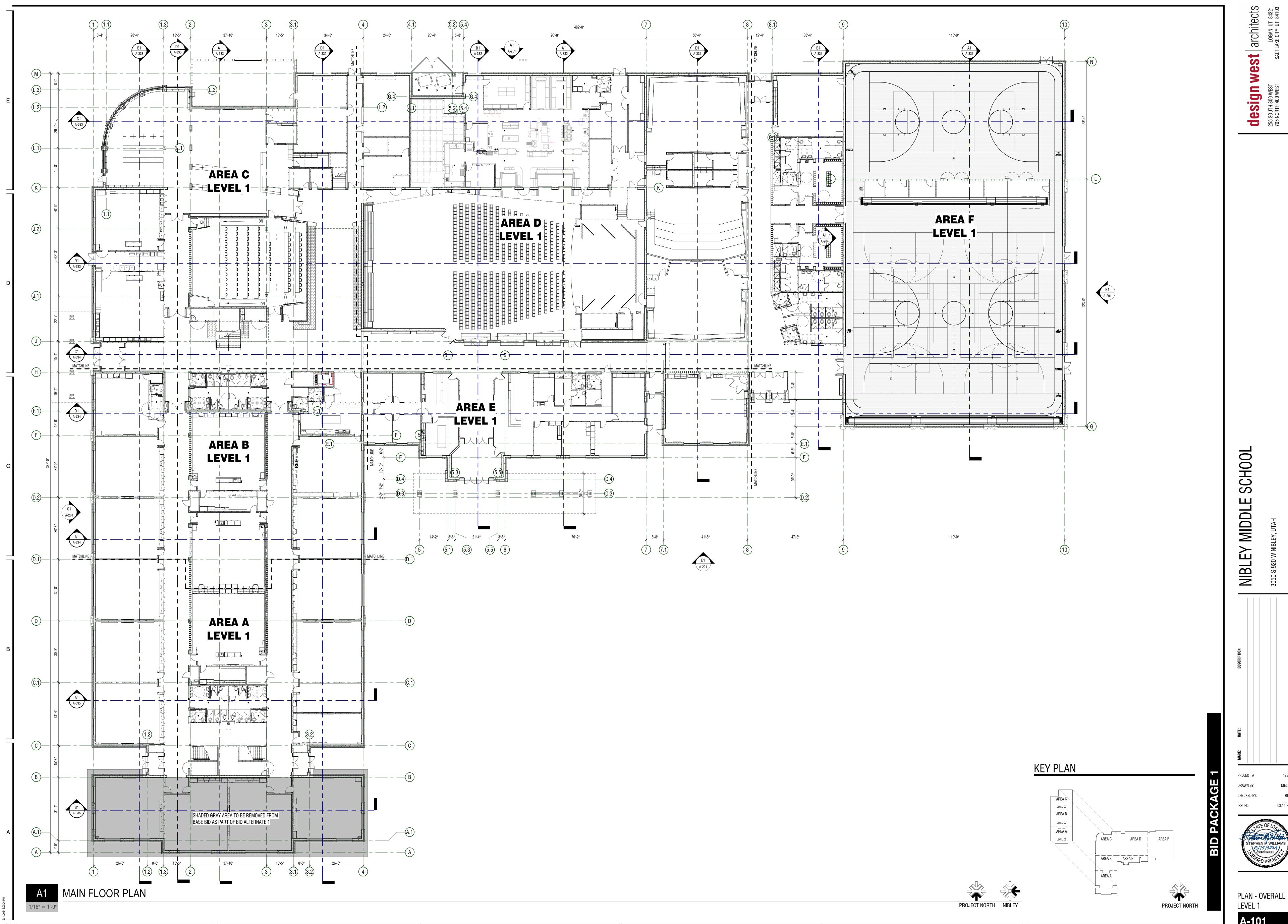
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REFERENCE

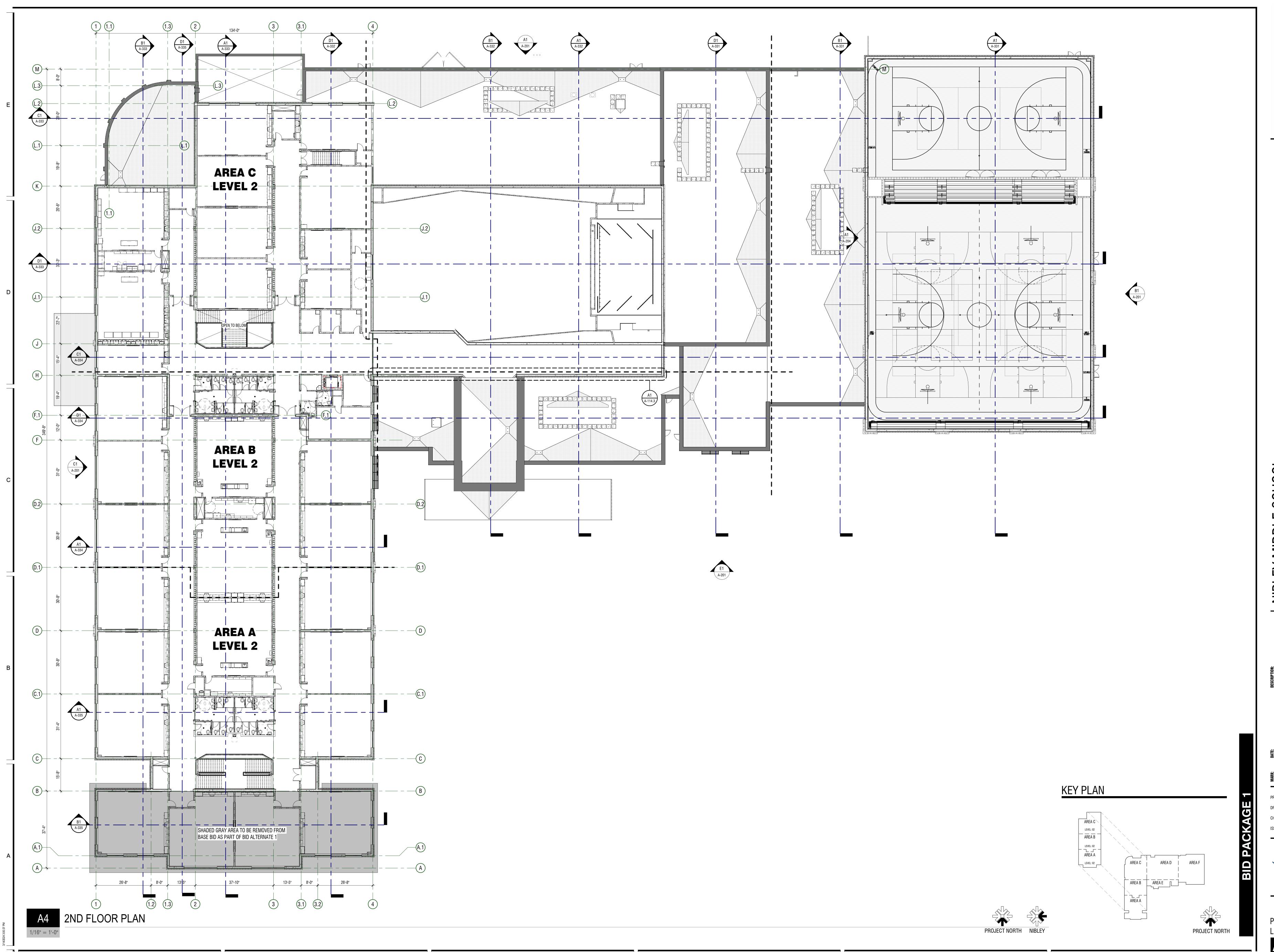


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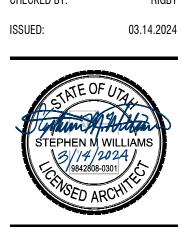


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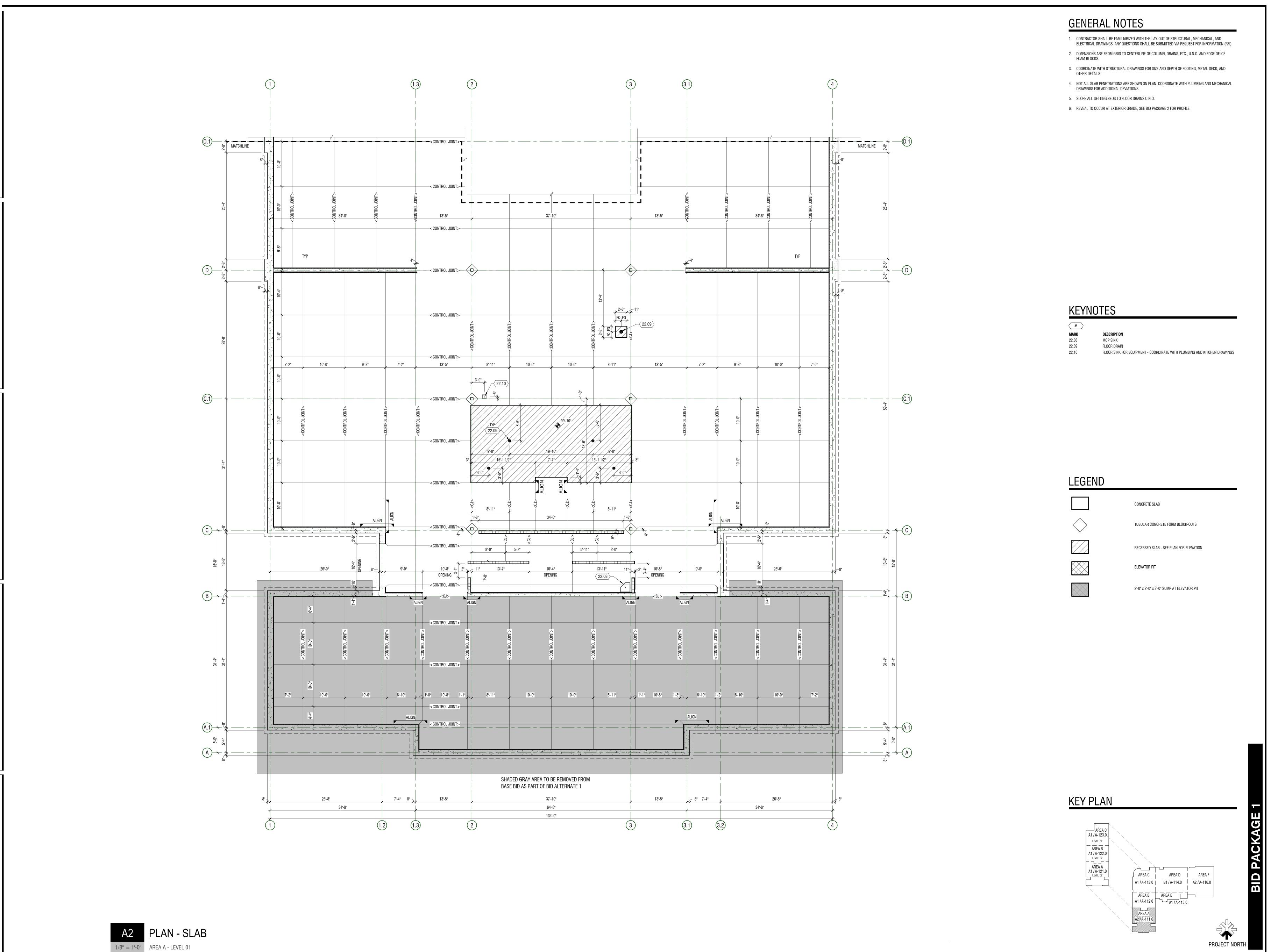


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PLAN - OVERALL LEVEL 2 A-102



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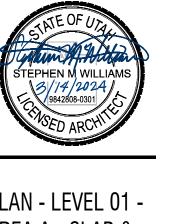
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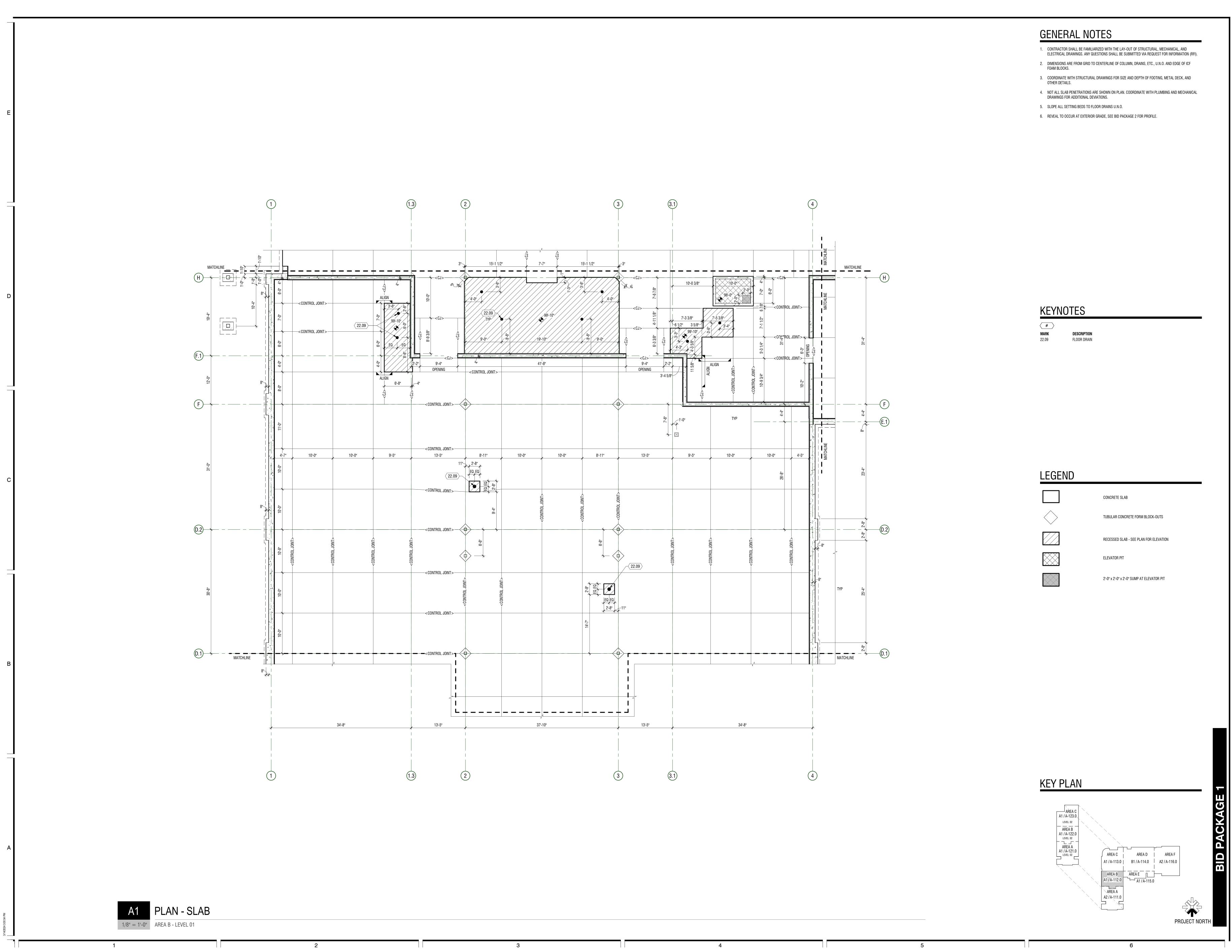
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LOGAN UT LAKE CITY UT

design 255 SOUTH 300 WEST 795 NORTH 400 WEST



PLAN - LEVEL 01 -AREA A - SLAB & FOUNDATION A-111.0



| architects LOGAN UT 84321 TLAKE CITY UT 84103

design west arc 255 SOUTH 300 WEST SALT LAKE C

SLEY MIDDLE SCHOOL

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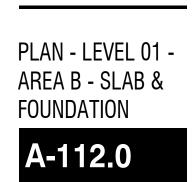
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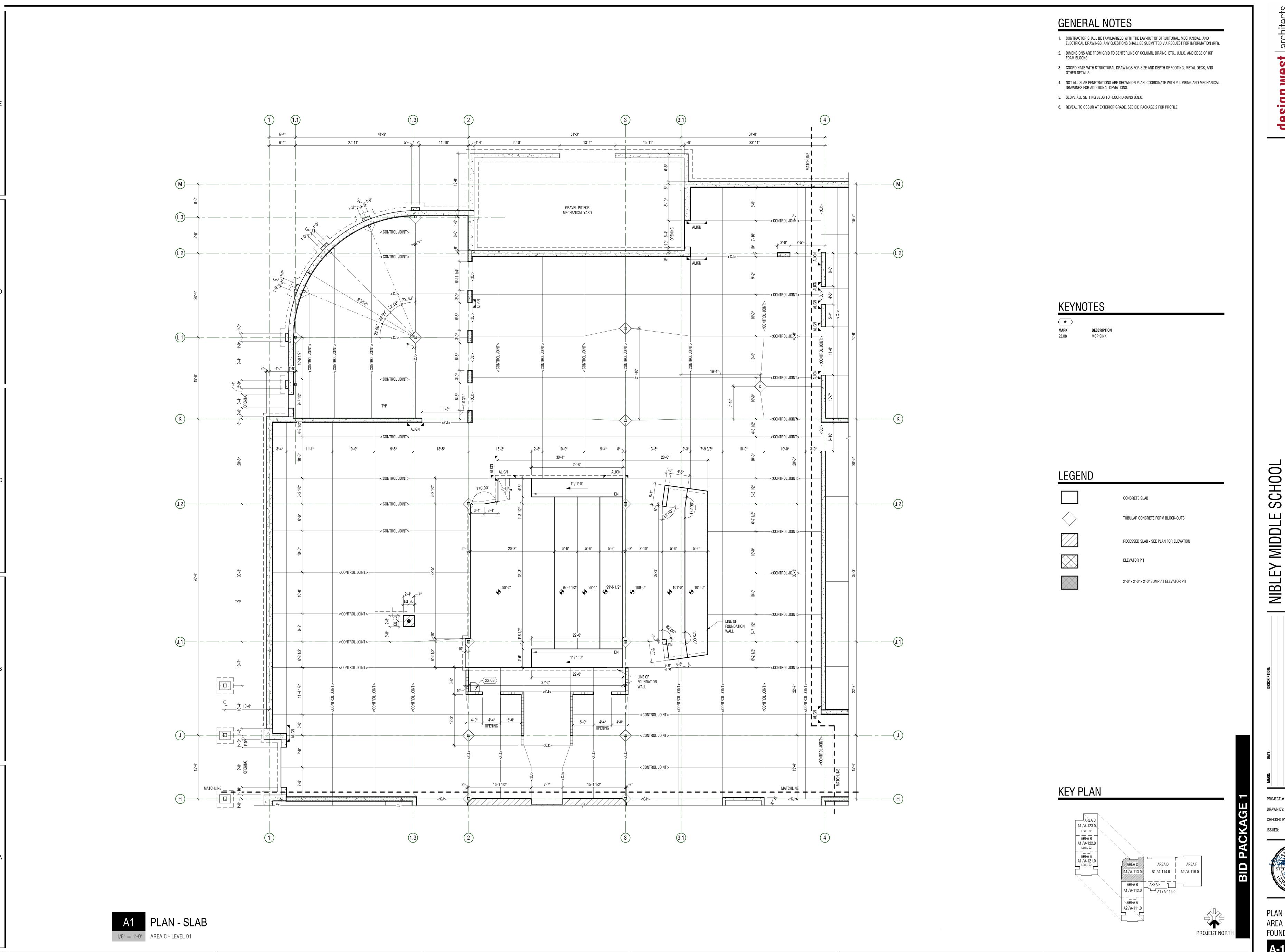
STEPHEN M WILLIAMS

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design 255 SOUTH 300 WEST 795 NORTH 400 WEST

20 W NIBLEY, UTAH

3050 S 920 W NIBI

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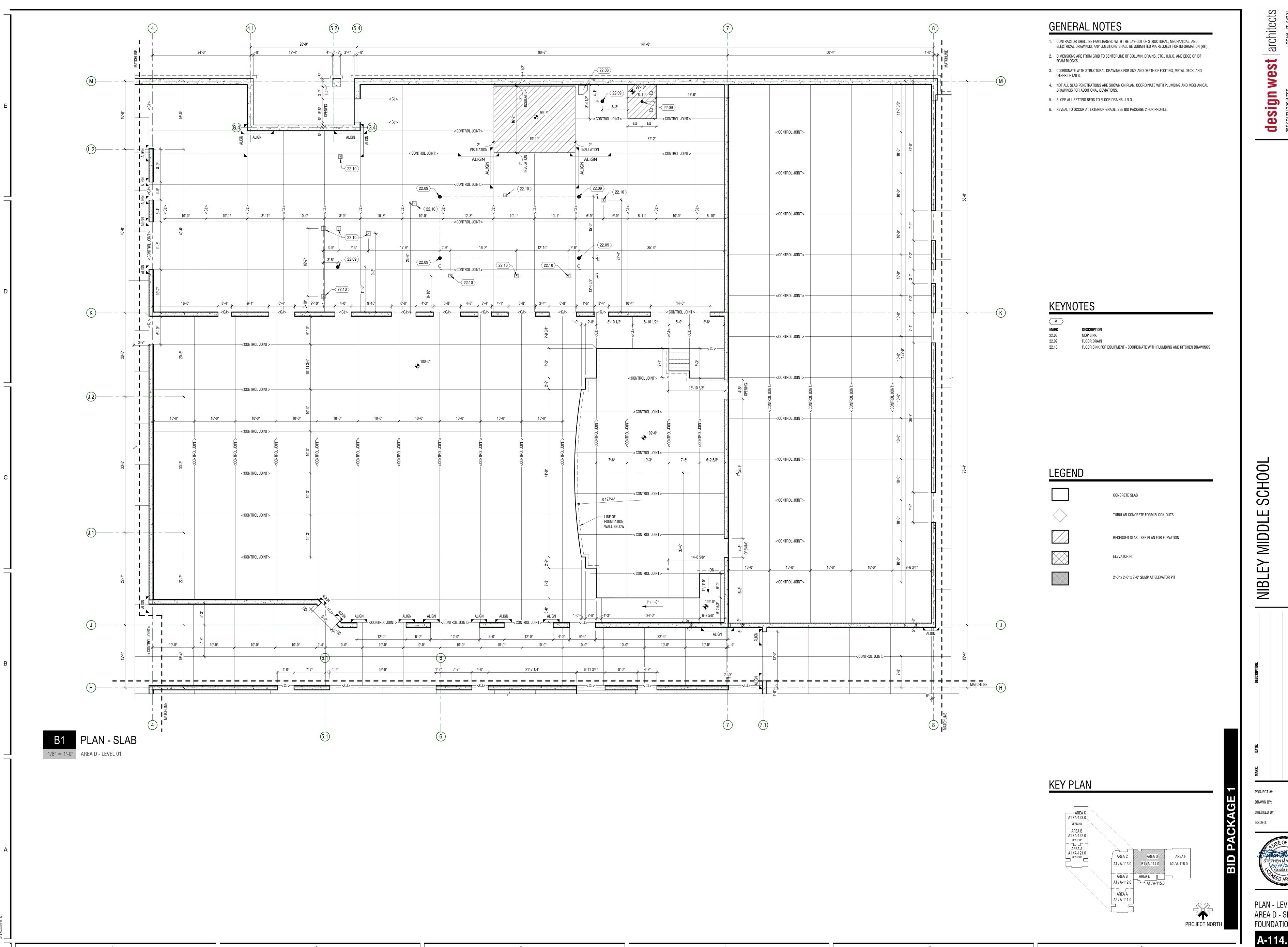
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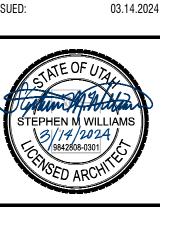
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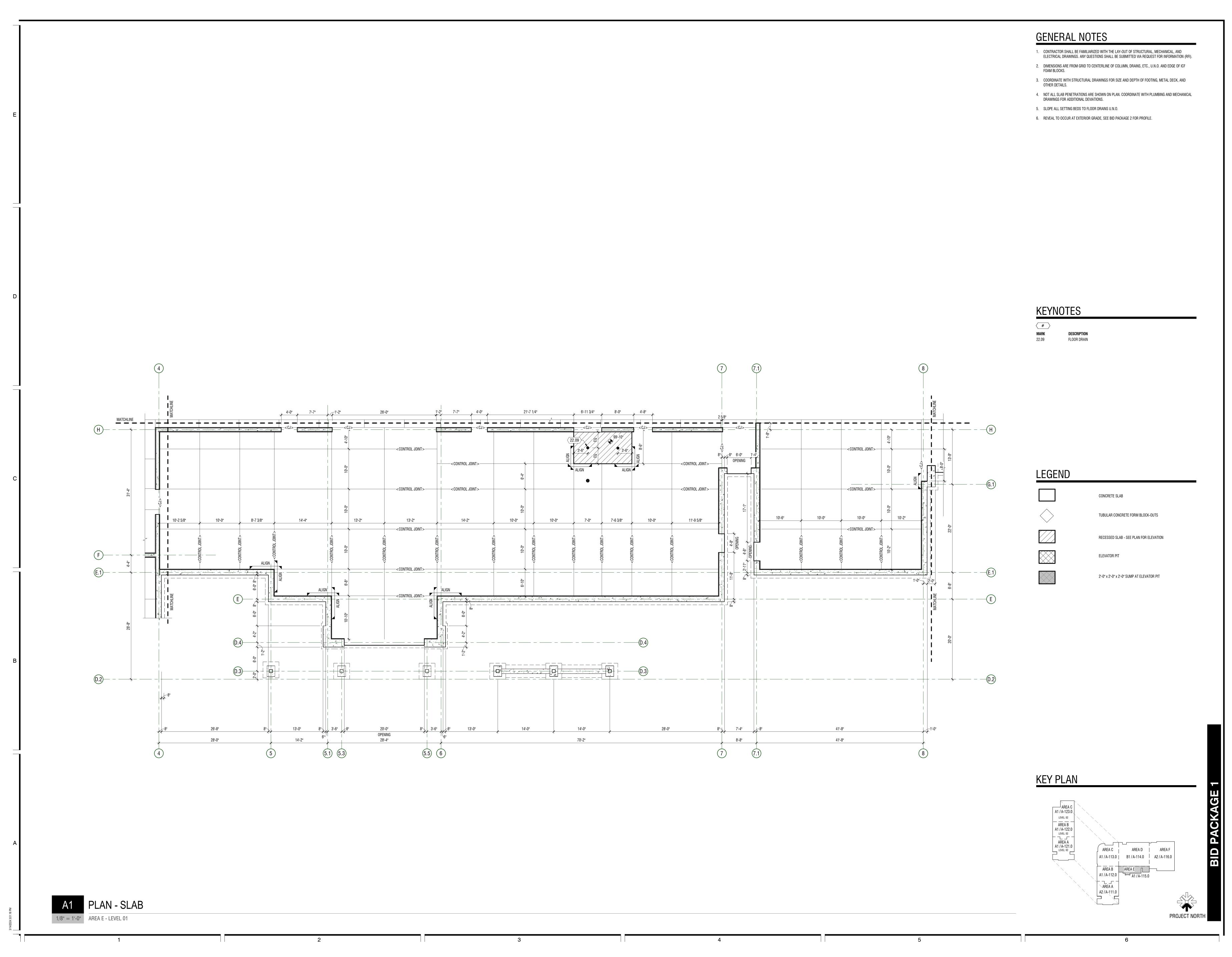
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PLAN - LEVEL 01 -AREA D - SLAB & FOUNDATION



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LOGAN UT 84321
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Lesign 5 SOUTH 300 WEST 5 NORTH 400 WEST

BLEY, UTAH

3050 S 920 W NIBLEY, UT

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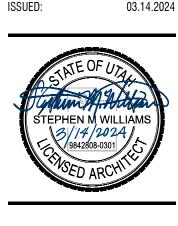
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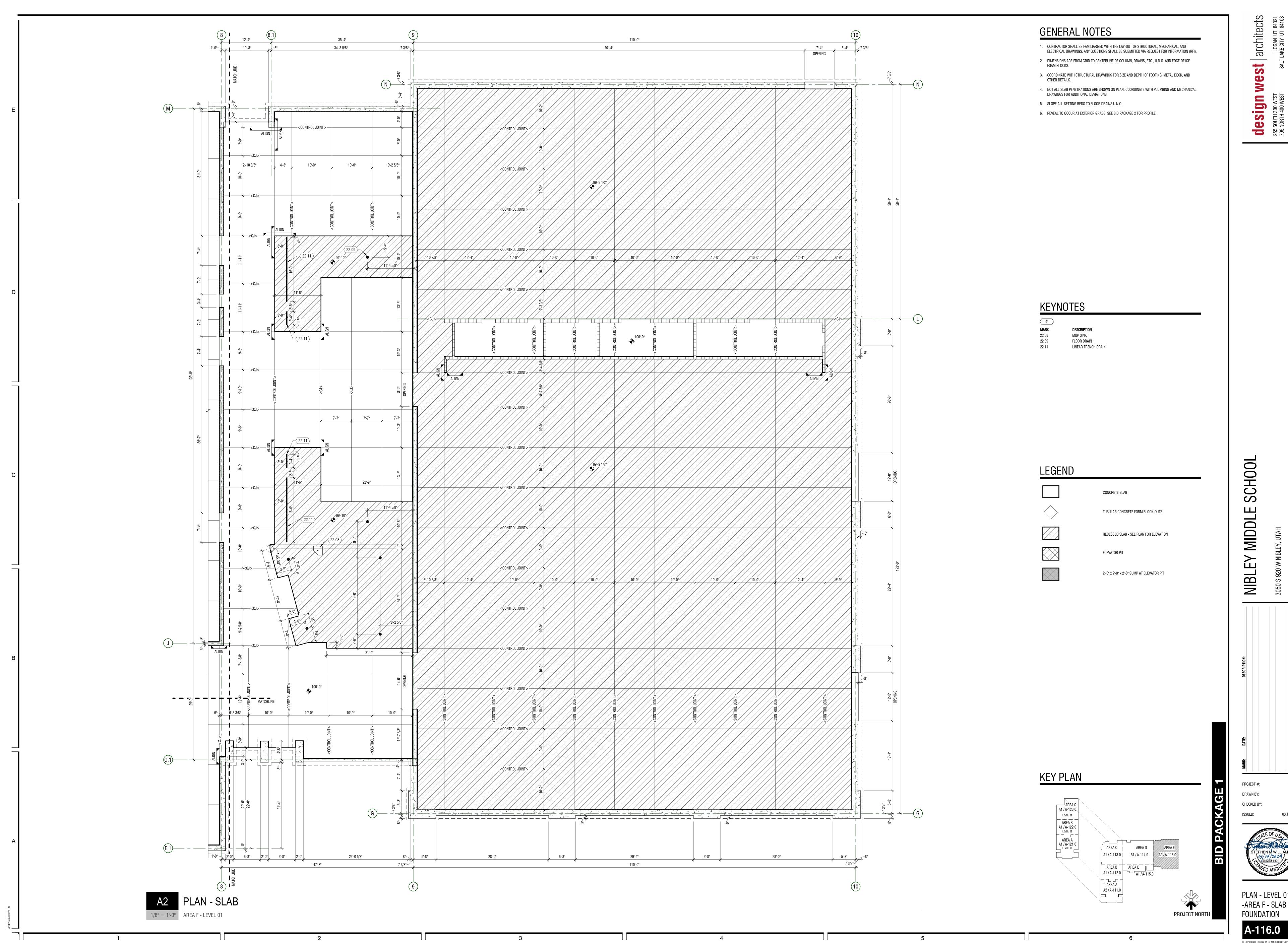
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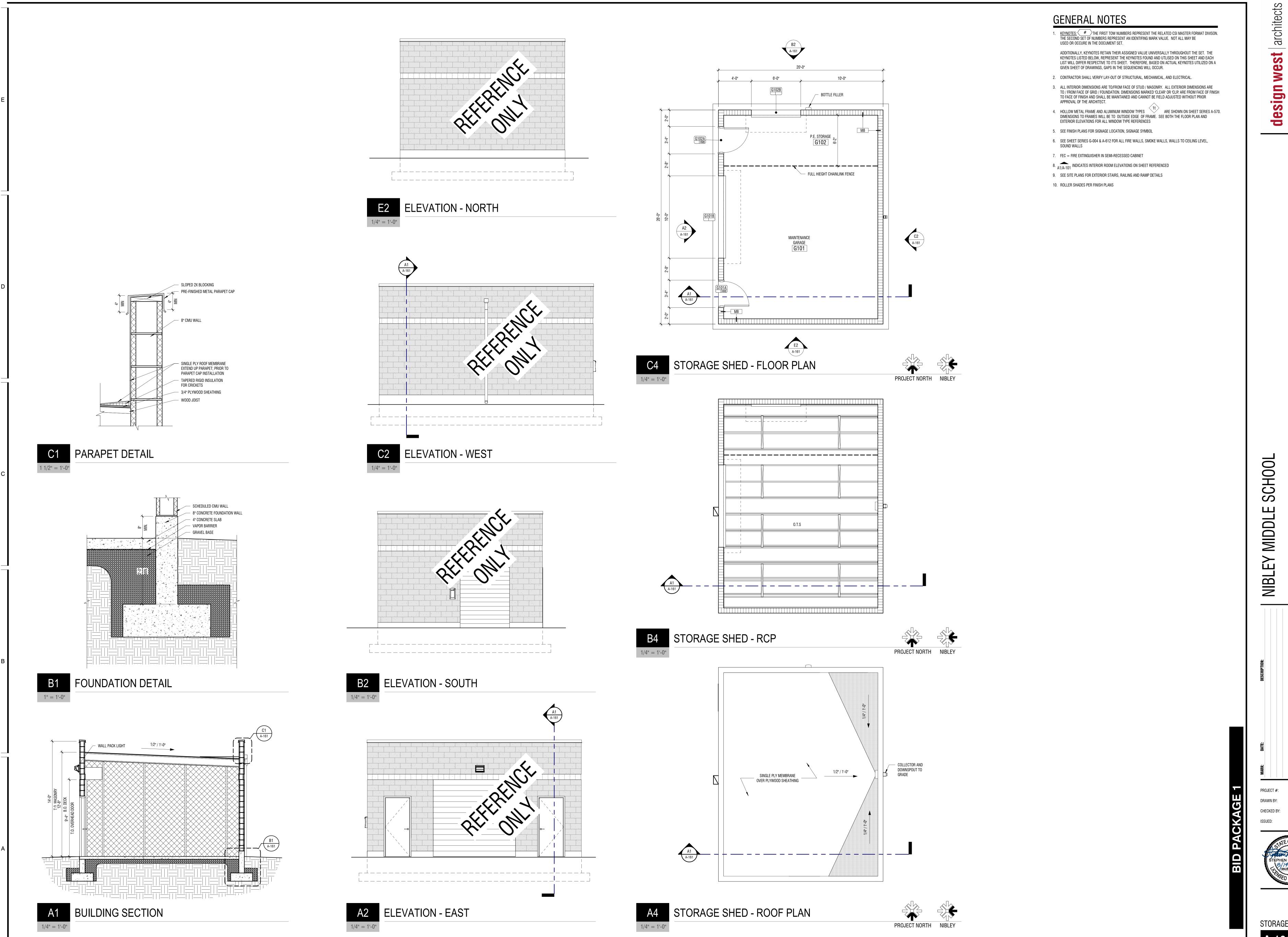
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PLAN - LEVEL 01 -AREA F - SLAB &



LOGAN UT 84321 LAKE CITY UT 84103



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