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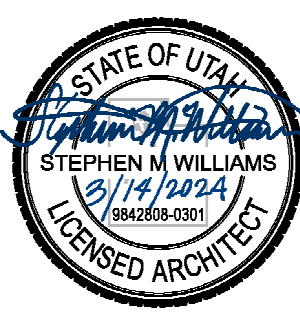
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**HYDE PARK MIDDLE SCHOOL**

250 W 200 S HYDE PARK, UTAH

MARK	DATE	DESCRIPTION

PROJECT #: 128005  
 DRAWN BY: NELSON  
 CHECKED BY: RIGBY  
 ISSUED: 03.14.2024



BID PACKAGE 1

COVER SHEET

G-001



# LEGAL NOTICE

NOTE: THE CONSTRUCTION DOCUMENTS FOR THIS PROJECT ARE COMPOSED OF SETS OF DRAWINGS AND SPECIFICATIONS, AND THEREFORE SHALL BE USED AND MAINTAINED IN THEIR ENTIRETY. ANY CONTRACTOR, SUBCONTRACTOR, VENDOR OR PARTY PARTICIPATING IN OR BRIDING ON THIS PROJECT SHALL BE EXPECTED TO PERFORM DUE DILIGENCE TO INSURE THEIR WORK PERFORMED, AND MATERIALS PROVIDED CONFORMS TO THE INFORMATION PROVIDED WITHIN ANY AND ALL SHEETS OF DRAWINGS AND SPECIFICATIONS, INCLUDING, BUT NOT LIMITED TO, ANY SUBSEQUENT ADDENDA OR CLARIFICATIONS THAT MAY BE ISSUED RELEVANT TO THEIR SCOPE OF WORK. PROJECT SCOPE MAY BE DEFINED WITHIN SPECIFICATIONS AND/OR DRAWINGS.

ADDITIONALLY, DRAWINGS MAY NOT BE RE-SCALED WHEN PRINTED. WRITTEN DIMENSIONS SHALL HAVE PRECEDENCE, AND LARGER SCALE DRAWINGS SHALL HAVE PRECEDENCE OVER SMALLER SCALE DRAWINGS.

ANY DEVIATION FROM OR CONFLICT WITHIN THE DRAWINGS AND/OR SPECIFICATIONS, MUST BE SUBMITTED VIA REQUEST FOR INFORMATION (RFI) AND RESPONDED TO BY THE ARCHITECT PRIOR TO BID OR BEFORE CONTINUING THAT PORTION OF WORK.

# BID ALTERNATES

REFER TO SPECIFICATIONS 01 2300-ALTERNATES FOR FULL DESCRIPTION OF ALTERNATES)

BID ALTERNATE 1  
• REMOVAL OF CLASSROOMS

BID ALTERNATE 2  
• ROOFING MEMBRANE

# DEFERRED SUBMITTALS

- FIRE ALARMS
- FIRE PROTECTION AND SEISMIC DESIGNS
- OPEN WEB JOISTS AND RELATED COMPONENTS
- COLD FORM FRAMING
- SEISMIC BRACING OF ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL ITEMS
- STEEL STAIRS

# BID PACKAGE NOTES

SHEET REVISIONS WILL BE ISSUED ACCORDING TO THEIR BID PACKAGE. (EX. AS1-BP-1)

**NOTES:**  
**BID PACKAGE 1**  
CIVIL (UNDERGROUND MEP) FOUNDATION  
BC ISSUED FOR BIDDING AND CONSTRUCTION  
RD ISSUED FOR REFERENCE ONLY.)

**BID PACKAGE 2**  
LANDSCAPE / STRUCTURAL / ARCHITECTURAL / MECH., PLUMBING & FIRE / ELECTRICAL / INTERCOM  
BC ISSUED FOR BIDDING AND CONSTRUCTION  
RD WR - RE-RELEASED (REVISIONS)

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## VOLUME 1

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G-002	PROJECT INFORMATION	BC	RR
G-003	PLAN - SITE CODE	BC	RR
G-004	CODE ANALYSIS	BC	RR
G-005	PLAN - CODE	BC	RR
G-006	PLAN - CODE	BC	RR
G-007	PLAN - CODE	BC	RR

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C-001	CIVIL GENERAL NOTES	BC	RR
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C-201	CIVIL - OVERALL SITE PLAN	RD	RR
C-202	CIVIL - SITE PLAN SOUTHWEST	RD	RR
C-203	CIVIL - SITE PLAN NORTHWEST	RD	RR
C-204	CIVIL - SITE PLAN NORTHEAST	RD	RR
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C-301	CIVIL UTILITY PLAN	BC	RR
C-302	CIVIL STORMWATER PLAN	RD	RR
C-401	CIVIL - OVERALL GRADING PLAN	BC	RR
C-402	CIVIL - GRADING PLAN SOUTHWEST	BC	RR
C-403	CIVIL - GRADING PLAN NORTHWEST	BC	RR
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C-405	CIVIL - GRADING PLAN SOUTHEAST	BC	RR
C-501	CIVIL UTILITY DETAILS	BC	RR
C-502	CIVIL UTILITY DETAILS	BC	RR
C-503	CIVIL UTILITY DETAILS	BC	RR
C-601	CIVIL - ROADWAY PLAN & PROFILE 200 SOUTH	RD	RR
C-602	CIVIL - ROADWAY PLAN & PROFILE 200 WEST	RD	RR

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L-101	PLANTING PLAN AREA A	BC	RR
L-102	PLANTING PLAN AREA B	BC	RR
L-103	PLANTING PLAN AREA C	BC	RR
L-104	PLANTING PLAN AREA D	BC	RR
L-501	LANDSCAPE DETAILS	BC	RR

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S-002	STRUCTURAL NOTES	BC	RR
S-010	SCHEDULES	BC	RR
S-011	SCHEDULES	BC	RR
S-012	SCHEDULES	BC	RR
S-013	SCHEDULES	BC	RR
S-110	FOOTING AND FOUNDATION PLAN - OVERALL	BC	RR
S-111	FOOTING AND FOUNDATION PLAN - AREA A	BC	RR
S-112	FOOTING AND FOUNDATION PLAN - AREA B	BC	RR
S-113	FOOTING AND FOUNDATION PLAN - AREA C	BC	RR
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S-115	FOOTING AND FOUNDATION PLAN - AREA E	BC	RR
S-116	FOOTING AND FOUNDATION PLAN - AREA F	BC	RR
S-120	FLOOR FRAMING PLAN - OVERALL	RD	RR
S-121	FLOOR FRAMING PLAN - AREA A	RD	RR
S-122	FLOOR FRAMING PLAN - AREA B	RD	RR
S-123	FLOOR FRAMING PLAN - AREA C	RD	RR
S-124	FLOOR FRAMING PLAN - AREA D	RD	RR
S-130	ROOF FRAMING PLAN - OVERALL	RD	RR
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S-134	ROOF FRAMING PLAN - AREA D	RD	RR
S-135	ROOF FRAMING PLAN - AREA E	RD	RR
S-136	ROOF FRAMING PLAN - AREA F	RD	RR
S-140	SPORTS STORAGE FOOTING, FPN, AND ROOF FRAMING PLAN	BC	RR
S-201	TYPICAL DETAILS	BC	RR
S-210	FOOTING & FOUNDATION DETAILS	BC	RR
S-211	FOOTING & FOUNDATION DETAILS	BC	RR
S-220	FLOOR FRAMING DETAILS	RD	RR
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AS-103	ARCHITECTURAL SITE PLAN AREA C	BC	RR
AS-104	ARCHITECTURAL SITE PLAN AREA D	BC	RR
AS-501	SITE DETAILS	BC	RR

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A-102	PLAN - OVERALL LEVEL 2	BC	RR
A-111.0	PLAN - LEVEL 01 - AREA A - SLAB & FOUNDATION	BC	RR
A-111.1	PLAN - LEVEL 01 - AREA A - DIMENSION	BC	RR
A-111.2	PLAN - LEVEL 01 - AREA A - ANNOTATION	BC	RR
A-111.3	PLAN - LEVEL 01 - AREA A - FINISH	BC	RR
A-111.4	PLAN - LEVEL 01 - AREA A - REFLECTED CEILING	BC	RR
A-112.0	PLAN - LEVEL 01 - AREA B - SLAB & FOUNDATION	BC	RR
A-112.1	PLAN - LEVEL 01 - AREA B - DIMENSION	BC	RR
A-112.2	PLAN - LEVEL 01 - AREA B - ANNOTATION	BC	RR
A-112.3	PLAN - LEVEL 01 - AREA B - REFLECTED CEILING	BC	RR
A-112.4	PLAN - LEVEL 01 - AREA B - FINISH	BC	RR
A-113.0	PLAN - LEVEL 01 - AREA C - SLAB & FOUNDATION	BC	RR
A-113.1	PLAN - LEVEL 01 - AREA C - DIMENSION	BC	RR
A-113.2	PLAN - LEVEL 01 - AREA C - ANNOTATION	BC	RR
A-113.3	PLAN - LEVEL 01 - AREA C - FINISH	BC	RR
A-113.4	PLAN - LEVEL 01 - AREA C - REFLECTED CEILING	BC	RR
A-114.0	PLAN - LEVEL 01 - AREA D - SLAB & FOUNDATION	BC	RR
A-114.1	PLAN - LEVEL 01 - AREA D - DIMENSION	BC	RR
A-114.2	PLAN - LEVEL 01 - AREA D - ANNOTATION	BC	RR
A-114.3	PLAN - LEVEL 01 - AREA D - FINISH	BC	RR
A-114.4	PLAN - LEVEL 01 - AREA D - REFLECTED CEILING	BC	RR
A-115.0	PLAN - LEVEL 01 - AREA E - SLAB & FOUNDATION	BC	RR
A-115.1	PLAN - LEVEL 01 - AREA E - DIMENSION	BC	RR
A-115.2	PLAN - LEVEL 01 - AREA E - ANNOTATION	BC	RR
A-115.3	PLAN - LEVEL 01 - AREA E - FINISH	BC	RR
A-115.4	PLAN - LEVEL 01 - AREA E - REFLECTED CEILING	BC	RR
A-116.0	PLAN - LEVEL 01 - AREA F - SLAB & FOUNDATION	BC	RR
A-116.1	PLAN - LEVEL 01 - AREA F - DIMENSION	BC	RR
A-116.2	PLAN - LEVEL 01 - AREA F - ANNOTATION	BC	RR
A-116.3	PLAN - LEVEL 01 - AREA F - FINISH	BC	RR
A-116.4	PLAN - LEVEL 01 - AREA F - REFLECTED CEILING	BC	RR
A-121.0	PLAN - LEVEL 02 - AREA A - SLAB	BC	RR
A-121.1	PLAN - LEVEL 02 - AREA A - DIMENSION	BC	RR
A-121.2	PLAN - LEVEL 02 - AREA A - ANNOTATION	BC	RR
A-121.3	PLAN - LEVEL 02 - AREA A - FINISH	BC	RR
A-121.4	PLAN - LEVEL 02 - AREA A - REFLECTED CEILING	BC	RR
A-122.0	PLAN - LEVEL 02 - AREA B - SLAB	BC	RR
A-122.1	PLAN - LEVEL 02 - AREA B - DIMENSION	BC	RR
A-122.2	PLAN - LEVEL 02 - AREA B - ANNOTATION	BC	RR
A-122.3	PLAN - LEVEL 02 - AREA B - FINISH	BC	RR
A-122.4	PLAN - LEVEL 02 - AREA B - REFLECTED CEILING	BC	RR
A-123.0	PLAN - LEVEL 02 - AREA C - SLAB	BC	RR
A-123.1	PLAN - LEVEL 02 - AREA C - DIMENSION	BC	RR
A-123.2	PLAN - LEVEL 02 - AREA C - ANNOTATION	BC	RR
A-123.3	PLAN - LEVEL 02 - AREA C - FINISH	BC	RR
A-123.4	PLAN - LEVEL 02 - AREA C - REFLECTED CEILING	BC	RR
A-124.0	PLAN - LEVEL 02 - AREA D - DIMENSION	BC	RR
A-124.1	PLAN - LEVEL 02 - AREA D - ANNOTATION	BC	RR
A-151	PLAN - ROOF - OVERALL	BC	RR
A-161	STORAGE SHED	BC	RR
A-201	ELEVATIONS - EXTERIOR	BC	RR
A-202	ELEVATIONS - EXTERIOR	BC	RR
A-203	ELEVATIONS - EXTERIOR	BC	RR
A-204	ELEVATIONS - EXTERIOR	BC	RR
A-205	ELEVATIONS - EXTERIOR	BC	RR
A-206	ELEVATIONS - EXTERIOR	BC	RR
A-301	SECTIONS - BUILDING	BC	RR
A-302	SECTIONS - BUILDING	BC	RR
A-303	SECTIONS - BUILDING	BC	RR
A-304	SECTIONS - BUILDING	BC	RR
A-305	SECTIONS - BUILDING	BC	RR
A-306	SECTIONS - BUILDING	BC	RR
A-307	SECTIONS - BUILDING	BC	RR
A-308	SECTIONS - BUILDING	BC	RR
A-309	SECTIONS - BUILDING	BC	RR
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A-314	SECTIONS - BUILDING	BC	RR
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A-326	SECTIONS - BUILDING	BC	RR
A-327	SECTIONS - BUILDING	BC	RR
A-328	SECTIONS - BUILDING	BC	RR
A-329	SECTIONS - BUILDING	BC	RR
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A-331	SECTIONS - BUILDING	BC	RR
A-332	SECTIONS - BUILDING	BC	RR
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A-340	SECTIONS - BUILDING	BC	RR
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A-342	SECTIONS - BUILDING	BC	RR
A-343	SECTIONS - BUILDING	BC	RR
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A-347	SECTIONS - BUILDING	BC	RR
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A-351	SECTIONS - BUILDING	BC	RR
A-352	SECTIONS - BUILDING	BC	RR
A-353	SECTIONS - BUILDING	BC	RR
A-354	SECTIONS - BUILDING	BC	RR
A-355	SECTIONS - BUILDING	BC	RR
A-356	SECTIONS - BUILDING	BC	RR
A-411	ENLARGED VIEWS	BC	RR
A-412	ENLARGED VIEWS	BC	RR
A-413	ENLARGED VIEWS	BC	RR
A-414	ENLARGED VIEWS	BC	RR
A-415	ENLARGED VIEWS	BC	RR

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A-417	ENLARGED VIEWS	BC	RR
A-418	ENLARGED VIEWS	BC	RR
A-419	ENLARGED VIEWS	BC	RR
A-420	ENLARGED VIEWS	BC	RR
A-421	ENLARGED VIEWS	BC	RR
A-422	ENLARGED VIEWS	BC	RR
A-423	ENLARGED VIEWS	BC	RR
A-424	ENLARGED VIEWS	BC	RR
A-425	ENLARGED VIEWS - STAIR/ELEVATOR LADDER	BC	RR
A-426	ENLARGED VIEWS - STAIR/ELEVATOR LADDER	BC	RR
A-427	ENLARGED VIEWS - STAIR/ELEVATOR LADDER	BC	RR
A-428	ENLARGED VIEWS - STAIR/ELEVATOR LADDER	BC	RR
A-429	ENLARGED VIEWS - STAIR/ELEVATOR LADDER	BC	RR
A-430	ENLARGED VIEWS - STAIR/ELEVATOR LADDER	BC	RR
A-431	ENLARGED VIEWS - STAIR/ELEVATOR LADDER	BC	RR
A-432	ENLARGED VIEWS - STAIR/ELEVATOR LADDER	BC	RR
A-433	ENLARGED VIEWS - STAIR/ELEVATOR LADDER	BC	RR
A-434	ENLARGED VIEWS - STAIR/ELEVATOR LADDER	BC	RR
A-435	ENLARGED VIEWS - STAIR/ELEVATOR LADDER	BC	RR
A-436	ENLARGED VIEWS - STAIR/ELEVATOR LADDER	BC	RR
A-437	ENLARGED VIEWS - STAIR/ELEVATOR LADDER	BC	RR
A-438	ENLARGED VIEWS - STAIR/ELEVATOR LADDER	BC	RR
A-439	ENLARGED VIEWS - STAIR/ELEVATOR LADDER	BC	RR
A-440	ENLARGED VIEWS - STAIR/ELEVATOR LADDER	BC	RR
A-441	ENLARGED VIEWS - STAIR/ELEVATOR LADDER	BC	RR
A-442	ENLARGED VIEWS - STAIR/ELEVATOR LADDER	BC	RR
A-443	ENLARGED VIEWS - STAIR/ELEVATOR LADDER	BC	RR
A-444	ENLARGED VIEWS - STAIR/ELEVATOR LADDER	BC	RR
A-445	ENLARGED VIEWS - STAIR/ELEVATOR LADDER	BC	RR
A-446	ENLARGED VIEWS - STAIR/ELEVATOR LADDER	BC	RR
A-447	ENLARGED VIEWS - STAIR/ELEVATOR LADDER	BC	RR
A-448	ENLARG		

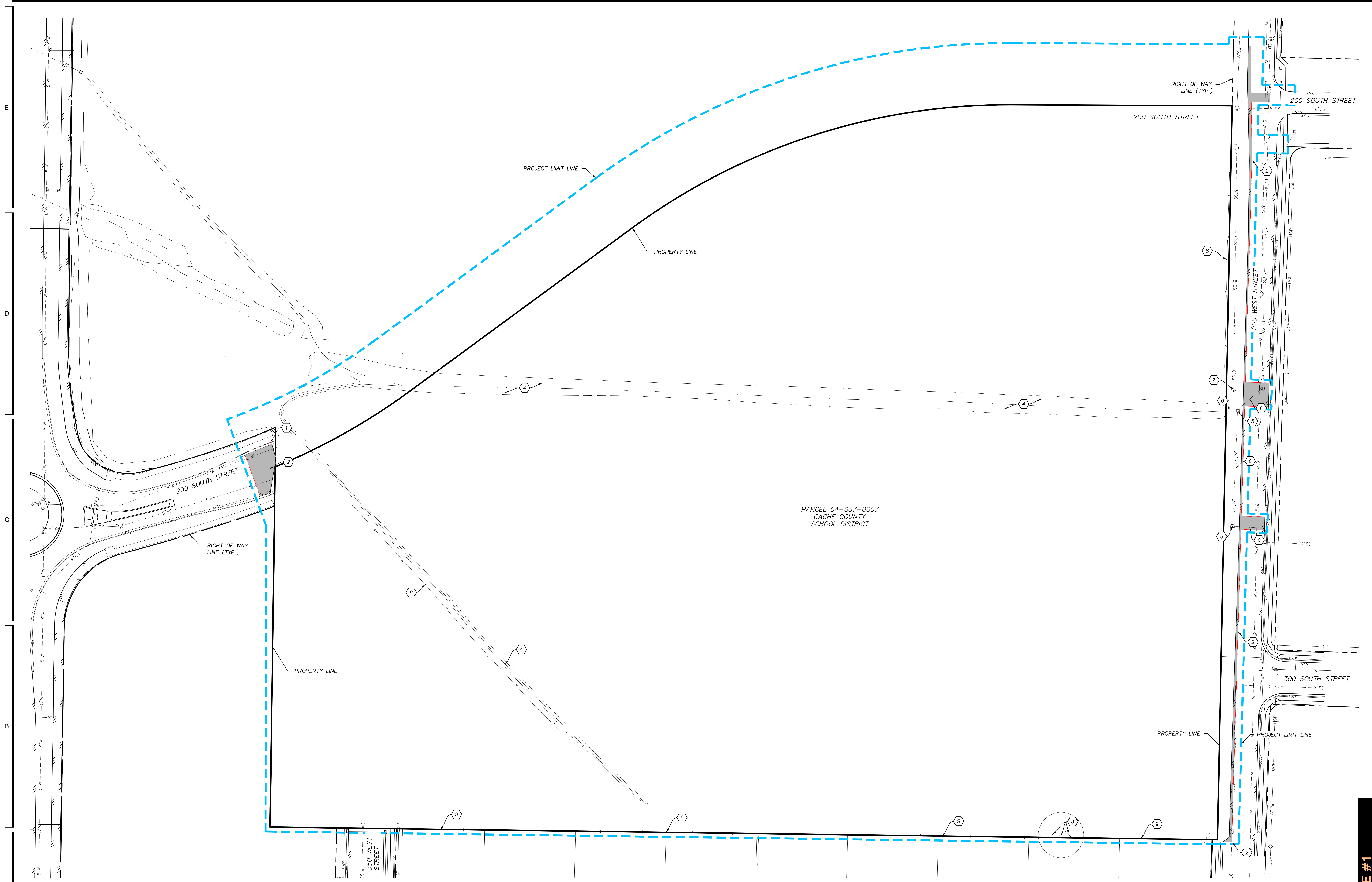








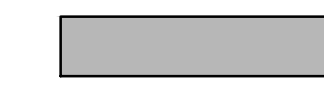





**DEMOLITION PLAN NOTES**

1. ALL EXISTING HARDSCAPE FEATURES, LANDSCAPING AND IRRIGATION SYSTEMS THAT ARE DAMAGED OR DISTURBED DURING CONSTRUCTION ARE TO BE REPAIRED OR REPLACED AT CONTRACTORS EXPENSE.
2. ALL UTILITIES TO BE PROTECTED IN PLACE UNLESS SPECIFIED OTHERWISE.
3. ALL TREES NOT SPECIFICALLY NOTED FOR REMOVAL TO BE PROTECTED IN PLACE INCLUDING PROTECTION OF EXISTING GRADES TO DRIP LINE.
4. ANY TOPSOIL STRIPPED DURING CONSTRUCTION TO BE STOCKPILED AND PROTECTED FOR REUSE WHERE POSSIBLE.
5. SAWCUT CONCRETE CURBING AND FLATWORK AT EXISTING JOINT LINES WHERE POSSIBLE.

**LEGEND**

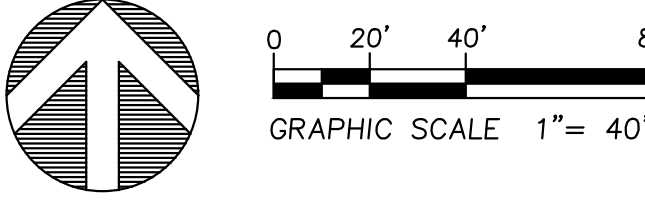
-  EXISTING ASPHALT TO BE REMOVED.
-  SAWCUT LINE

**DEMOLITION NOTES**

- 1 SAWCUT AND REMOVE CONCRETE CURB AND GUTTER (4.9' FROM END OF CURB).
- 2 SAWCUT AND REMOVE EXISTING ASPHALT (2' MIN. FROM EDGE OF ASPHALT).
- 3 REMOVE EXISTING TREE AND FENCE SECTIONS. COORDINATE WITH CCSD AND OWNER OF PARCEL #04-201-0002.
- 4 REMOVE/FILL-IN EXISTING IRRIGATION DITCH. SEE STORMWATER PLAN FOR PIPING OF IRRIGATION WATER.
- 5 REMOVE EXISTING STORMWATER JUNCTION BOX. SEE ROADWAY STORMWATER PLAN FOR REPLACEMENT.
- 6 REMOVE EXISTING STORMWATER PIPE. SEE ROADWAY STORMWATER PLAN FOR REPLACEMENT.
- 7 REMOVE SEWER MANHOLE LID ABOVE CLEANOUT. SEE 200 WEST PLAN AND PROFILE SHEET.
- 8 REMOVE EXISTING FARM FENCE.
- 9 PROTECT EXISTING FENCE ALONG SOUTH PROPERTY LINE (SEE KEY NOTE #3 FOR EXCEPTION).

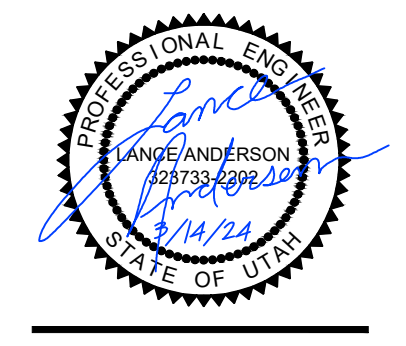
**DEMOLITION PLAN**

SCALE: 1"=40'



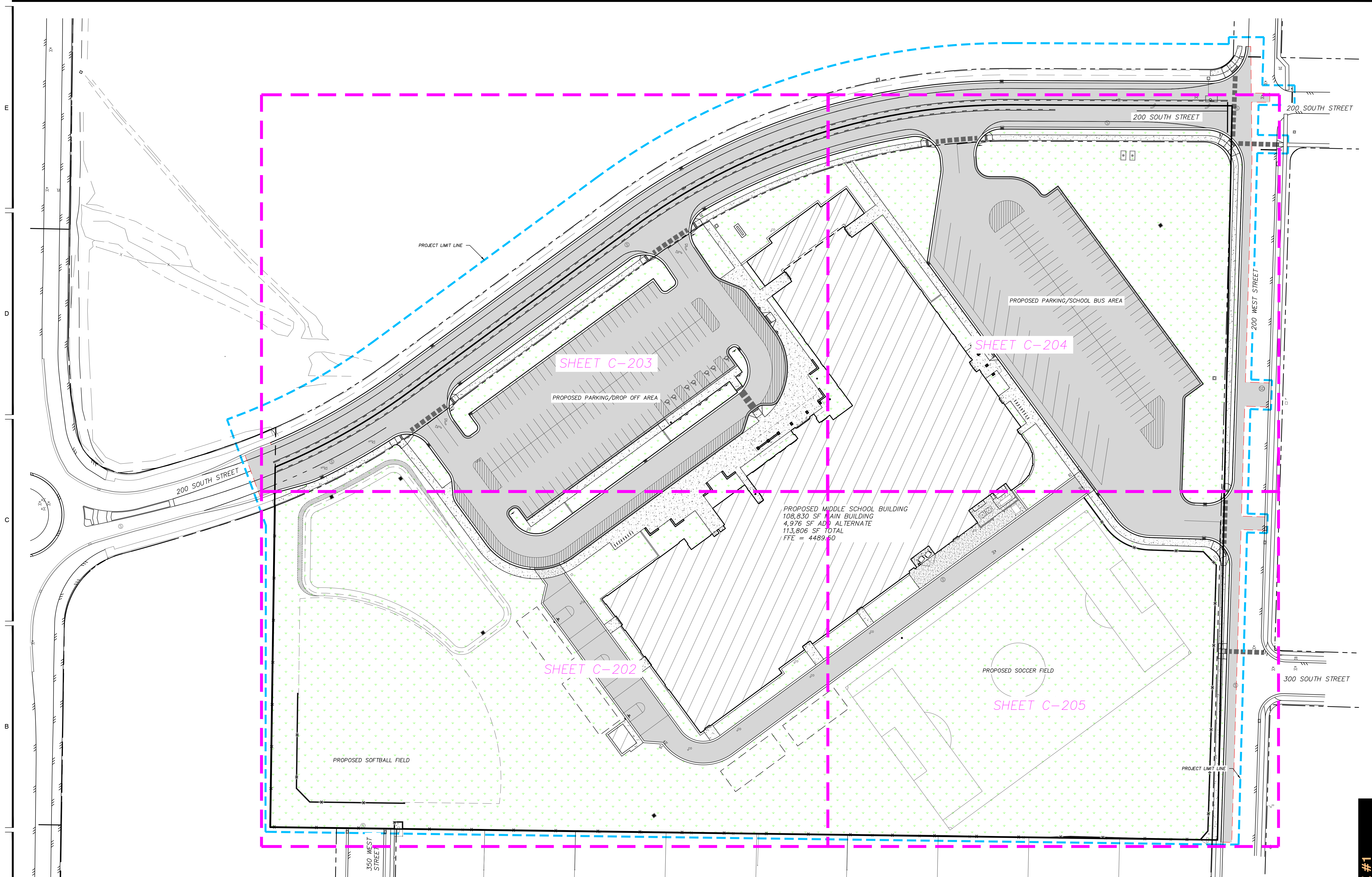
MARK	DATE	DESCRIPTION

PROJECT #: 123005  
 DRAWN BY: J. JENSEN  
 CHECKED BY: L. ANDERSON  
 ISSUED: 03.14.2024



**BID PACKAGE #1**





PROPOSED MIDDLE SCHOOL BUILDING  
 108,830 SF MAIN BUILDING  
 4,976 SF ADD ALTERNATE  
 113,806 SF TOTAL  
 FFE = 4489.60

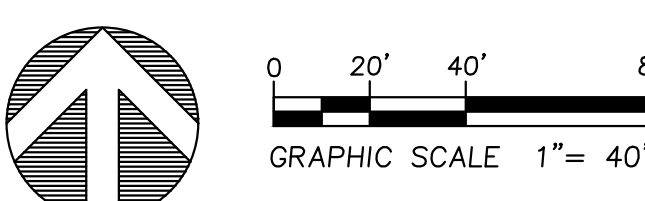
**SITE PLAN NOTES**

1. ALL DIMENSIONS AND RADII ARE MEASURED AT TOP BACK OF CURB UNLESS STATED OTHERWISE OR WHERE NO CURB EXISTS.
2. ALL EXISTING HARDSCAPE FEATURES, LANDSCAPING AND IRRIGATION SYSTEMS THAT ARE DAMAGED OR DISTURBED DURING CONSTRUCTION ARE TO BE REPAIRED OR REPLACED AT CONTRACTORS EXPENSE.
3. ALL WORK TO COMPLY WITH HYDE PARK CITY STANDARDS AND SPECIFICATIONS.
4. ALL IMPROVEMENTS TO COMPLY WITH ADA STANDARDS.
5. ALL PAYEMENT MARKINGS TO CONFORM WITH THE LATEST EDITION OF THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD).

**LEGEND**

- PROPOSED STANDARD DUTY CONCRETE
- PROPOSED HEAVY DUTY CONCRETE
- PROPOSED ASPHALT
- PROPOSED LANDSCAPE AREA
- PROPOSED MIDDLE SCHOOL BUILDING
- PROPOSED MIDDLE SCHOOL BUILDING ADD ALTERNATE (SEE ARCHITECTURAL PLANS)
- PROPOSED SPILL CURB AND GUTTER
- PROPOSED CATCH CURB AND GUTTER
- PROPOSED ROLL CURB (TRANSITION FROM ROLL CURB TO CATCH OR SPILL CURB WITHIN 5')

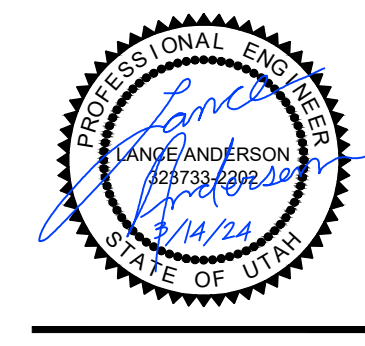
**OVERALL SITE PLAN**  
 SCALE: 1"=40'



**FOR REFERENCE ONLY**

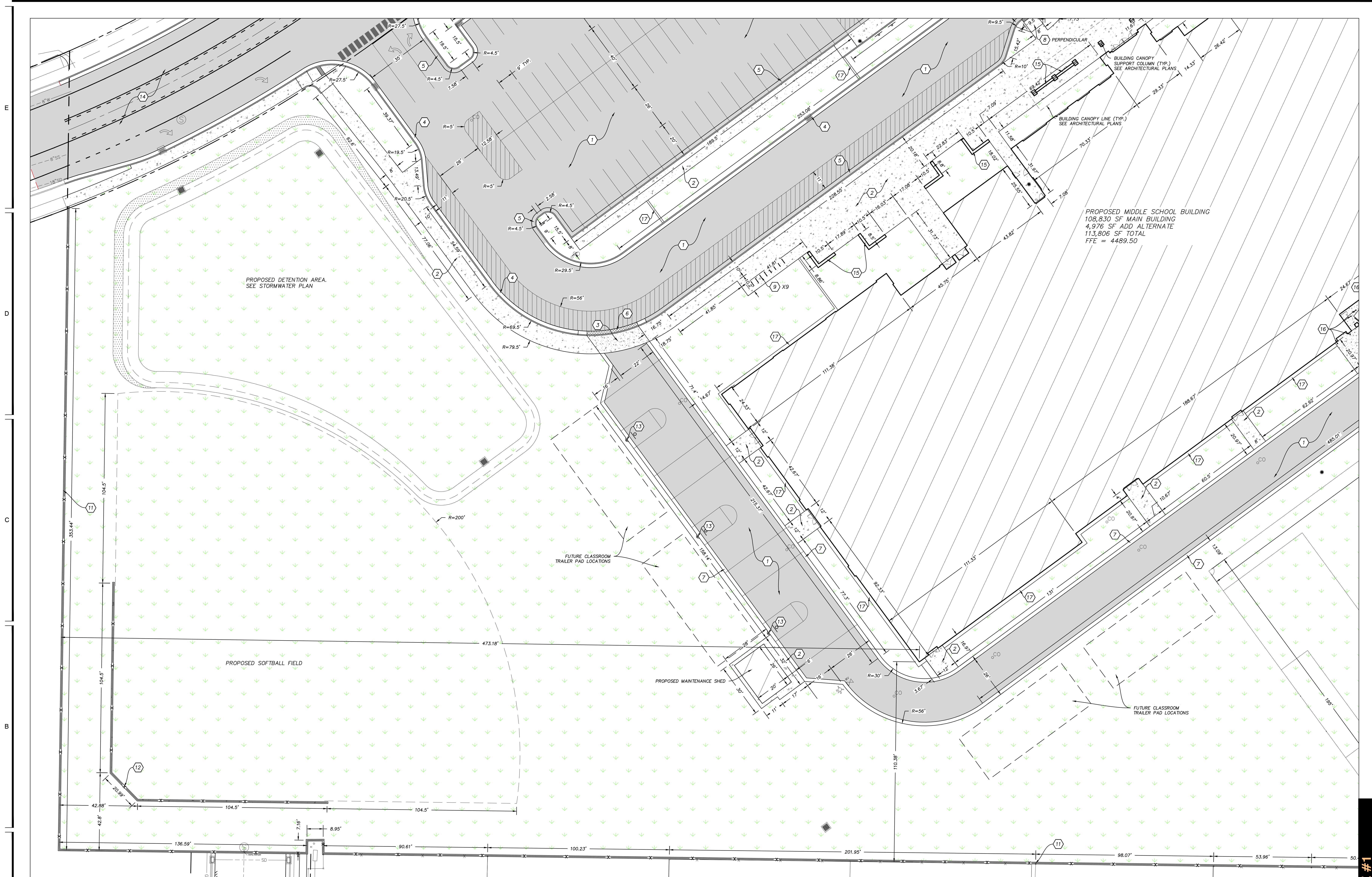
MARK	DATE	DESCRIPTION

PROJECT #: 123005  
 DRAWN BY: J. JENSEN  
 CHECKED BY: L. ANDERSON  
 ISSUED: 03.14.2024



**BID PACKAGE #1**





PROPOSED MIDDLE SCHOOL BUILDING  
 108,830 SF MAIN BUILDING  
 4,976 SF ADD ALTERNATE  
 113,806 SF TOTAL  
 FFE = 4489.50

**SITE PLAN NOTES**

- ALL DIMENSIONS AND RADII ARE MEASURED AT TOP BACK OF CURB UNLESS STATED OTHERWISE OR WHERE NO CURB EXISTS.
- ALL EXISTING HARDSCAPE FEATURES, LANDSCAPING AND IRRIGATION SYSTEMS THAT ARE DAMAGED OR DISTURBED DURING CONSTRUCTION ARE TO BE REPAIRED OR REPLACED AT CONTRACTOR'S EXPENSE.
- ALL WORK TO COMPLY WITH HYDE PARK CITY STANDARDS AND SPECIFICATIONS.
- ALL IMPROVEMENTS TO COMPLY WITH ADA STANDARDS.
- ALL PAYMENT MARKINGS TO CONFORM WITH THE LATEST EDITION OF THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD).

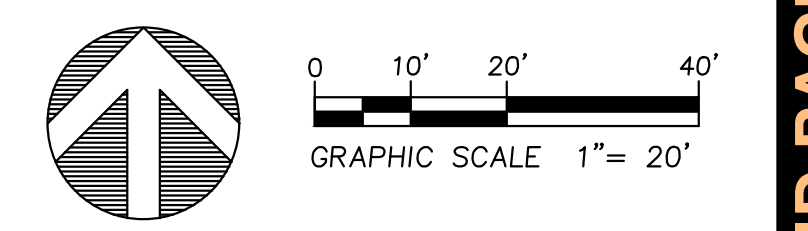
**LEGEND**

- PROPOSED STANDARD DUTY CONCRETE
- PROPOSED HEAVY DUTY CONCRETE
- PROPOSED ASPHALT
- PROPOSED LANDSCAPE AREA
- PROPOSED MIDDLE SCHOOL BUILDING
- PROPOSED MIDDLE SCHOOL BUILDING ADD ALTERNATE (SEE ARCHITECTURAL PLANS)
- PROPOSED SPILL CURB AND GUTTER
- PROPOSED CATCH CURB AND GUTTER
- PROPOSED ROLL CURB (TRANSITION FROM ROLL CURB TO CATCH OR SPILL CURB WITHIN 5')

**SITE CONSTRUCTION NOTES**

- CONSTRUCT STANDARD ASPHALT SECTION. SEE SHEET C-501/DETAIL 1.
- CONSTRUCT STANDARD DUTY CONCRETE SECTION. SEE SHEET C-501/DETAIL 2.
- CONSTRUCT HEAVY DUTY CONCRETE SECTION. SEE SHEET C-501/DETAIL 3.
- CONSTRUCT CONCRETE CATCH CURB. SEE SHEET C-501/DETAIL 4.
- CONSTRUCT CONCRETE ROLL CURB. SEE SHEET C-501/DETAIL 5.
- CONSTRUCT 2" RIBBON CURB. SEE SHEET C-501/DETAIL 6.
- CONSTRUCT ADA RAMP. SEE SHEET C-501.
- INSTALL BIKE RACK PER MANUFACTURER SPECIFICATIONS. SEE ARCHITECTURAL SITE PLAN.
- INSTALL FLAGPOLE PER MANUFACTURER SPECIFICATIONS. SEE ARCHITECTURAL SITE PLAN.
- INSTALL FENCE WITH CONCRETE CURBING. SEE ARCHITECTURAL SITE PLAN.
- INSTALL BACKSTOP FENCE WITH CONCRETE CURBING. SEE ARCHITECTURAL SITE PLAN.
- INSTALL BASKETBALL STANDARD PER MANUFACTURER SPECIFICATIONS.
- CONSTRUCT ROADWAY SECTION. SEE ROAD PLAN AND PROFILE SHEETS.
- CONSTRUCT SEAT WALL. SEE ARCHITECTURAL SITE PLAN.
- INSTALL CONCRETE BOLLARD. SEE SHEET C-501/DETAIL 7.
- CONSTRUCT CONCRETE MONOCURB. SEE ARCHITECTURAL SITE PLAN.
- INSTALL MONUMENT SIGN. SEE ARCHITECTURAL SITE PLAN.

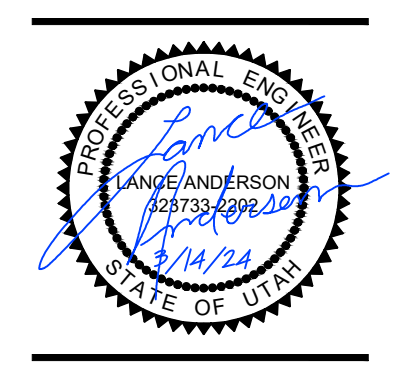
**SITE PLAN - SOUTHWEST**  
 SCALE: 1"=20'



**FOR REFERENCE ONLY**

MARK	DATE	DESCRIPTION

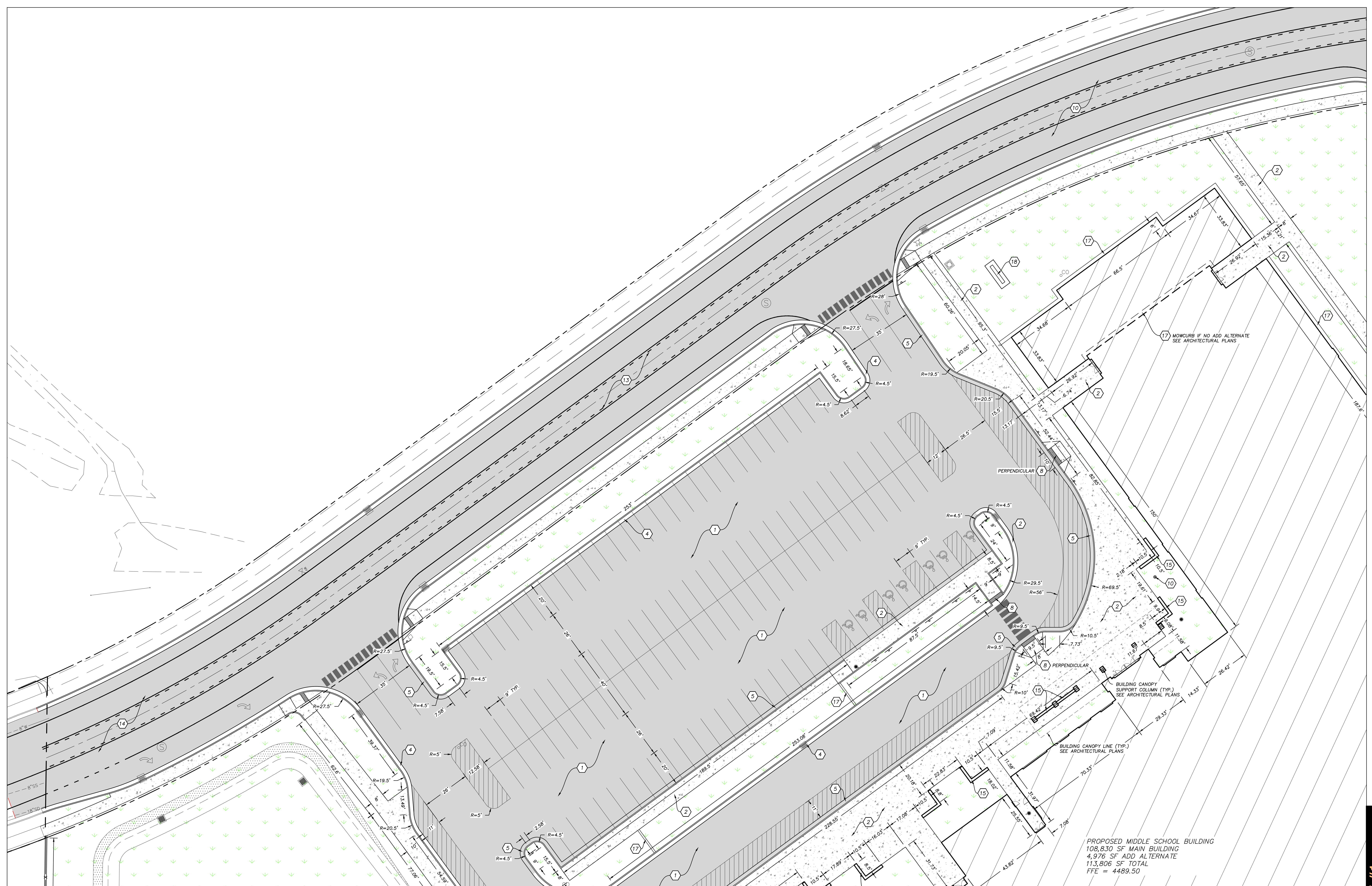
PROJECT #: 123005  
 DRAWN BY: J. JENSEN  
 CHECKED BY: L. ANDERSON  
 ISSUED: 03.14.2024



**BID PACKAGE #1**



E  
D  
C  
B  
A



PROPOSED MIDDLE SCHOOL BUILDING  
 108,630 SF MAIN BUILDING  
 4,976 SF ADD ALTERNATE  
 113,606 SF TOTAL  
 FFE = 4489.50

**SITE PLAN NOTES**

- ALL DIMENSIONS AND RADII ARE MEASURED AT TOP BACK OF CURB UNLESS STATED OTHERWISE OR WHERE NO CURB EXISTS.
- ALL EXISTING HARDSCAPE FEATURES, LANDSCAPING AND IRRIGATION SYSTEMS THAT ARE DAMAGED OR DISTURBED DURING CONSTRUCTION ARE TO BE REPAIRED OR REPLACED AT CONTRACTORS EXPENSE.
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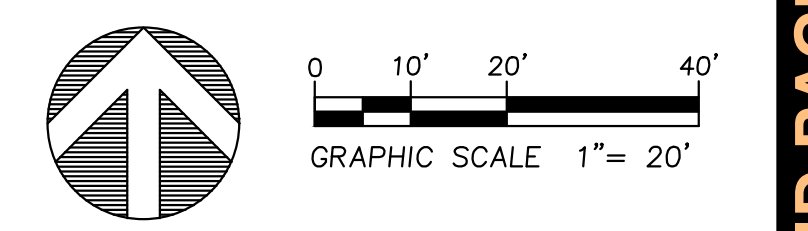
**LEGEND**

- PROPOSED STANDARD DUTY CONCRETE
- PROPOSED HEAVY DUTY CONCRETE
- PROPOSED ASPHALT
- PROPOSED LANDSCAPE AREA
- PROPOSED MIDDLE SCHOOL BUILDING
- PROPOSED MIDDLE SCHOOL BUILDING ADD ALTERNATE (SEE ARCHITECTURAL PLANS)
- PROPOSED SPILL CURB AND GUTTER
- PROPOSED CATCH CURB AND GUTTER
- PROPOSED ROLL CURB (TRANSITION FROM ROLL CURB TO CATCH OR SPILL CURB WITHIN 5')

**SITE CONSTRUCTION NOTES**

- CONSTRUCT STANDARD ASPHALT SECTION. SEE SHEET C-501/DETAIL 1.
- CONSTRUCT STANDARD DUTY CONCRETE SECTION. SEE SHEET C-501/DETAIL 2.
- CONSTRUCT HEAVY DUTY CONCRETE SECTION. SEE SHEET C-501/DETAIL 3.
- CONSTRUCT CONCRETE CATCH CURB. SEE SHEET C-501/DETAIL 4.
- CONSTRUCT CONCRETE SPILL CURB. SEE SHEET C-501/DETAIL 4.
- CONSTRUCT CONCRETE ROLL CURB. SEE SHEET C-501/DETAIL 5.
- CONSTRUCT 2" RIBBON CURB. SEE SHEET C-501/DETAIL 6.
- CONSTRUCT ADA RAMP. SEE SHEET C-501.
- INSTALL BIKE RACK PER MANUFACTURER SPECIFICATIONS. SEE ARCHITECTURAL SITE PLAN.
- INSTALL FLAGPOLE PER MANUFACTURER SPECIFICATIONS. SEE ARCHITECTURAL SITE PLAN.
- INSTALL FENCE WITH CONCRETE CURBING. SEE ARCHITECTURAL SITE PLAN.
- INSTALL BACKSTOP FENCE WITH CONCRETE CURBING. SEE ARCHITECTURAL SITE PLAN.
- INSTALL BASKETBALL STANDARD PER MANUFACTURER SPECIFICATIONS.
- CONSTRUCT ROADWAY SECTION. SEE ROAD PLAN AND PROFILE SHEETS.
- CONSTRUCT SEAT WALL. SEE ARCHITECTURAL SITE PLAN.
- INSTALL CONCRETE BOLLARD. SEE SHEET C-501/DETAIL 7.
- CONSTRUCT CONCRETE MONOCURB. SEE ARCHITECTURAL SITE PLAN.
- INSTALL MONUMENT SIGN. SEE ARCHITECTURAL SITE PLAN.

**SITE PLAN-NORTHWEST**  
 SCALE: 1"=20'

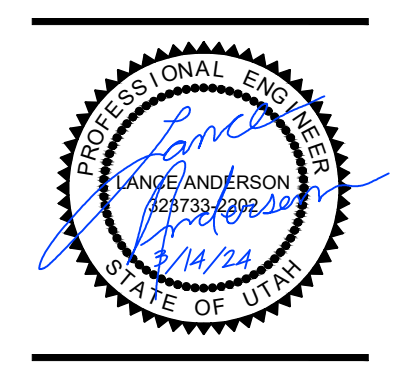


**FOR REFERENCE ONLY**

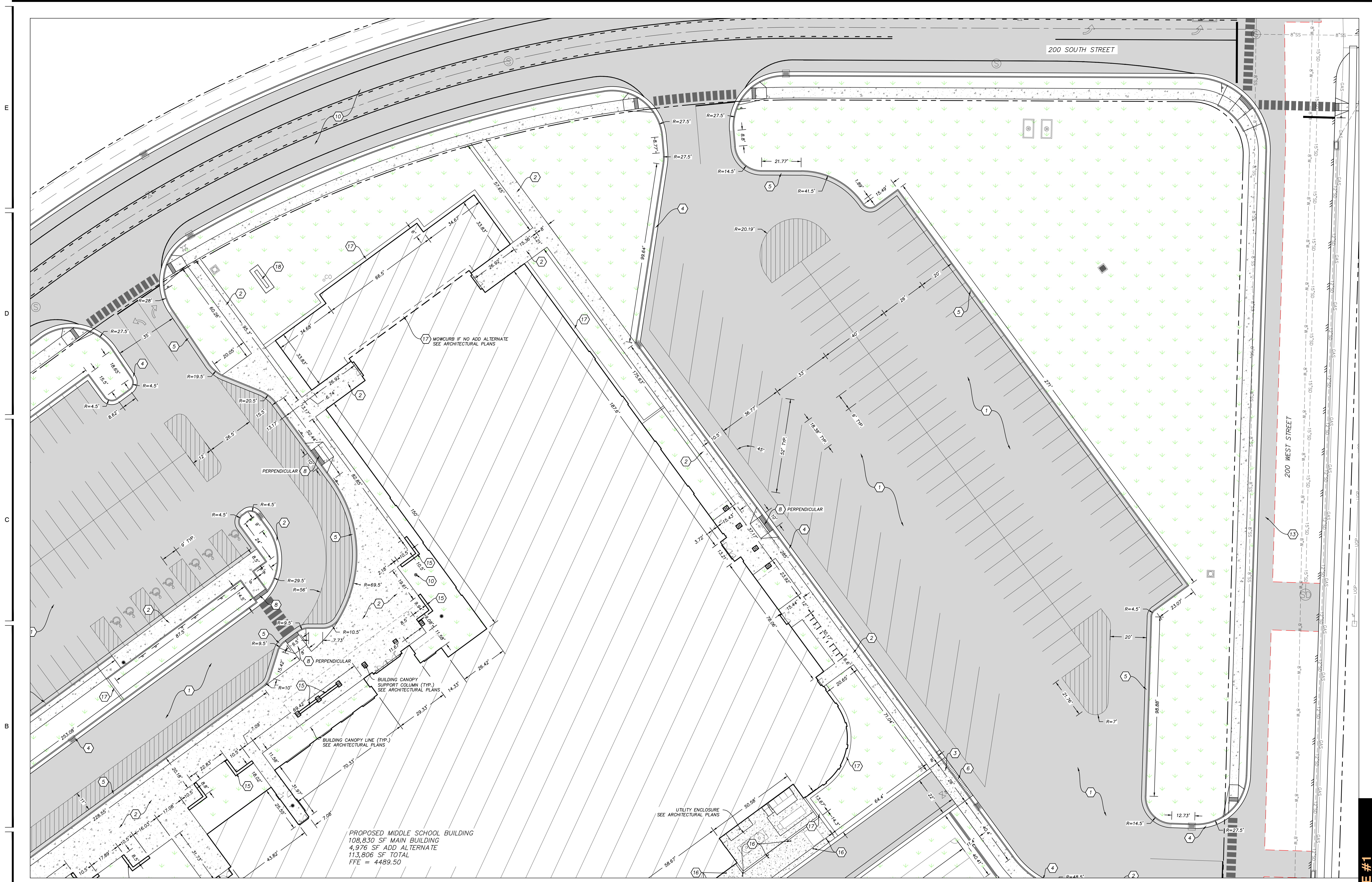
**BID PACKAGE #1**

MARK	DATE	DESCRIPTION

PROJECT #: 123005  
 DRAWN BY: J. JENSEN  
 CHECKED BY: L. ANDERSON  
 ISSUED: 03.14.2024







PROPOSED MIDDLE SCHOOL BUILDING  
 108,830 SF MAIN BUILDING  
 4,976 SF ADD ALTERNATE  
 113,806 SF TOTAL  
 FFE = 4489.50

**SITE PLAN NOTES**

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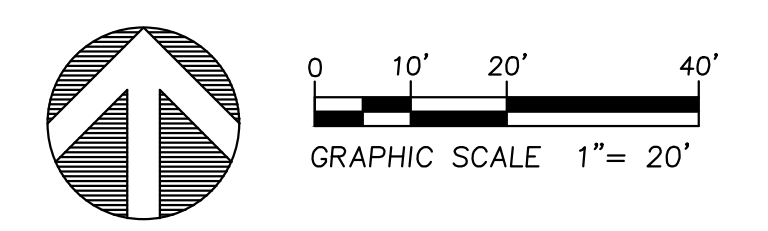
**LEGEND**

- PROPOSED STANDARD DUTY CONCRETE
- PROPOSED HEAVY DUTY CONCRETE
- PROPOSED ASPHALT
- PROPOSED LANDSCAPE AREA
- PROPOSED MIDDLE SCHOOL BUILDING
- PROPOSED MIDDLE SCHOOL BUILDING ADD ALTERNATE (SEE ARCHITECTURAL PLANS)
- PROPOSED SPILL CURB AND GUTTER
- PROPOSED CATCH CURB AND GUTTER
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**SITE CONSTRUCTION NOTES**

1. CONSTRUCT STANDARD ASPHALT SECTION. SEE SHEET C-501/DETAIL 1.
2. CONSTRUCT STANDARD DUTY CONCRETE SECTION. SEE SHEET C-501/DETAIL 2.
3. CONSTRUCT HEAVY DUTY CONCRETE SECTION. SEE SHEET C-501/DETAIL 3.
4. CONSTRUCT CONCRETE CATCH CURB. SEE SHEET C-501/DETAIL 4.
5. CONSTRUCT CONCRETE SPILL CURB. SEE SHEET C-501/DETAIL 4.
6. CONSTRUCT CONCRETE ROLL CURB. SEE SHEET C-501/DETAIL 5.
7. CONSTRUCT 2" RIBBON CURB. SEE SHEET C-501/DETAIL 6.
8. CONSTRUCT ADA RAMP. SEE SHEET C-501.
9. INSTALL BIKE RACK PER MANUFACTURER SPECIFICATIONS. SEE ARCHITECTURAL SITE PLAN.
10. INSTALL FLAGPOLE PER MANUFACTURER SPECIFICATIONS. SEE ARCHITECTURAL SITE PLAN.
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15. CONSTRUCT SEAT WALL. SEE ARCHITECTURAL SITE PLAN.
16. INSTALL CONCRETE BOLLARD. SEE SHEET C-501/DETAIL 7.
17. CONSTRUCT CONCRETE MONOCURB. SEE ARCHITECTURAL SITE PLAN.
18. INSTALL MONUMENT SIGN. SEE ARCHITECTURAL SITE PLAN.

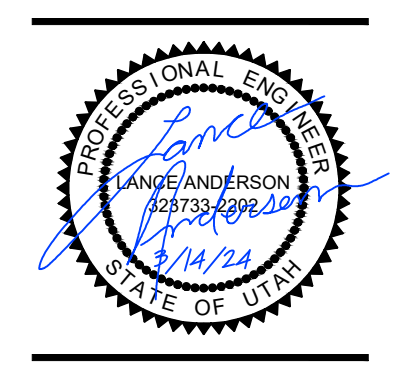
SITE PLAN-NORTHEAST  
 SCALE: 1"=20'



FOR REFERENCE ONLY

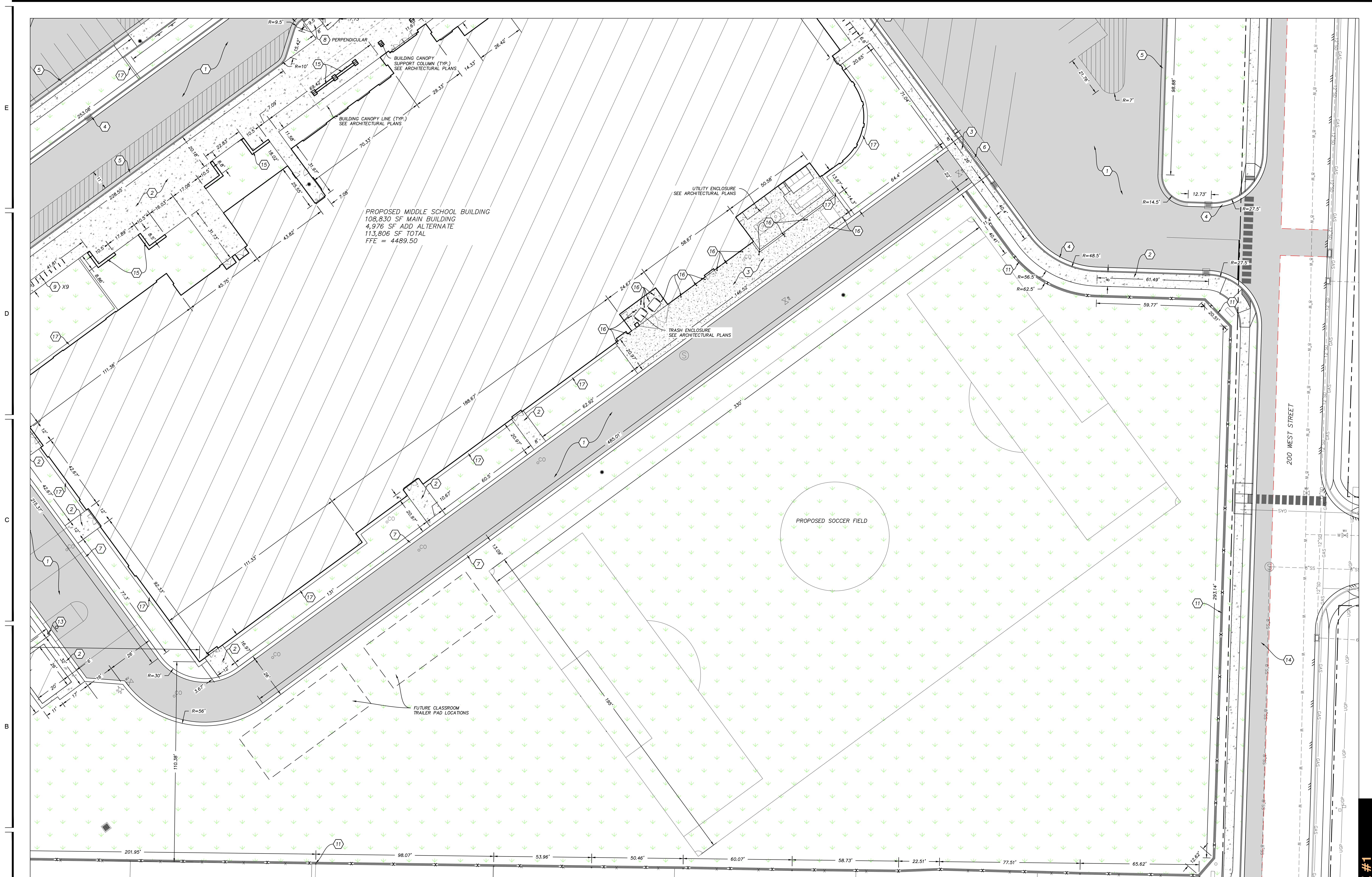
MARK	DATE	DESCRIPTION

PROJECT #: 123005  
 DRAWN BY: J. JENSEN  
 CHECKED BY: L. ANDERSON  
 ISSUED: 03.14.2024



BID PACKAGE #1





PROPOSED MIDDLE SCHOOL BUILDING  
 108,830 SF MAIN BUILDING  
 4,976 SF ADD ALTERNATE  
 113,806 SF TOTAL  
 FFE = 4489.50

PROPOSED SOCCER FIELD

**SITE PLAN NOTES**

1. ALL DIMENSIONS AND RADII ARE MEASURED AT TOP BACK OF CURB UNLESS STATED OTHERWISE OR WHERE NO CURB EXISTS.
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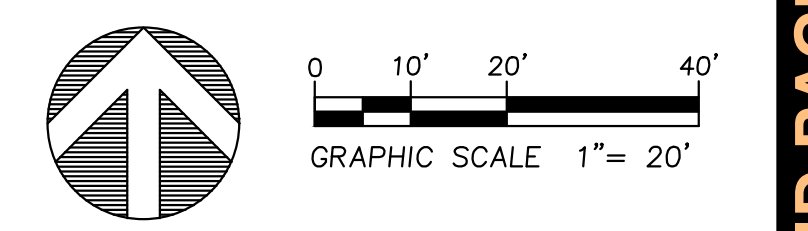
**LEGEND**

- PROPOSED STANDARD DUTY CONCRETE
- PROPOSED HEAVY DUTY CONCRETE
- PROPOSED ASPHALT
- PROPOSED LANDSCAPE AREA
- PROPOSED MIDDLE SCHOOL BUILDING
- PROPOSED MIDDLE SCHOOL BUILDING ADD ALTERNATE (SEE ARCHITECTURAL PLANS)
- PROPOSED SPILL CURB AND GUTTER
- PROPOSED CATCH CURB AND GUTTER
- PROPOSED ROLL CURB (TRANSITION FROM ROLL CURB TO CATCH OR SPILL CURB WITHIN 5')

**SITE CONSTRUCTION NOTES**

1. CONSTRUCT STANDARD ASPHALT SECTION. SEE SHEET C-501/DETAIL 1.
2. CONSTRUCT STANDARD DUTY CONCRETE SECTION. SEE SHEET C-501/DETAIL 2.
3. CONSTRUCT HEAVY DUTY CONCRETE SECTION. SEE SHEET C-501/DETAIL 3.
4. CONSTRUCT CONCRETE CATCH CURB. SEE SHEET C-501/DETAIL 4.
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6. CONSTRUCT CONCRETE ROLL CURB. SEE SHEET C-501/DETAIL 5.
7. CONSTRUCT 2" RIBBON CURB. SEE SHEET C-501/DETAIL 6.
8. CONSTRUCT ADA RAMP. SEE SHEET C-501.
9. INSTALL BIKE RACK PER MANUFACTURER SPECIFICATIONS. SEE ARCHITECTURAL SITE PLAN.
10. INSTALL FLAGPOLE PER MANUFACTURER SPECIFICATIONS. SEE ARCHITECTURAL SITE PLAN.
11. INSTALL FENCE WITH CONCRETE CURBING. SEE ARCHITECTURAL SITE PLAN.
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16. INSTALL CONCRETE BOLLARD. SEE SHEET C-501/DETAIL 7.
17. CONSTRUCT CONCRETE MONOCURB. SEE ARCHITECTURAL SITE PLAN.
18. INSTALL MONUMENT SIGN. SEE ARCHITECTURAL SITE PLAN.

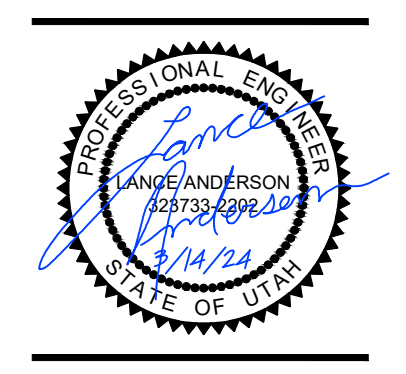
SITE PLAN - SOUTHEAST  
 SCALE: 1"=20'



FOR REFERENCE ONLY

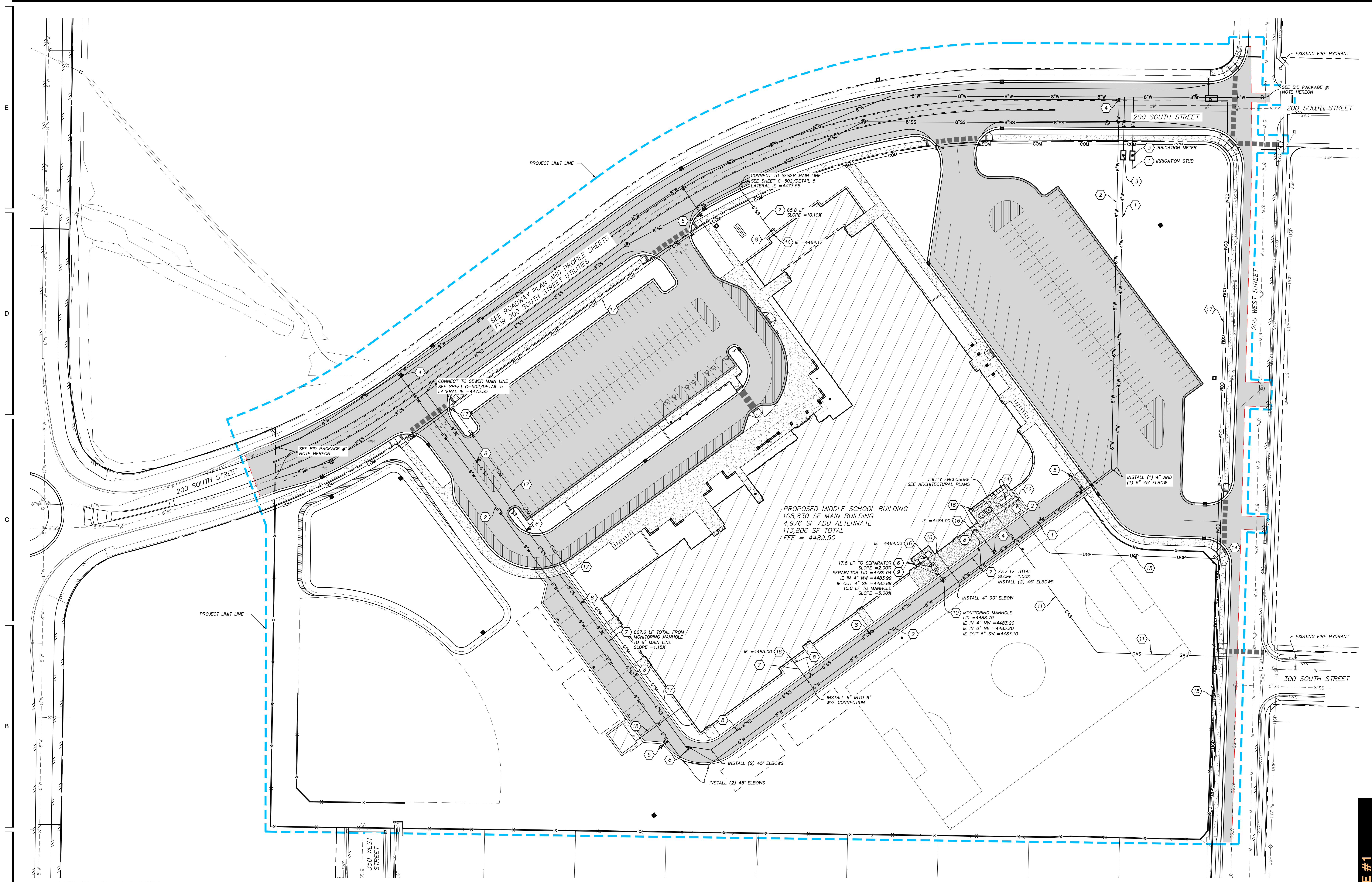
MARK	DATE	DESCRIPTION

PROJECT #: 123005  
 DRAWN BY: J. JENSEN  
 CHECKED BY: L. ANDERSON  
 ISSUED: 03.14.2024



BID PACKAGE #1





PROPOSED MIDDLE SCHOOL BUILDING  
 108,830 SF MAIN BUILDING  
 4,976 SF ADD ALTERNATE  
 113,806 SF TOTAL  
 FFE = 4489.50

**UTILITY PLAN NOTES**

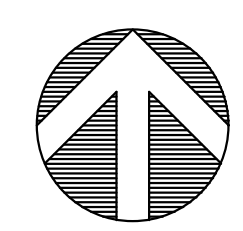
1. CONTRACTOR SHALL VERIFY ALL EXISTING UTILITY LOCATIONS AND ELEVATIONS PRIOR TO CONSTRUCTION, AND NOTIFY ENGINEER OF ANY CONFLICTS WITH THE PROPOSED PLAN.
2. ALL EXISTING UTILITIES TO BE PROTECTED IN PLACE UNLESS NOTED OTHERWISE ON DEMO PLAN.
3. ALL EXISTING HARDSCAPE FEATURES, LANDSCAPING AND IRRIGATION SYSTEMS THAT ARE DAMAGED OR DISTURBED DURING CONSTRUCTION ARE TO BE REPAIRED OR REPLACED AT CONTRACTOR'S EXPENSE.
4. ALL WORK TO COMPLY WITH GOVERNING AGENCIES STANDARDS AND SPECIFICATIONS. ALL STORM DRAIN BOXES TO HAVE 1 FOOT MINIMUM SUMP UNLESS SPECIFIED OTHERWISE.
5. CONTRACTOR TO FIELD VERIFY ALL DEPTHS AND LOCATIONS OF UTILITY CONNECTIONS.
6. ALL WATER FITTINGS TO HAVE THRUST BLOCK, SEE SHEET C-503/DETAIL 3.
7. ALL WATER LINES TO BE C900 PVC.
8. ALL SEWER LINES TO BE SDR-35 PVC.
9. ALL WATER LINES TO HAVE A MINIMUM OF 5' OF COVER TO FG.
10. ALL STORMWATER PIPE TO HAVE MINIMUM OF 18" OF COVER TO FG UNLESS NOTED OTHERWISE.
11. WATER AND SEWER LINES TO HAVE MINIMUM 18" OF VERTICAL SEPARATION WHERE CONFLICTS OCCUR. STORMWATER LINES TO HAVE 6" MINIMUM VERTICAL SEPARATION FROM OTHER UTILITY LINES. EXCEPTIONS NOTED. LOOP WATER LINES WHERE NECESSARY.

**UTILITY CONSTRUCTION NOTES**

1. INSTALL 4" WATER LINE. SEE SHEET C-502/DETAIL 1.
2. INSTALL 6" WATER LINE. SEE SHEET C-502/DETAIL 1.
3. INSTALL 4" WATER METER. SEE SHEET C-502/DETAIL 2.
4. INSTALL 6" WATER VALVE. SEE SHEET C-502/DETAIL 4.
5. INSTALL FIRE HYDRANT. SEE SHEET C-502/DETAIL 3.
6. INSTALL 4" SEWER LINE. SEE SHEET C-502/DETAIL 1.
7. INSTALL 6" SEWER LINE. SEE SHEET C-502/DETAIL 1.
8. INSTALL SEWER CLEANOUT WITHIN 5' OF BUILDING AND EVERY 100 LF. SEE SHEET C-503/DETAIL 2.
9. INSTALL 1,500 GALLON GREASE INTERCEPTOR. SEE SHEET C-503/DETAIL 1.
10. INSTALL SEWER MANHOLE. SEE SHEET C-502/DETAIL 6.

11. INSTALL GAS LINE. COORDINATE SIZE AND CONNECTION WITH DOMINION ENERGY.
12. INSTALL POWER TRANSFORMER. COORDINATE WITH ROCKY MOUNTAIN POWER. SEE ELECTRICAL PLANS.
13. INSTALL POWER JUNCTION BOX. COORDINATE WITH ROCKY MOUNTAIN POWER. SEE ELECTRICAL PLANS.
14. INSTALL BACKUP POWER GENERATOR. COORDINATE WITH ROCKY MOUNTAIN POWER. SEE ELECTRICAL PLANS.
15. INSTALL UNDERGROUND POWER LINE. SEE ELECTRICAL PLANS.
16. COORDINATE BUILDING CONNECTION WITH MECHANICAL PLANS.
17. INSTALL COMMUNICATION LINE AND JUNCTION BOXES. COORDINATE WITH RESPECTIVE UTILITY COMPANY. SEE ELECTRICAL PLANS.
18. INSTALL 3/4" WATER LINE TO MAINTENANCE SHED. SEE PLUMBING PLANS FOR CONNECTION TO BUILDINGS.

**BID PACKAGE #1 NOTE:**  
 PRIOR TO CONSTRUCTION, CONTRACTOR TO FIELD VERIFY ALL EXISTING UTILITY LINE LOCATIONS INCLUDING, BUT NOT LIMITED TO, VERIFYING EXISTING WATER LINE AND SEWER LINE STUB ELEVATIONS AT EXISTING END OF 200 SOUTH STREET AND AT INTERSECTION OF 200 WEST AND 200 SOUTH. NOTIFY ENGINEER OF EXISTING SEWER LINE ELEVATIONS.

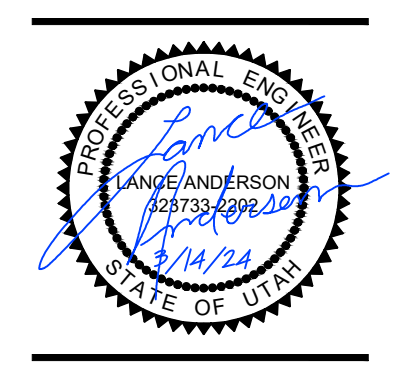


**UTILITY PLAN**  
 SCALE: 1"=40'  
 GRAPHIC SCALE 1"= 40'

**BID PACKAGE #1**

MARK	DATE	DESCRIPTION

PROJECT #: 123005  
 DRAWN BY: J. JENSEN  
 CHECKED BY: L. ANDERSON  
 ISSUED: 03.14.2024



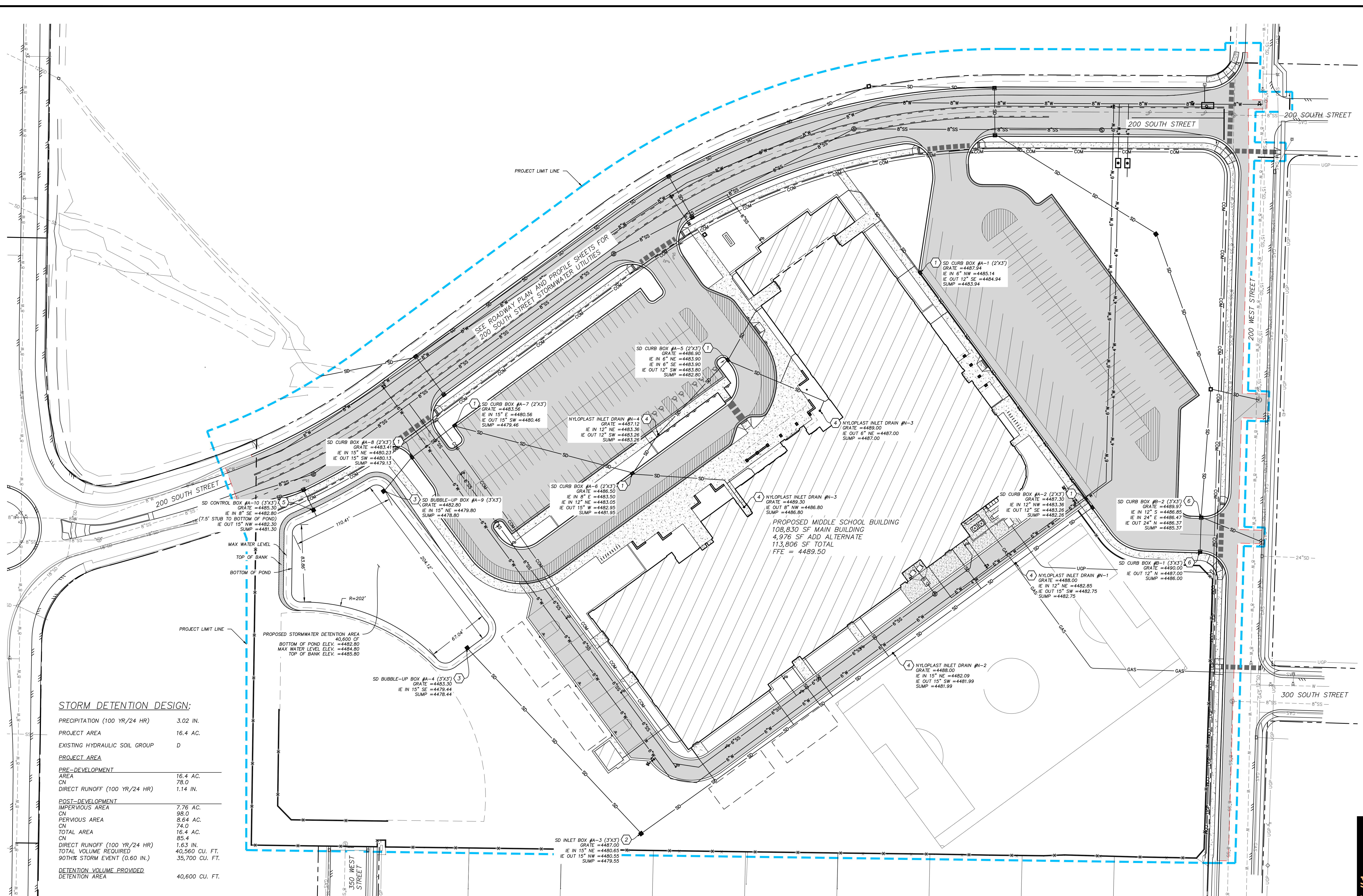


MARK	DATE	DESCRIPTION

PROJECT #: 123005  
 DRAWN BY: J. JENSEN  
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 ISSUED: 03.14.2024



BID PACKAGE #1



**STORM DETENTION DESIGN:**

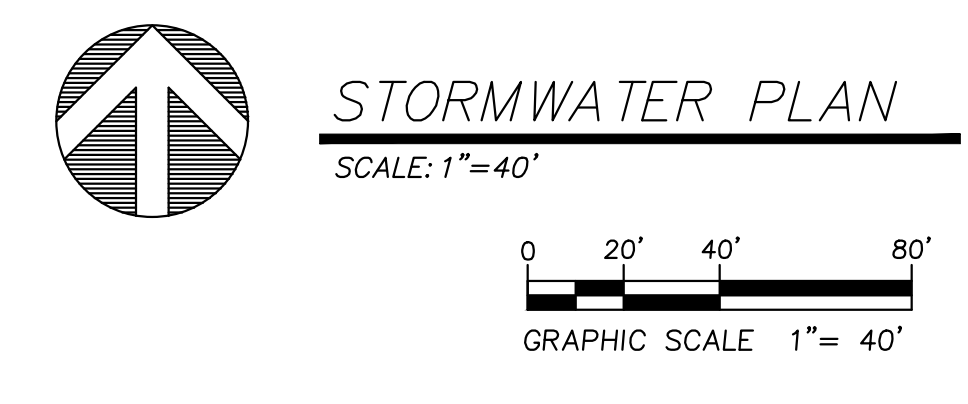
PRECIPITATION (100 YR/24 HR)	3.02 IN.
PROJECT AREA	16.4 AC.
EXISTING HYDRAULIC SOIL GROUP	D
<b>PROJECT AREA</b>	
<b>PRE-DEVELOPMENT</b>	
AREA	16.4 AC.
CN	78.0
DIRECT RUNOFF (100 YR/24 HR)	1.14 IN.
<b>POST-DEVELOPMENT</b>	
IMPERVIOUS AREA	7.76 AC.
CN	98.0
PERVIOUS AREA	8.64 AC.
CN	74.0
TOTAL AREA	16.4 AC.
CN	85.4
DIRECT RUNOFF (100 YR/24 HR)	1.63 IN.
TOTAL VOLUME REQUIRED	40,560 CU. FT.
90TH% STORM EVENT (0.60 IN.)	35,700 CU. FT.
<b>DETENTION VOLUME PROVIDED</b>	
DETENTION AREA	40,600 CU. FT.

PROPOSED STORMWATER DETENTION AREA  
 40,600 CF  
 BOTTOM OF POND ELEV. = 4482.80  
 MAX WATER LEVEL ELEV. = 4484.80  
 TOP OF BANK ELEV. = 4485.80

PROPOSED MIDDLE SCHOOL BUILDING  
 108,830 SF MAIN BUILDING  
 4,976 SF ADD ALTERNATE  
 113,806 SF TOTAL  
 FFE = 4489.50

- STORMWATER CONSTRUCTION NOTES**
1. INSTALL 2'X3' STORM DRAIN CURB INLET BOX. SEE SHEET C-504/DETAIL 1.
  2. INSTALL 3'X3' STORM DRAIN INLET BOX. SEE SHEET C-504/DETAIL 2.
  3. INSTALL 3'X3' STORM DRAIN BUBBLE-UP BOX. SEE SHEET C-504/DETAIL 3.
  4. INSTALL 18" NYLOPLAST (OR EQUIVALENT) IN-LINE PLASTIC DRAIN BOX. SEE SHEET C-504/DETAIL 4.
  5. INSTALL 3'X3' STORM DRAIN CONTROL BOX. SEE SHEET C-504/DETAIL 7.
  6. INSTALL 3'X3' STORM DRAIN CURB INLET BOX. SEE SHEET C-504/DETAIL 5.
  7. INSTALL STORM DRAIN PIPE. SIZE AND TYPE AS LISTED ON PLAN. SEE SHEET C-504/DETAIL 6.
  8. CONNECT PVC STORM DRAIN PIPE TO BUILDING ROOF DRAINS. COORDINATE WITH PLUMBING PLANS.

- UTILITY PLAN NOTES**
1. CONTRACTOR SHALL VERIFY ALL EXISTING UTILITY LOCATIONS AND ELEVATIONS PRIOR TO CONSTRUCTION, AND NOTIFY ENGINEER OF ANY CONFLICTS WITH THE PROPOSED PLAN.
  2. ALL EXISTING UTILITIES TO BE PROTECTED IN PLACE UNLESS NOTED OTHERWISE ON DEMO PLAN.
  3. ALL EXISTING HARDSCAPE FEATURES, LANDSCAPING AND IRRIGATION SYSTEMS THAT ARE DAMAGED OR DISTURBED DURING CONSTRUCTION ARE TO BE REPAIRED OR REPLACED AT CONTRACTORS EXPENSE.
  4. ALL WORK TO COMPLY WITH GOVERNING AGENCIES STANDARDS AND SPECIFICATIONS. ALL STORM DRAIN BOXES TO HAVE 1 FOOT MINIMUM SUMP UNLESS SPECIFIED OTHERWISE.
  5. CONTRACTOR TO FIELD VERIFY ALL DEPTHS AND LOCATIONS OF UTILITY CONNECTIONS.
  6. ALL WATER FITTINGS TO HAVE THRUST BLOCK, SEE SHEET C-503/DETAIL 3.
  7. ALL WATER LINES TO BE C900 PVC.
  8. ALL SEWER LINES TO BE SDR-35 PVC.
  9. ALL WATER LINES TO HAVE A MINIMUM OF 5' OF COVER TO FG.
  10. ALL STORMWATER PIPE TO HAVE MINIMUM OF 18" OF COVER TO FG UNLESS NOTED OTHERWISE.
  11. WATER AND SEWER LINES TO HAVE MINIMUM 18" OF VERTICAL SEPARATION WHERE CONFLICTS OCCUR. STORMWATER LINES TO HAVE 6" MINIMUM VERTICAL SEPARATION FROM OTHER UTILITY LINES. EXCEPTIONS NOTED. LOOP WATER LINES WHERE NECESSARY.

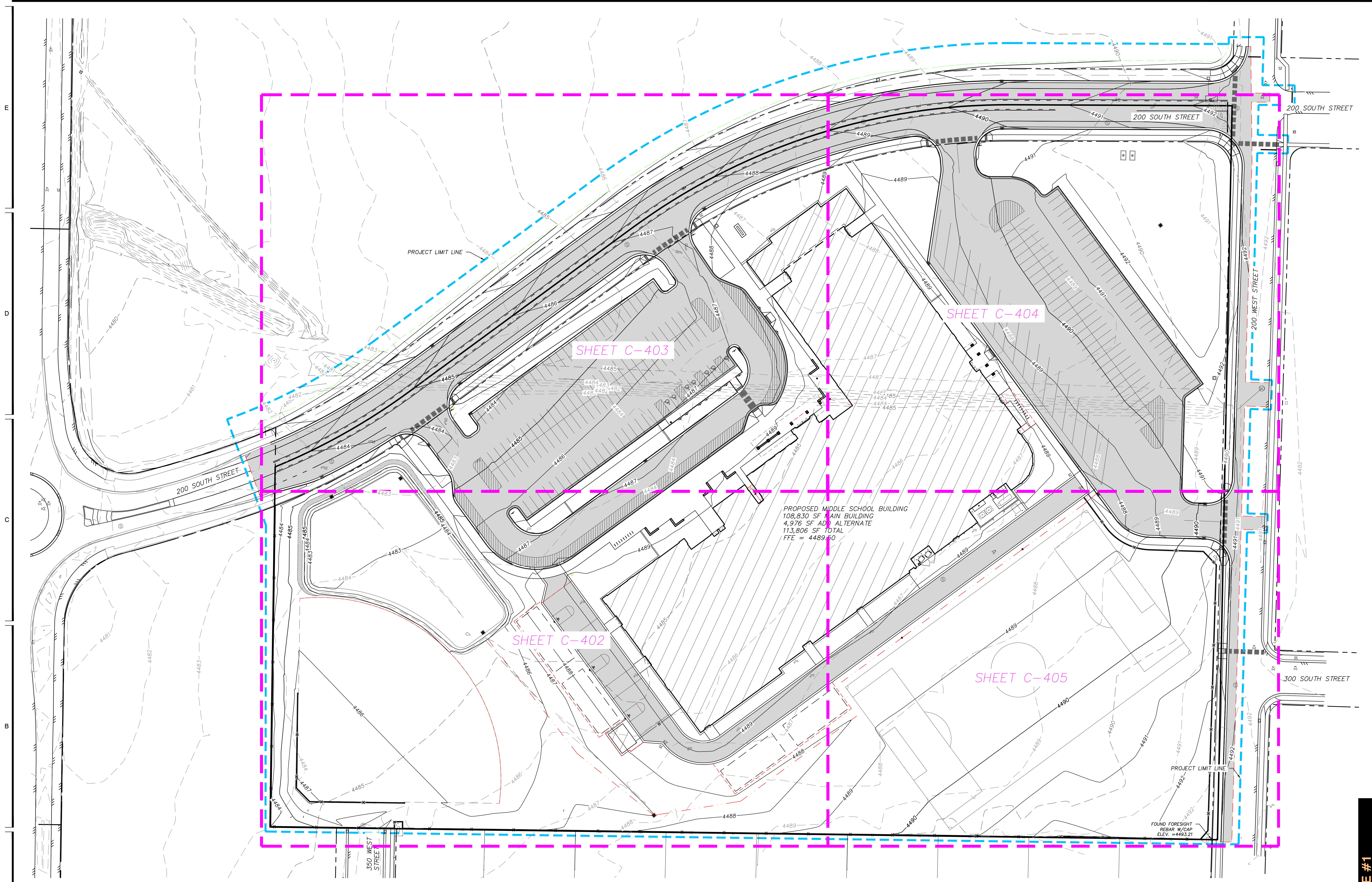


FOR REFERENCE ONLY

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1 2 3 4 5 6





**GRADING NOTES**

1. CONTRACTOR SHALL MATCH GRADES AT EXISTING ASPHALT AND CONCRETE.
2. TYPICAL CONTOUR INTERVAL IS 1.0' FOR EXISTING AND 1.0' FOR PROPOSED.
3. ALL SLOPE ARROWS ARE ORIENTED TOWARD THE DIRECTION OF WATER FLOW ON THE SURFACE. SIDEWALK CROSS-SLOPE NOT TO EXCEED 2.0%.
4. SLOPES IN ADA PARKING AREA NOT TO EXCEED 2.0% IN ANY DIRECTION.
5. SEE ROADWAY PLAN AND PROFILE SHEETS FOR ROADWAY GRADING.

**GRADING LEGEND**

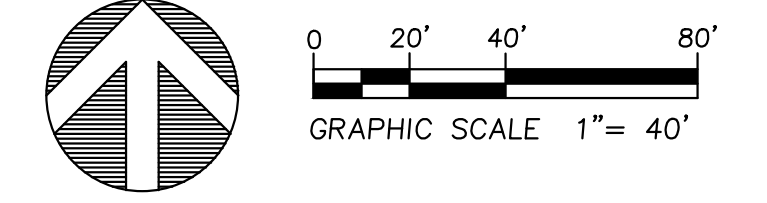
- 4500 — EXIST. MAJOR CONTOUR (5' INT)
- 4501 — EXIST. MINOR CONTOUR (1' INT)
- 4500 — PROP. MAJOR CONTOUR (5' INT)
- 4501 — PROP. MINOR CONTOUR (1' INT)
- - - - - GRADE BREAK
- - - - - GRADE BREAK-SWALE
- — — DAYLIGHT LINE

**SPOT ELEVATION LEGEND**

- TA: XX.XX TOP OF ASPHALT
- TG: XX.XX TOP OF GRAVEL
- TC: XX.XX TOP OF CONCRETE
- TBC XX.XX TOP BACK OF CURB
- TBRC XX.XX TOP BACK OF RIBBON CURB
- GB: GRADE BREAK
- FFE: FINISHED FLOOR ELEVATION
- HP: LOCAL HIGH POINT
- LP: LOCAL LOW POINT
- MATCH: MATCH EXISTING ELEVATION

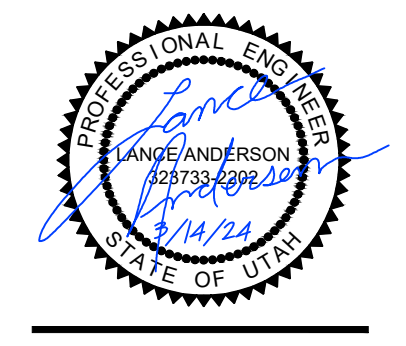
**OVERALL GRADING PLAN**

SCALE: 1"=40'



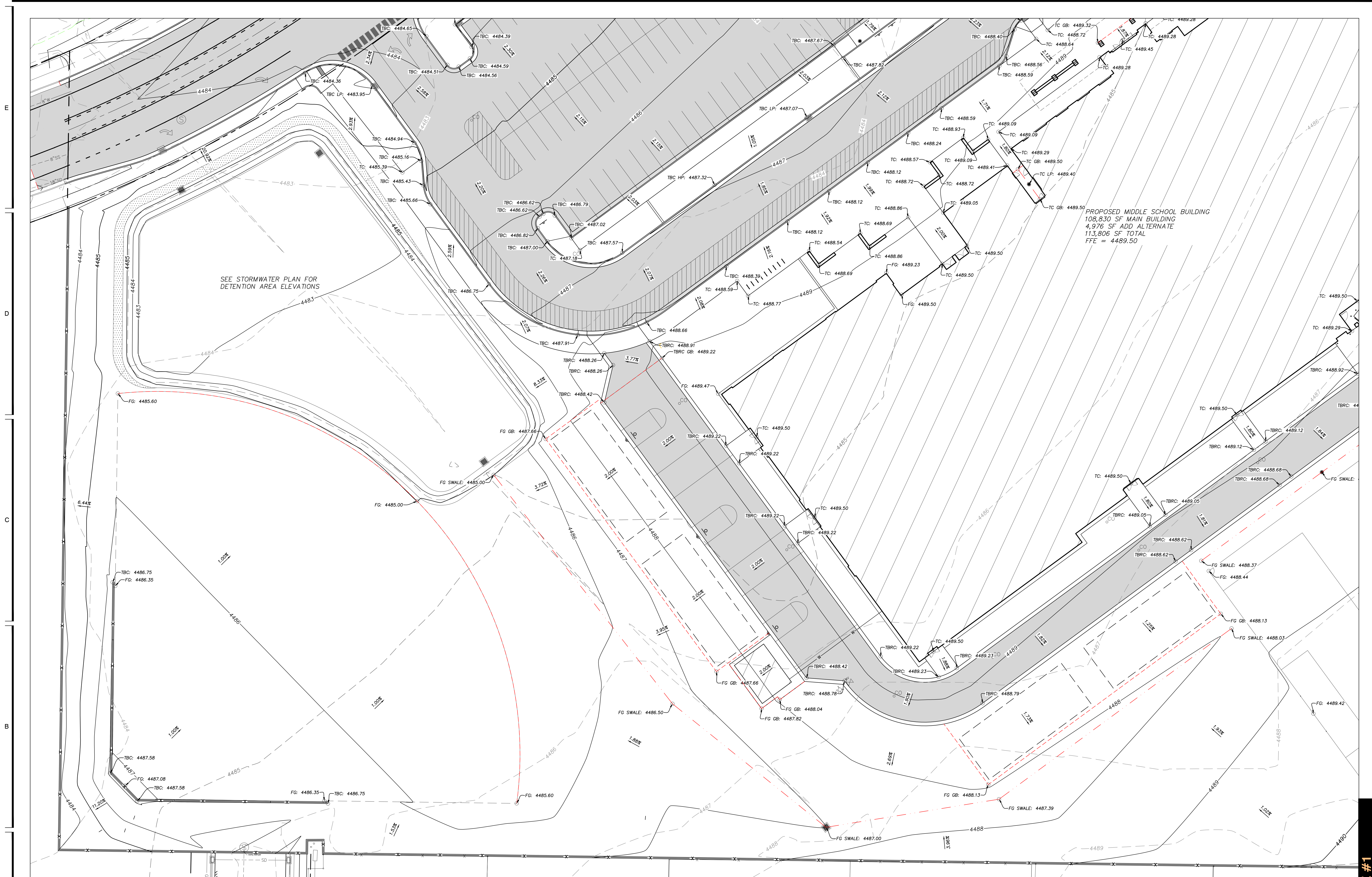
MARK	DATE	DESCRIPTION

PROJECT #: 123005  
 DRAWN BY: J. JENSEN  
 CHECKED BY: L. ANDERSON  
 ISSUED: 03.14.2024



**BID PACKAGE #1**





PROPOSED MIDDLE SCHOOL BUILDING  
 108,830 SF MAIN BUILDING  
 4,976 SF ADD ALTERNATE  
 113,806 SF TOTAL  
 FFE = 4489.50

SEE STORMWATER PLAN FOR  
 DETENTION AREA ELEVATIONS

**GRADING NOTES**

- CONTRACTOR SHALL MATCH GRADES AT EXISTING ASPHALT AND CONCRETE.
- TYPICAL CONTOUR INTERVAL IS 1.0' FOR EXISTING AND 1.0' FOR PROPOSED.
- ALL SLOPE ARROWS ARE ORIENTED TOWARD THE DIRECTION OF WATER FLOW ON THE SURFACE. SIDEWALK CROSS-SLOPE NOT TO EXCEED 2.0%.
- SLOPES IN ADA PARKING AREA NOT TO EXCEED 2.0% IN ANY DIRECTION.
- SEE ROADWAY PLAN AND PROFILE SHEETS FOR ROADWAY GRADING.

**GRADING LEGEND**

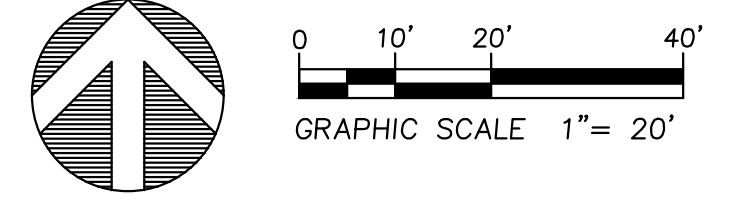
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- - - - - GRADE BREAK-SWALE
- — — DAYLIGHT LINE

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- GB: GRADE BREAK
- FFE: FINISHED FLOOR ELEVATION
- HP: LOCAL HIGH POINT
- LP: LOCAL LOW POINT
- MATCH: MATCH EXISTING ELEVATION

**GRADING PLAN-SOUTHWEST**

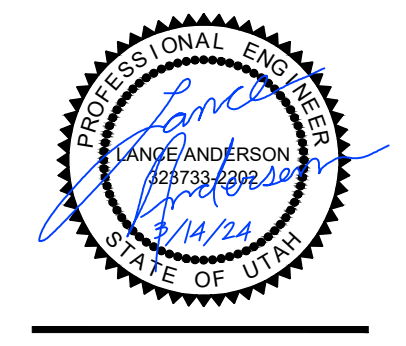
SCALE: 1"=20'



**BID PACKAGE #1**

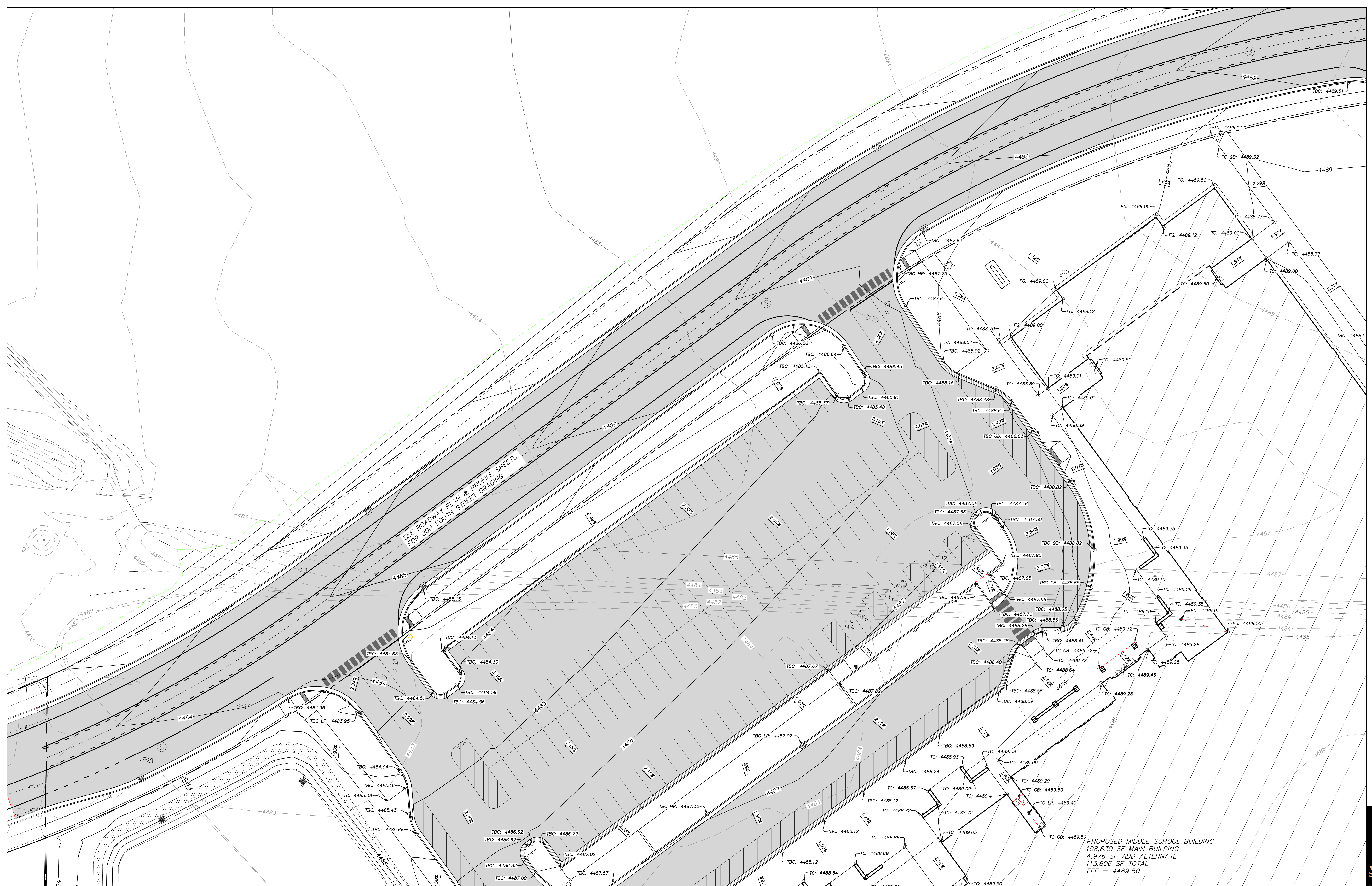
MARK	DATE	DESCRIPTION

PROJECT #: 123005  
 DRAWN BY: J. JENSEN  
 CHECKED BY: L. ANDERSON  
 ISSUED: 03.14.2024





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**GRADING NOTES**

1. CONTRACTOR SHALL MATCH GRADES AT EXISTING ASPHALT AND CONCRETE.
2. TYPICAL CONTOUR INTERVAL IS 1.0' FOR EXISTING AND 1.0' FOR PROPOSED.
3. ALL SLOPE ARROWS ARE ORIENTED TOWARD THE DIRECTION OF WATER FLOW ON THE SURFACE. SIDEWALK CROSS-SLOPE NOT TO EXCEED 2.0%.
4. SLOPES IN ADA PARKING AREA NOT TO EXCEED 2.0% IN ANY DIRECTION.
5. SEE ROADWAY PLAN AND PROFILE SHEETS FOR ROADWAY GRADING.

**GRADING LEGEND**

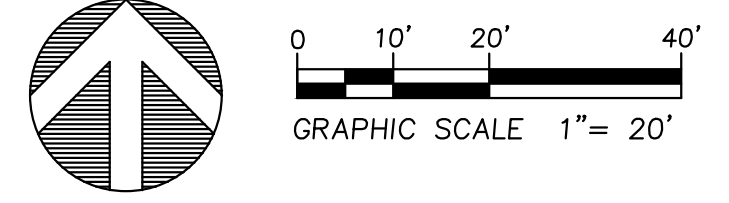
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- — — DAYLIGHT LINE

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- GB: GRADE BREAK
- FFE: FINISHED FLOOR ELEVATION
- HP: LOCAL HIGH POINT
- LP: LOCAL LOW POINT
- MATCH: MATCH EXISTING ELEVATION

**GRADING PLAN-NORTHWEST**

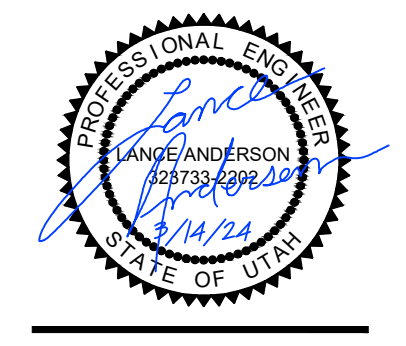
SCALE: 1"=20'



**BID PACKAGE #1**

MARK	DATE	DESCRIPTION

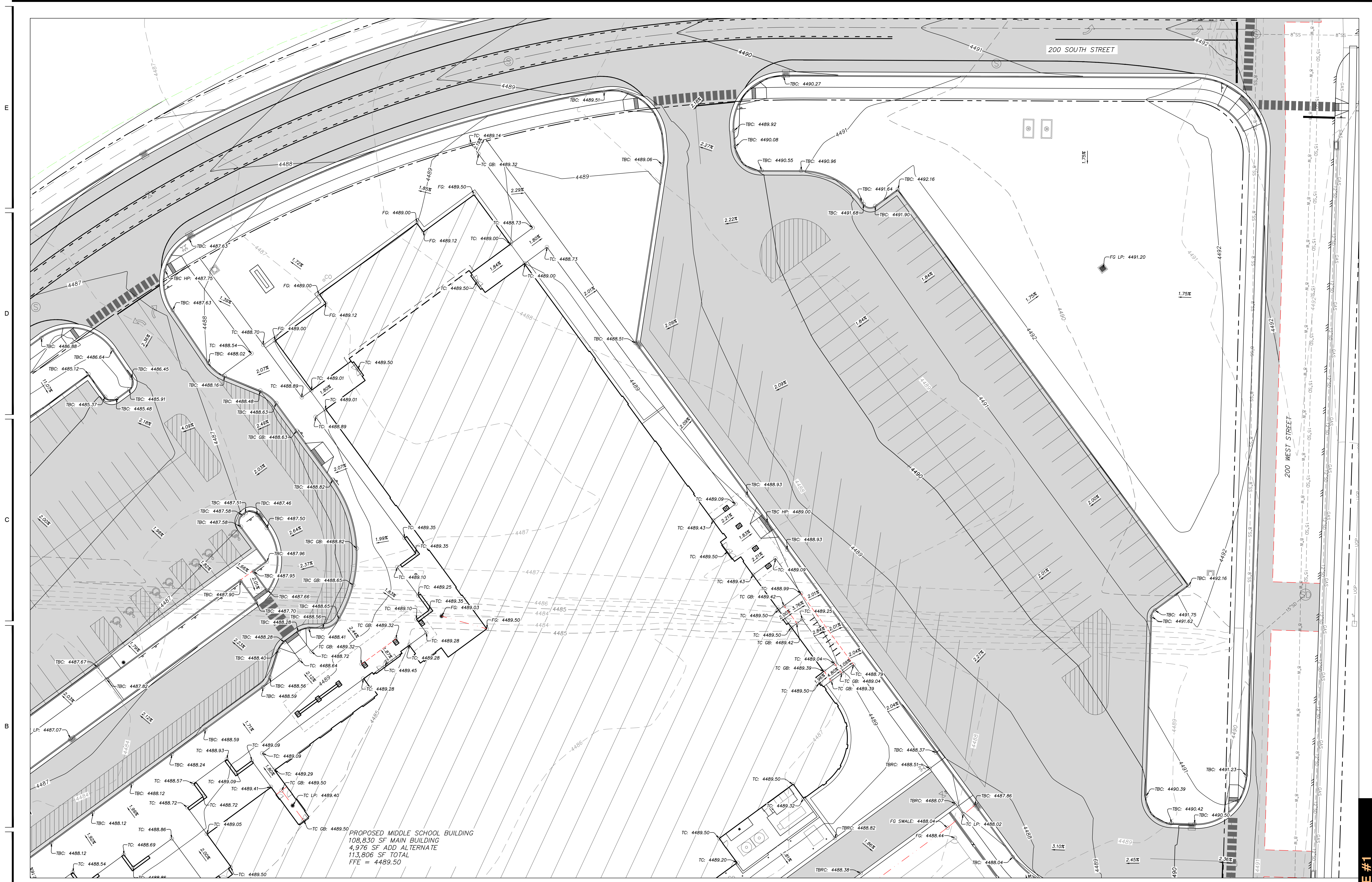
PROJECT #: 123005  
 DRAWN BY: J. JENSEN  
 CHECKED BY: L. ANDERSON  
 ISSUED: 03.14.2024



CIVIL - GRADING  
 PLAN NORTHWEST

C-403





PROPOSED MIDDLE SCHOOL BUILDING  
 108,830 SF MAIN BUILDING  
 4,976 SF ADD ALTERNATE  
 113,806 SF TOTAL  
 FFE = 4489.50

**GRADING NOTES**

1. CONTRACTOR SHALL MATCH GRADES AT EXISTING ASPHALT AND CONCRETE.
2. TYPICAL CONTOUR INTERVAL IS 1.0' FOR EXISTING AND 1.0' FOR PROPOSED.
3. ALL SLOPE ARROWS ARE ORIENTED TOWARD THE DIRECTION OF WATER FLOW ON THE SURFACE. SIDEWALK CROSS-SLOPE NOT TO EXCEED 2.0%.
4. SLOPES IN ADA PARKING AREA NOT TO EXCEED 2.0% IN ANY DIRECTION.
5. SEE ROADWAY PLAN AND PROFILE SHEETS FOR ROADWAY GRADING.

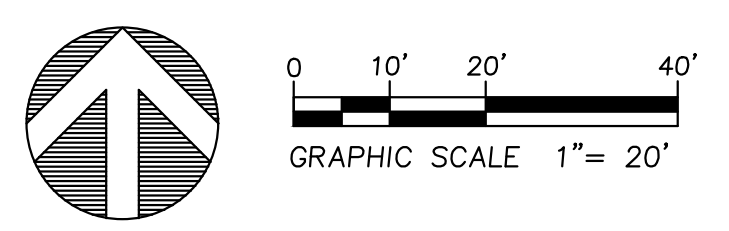
**GRADING LEGEND**

- 4500 — EXIST. MAJOR CONTOUR (5' INT)
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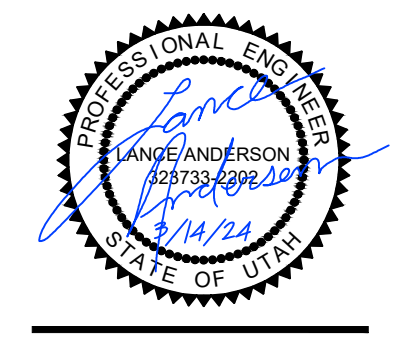
**GRADING PLAN-NORTHEAST**  
 SCALE: 1"=20'



**BID PACKAGE #1**

MARK	DATE	DESCRIPTION

PROJECT #: 123005  
 DRAWN BY: J. JENSEN  
 CHECKED BY: L. ANDERSON  
 ISSUED: 03.14.2024



CIVIL - GRADING  
 PLAN NORTHEAST

C-404









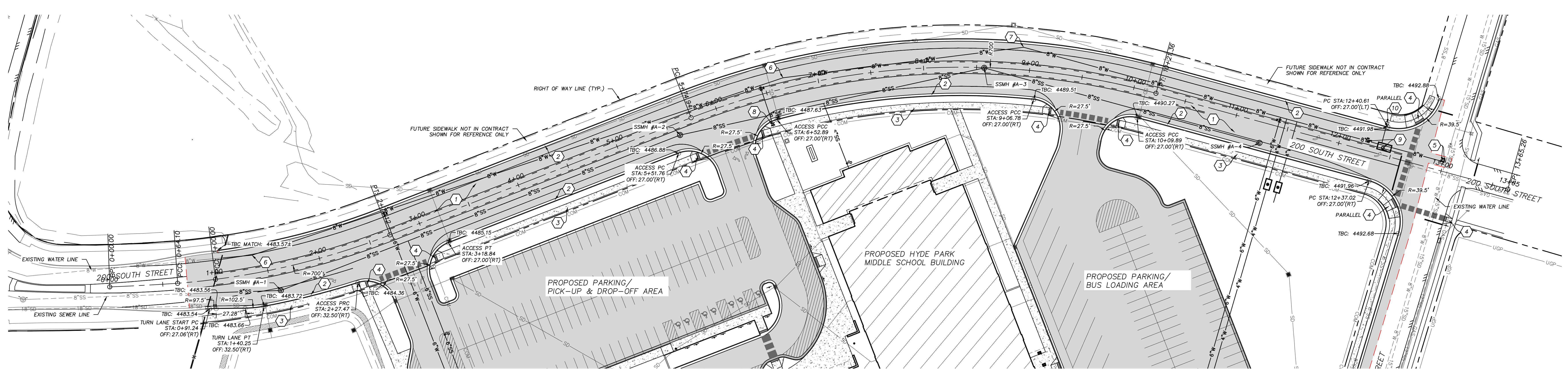












**GENERAL NOTES**

1. ALL UTILITY LINES TO BE INSTALLED PER HYDE PARK CITY STANDARDS.
2. PLACE THURST BLOCKS PER HYDE PARK CITY STANDARDS.
3. WATER MAIN TO HAVE A MINIMUM COVER OF 5 FEET. FIRE HYDRANTS TO BE 6.5-FT BURY MINIMUM.
4. PROVIDE TRACER WIRE ON ALL MAINS AND SERVICES PER HYDE PARK CITY STANDARDS.
5. ALL EXISTING HARDSCAPE FEATURES, LANDSCAPING AND IRRIGATION SYSTEMS THAT ARE DAMAGED OR DISTURBED DURING CONSTRUCTION ARE TO BE REPAIRED OR REPLACED AT CONTRACTOR'S EXPENSE.
6. ALL IMPROVEMENTS TO COMPLY WITH ADA STANDARDS.
7. ALL PAVEMENT MARKINGS TO CONFORM WITH THE LATEST EDITION OF THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD).

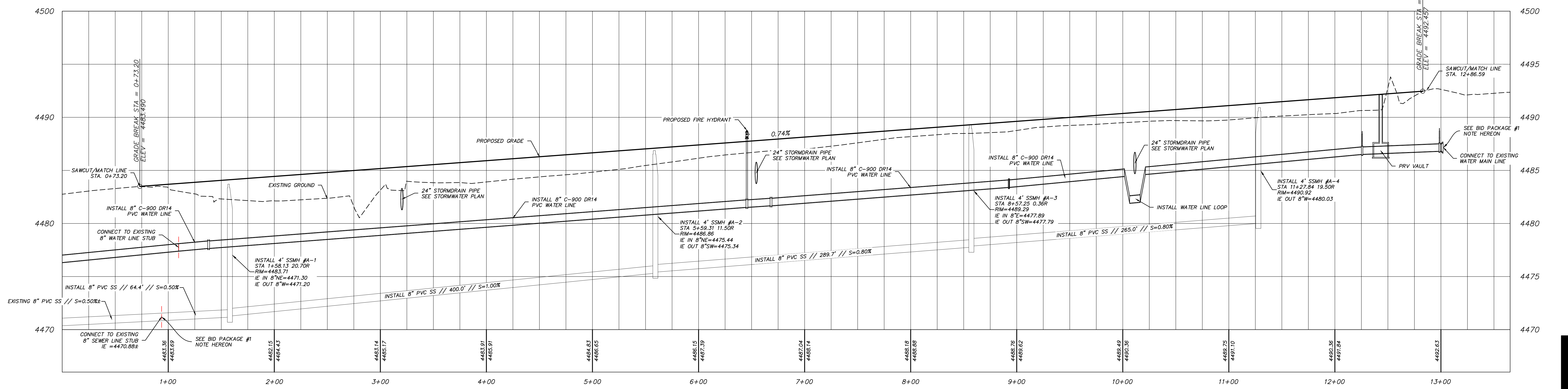
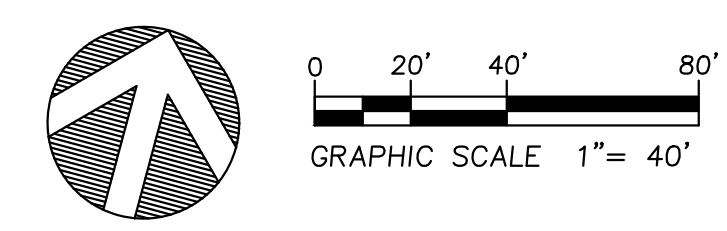
CONTRACTOR TO PROVIDE SEPARATE COST ESTIMATE FOR CONSTRUCTION OF 200 SOUTH STREET NORTH OF CENTER LINE AND SEPARATE COST ESTIMATE FOR PRV INSTALLATION.

**CONSTRUCTION NOTES**

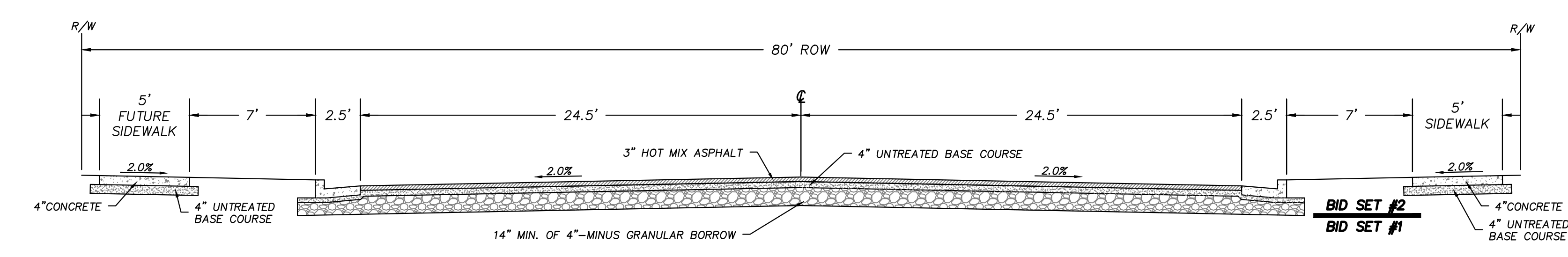
1. CONSTRUCT ASPHALT SECTION. SEE ROADWAY SECTION HEREON.
2. CONSTRUCT 30" ROADWAY CURB & GUTTER. SEE SHEET C-501/DETAIL 4.
3. CONSTRUCT CONCRETE SIDEWALK SECTION. SEE ROADWAY SECTION HEREON.
4. CONSTRUCT ADA RAMP. SEE SHEET C-501.
5. INSTALL 8"X8" HOT TAPPING TEE. INSTALL 8" GATE VALVE ON WATER LINE.
6. INSTALL 11.25" ELBOW.
7. INSTALL 22.5" ELBOW.
8. INSTALL FIRE HYDRANT. SEE UTILITY PLAN.
9. INSTALL PRESSURE REDUCING VALVE AND VAULT. SEE SHEET C-503/DETAIL 4. INSTALL 8" GATE VALVE WITHIN 20' OF WEST SIDE OF VAULT.
10. INSTALL VENT FOR PRV VAULT. SEE SHEET C-503/DETAIL 4.

**BID PACKAGE #1 NOTE:**  
 PRIOR TO CONSTRUCTION, CONTRACTOR TO FIELD VERIFY ALL EXISTING UTILITY LINE LOCATIONS INCLUDING, BUT NOT LIMITED TO, VERIFYING EXISTING WATER LINE AND SEWER LINE STUB ELEVATIONS AT EXISTING END OF 200 SOUTH STREET AND AT INTERSECTION OF 200 WEST AND 200 SOUTH. NOTIFY ENGINEER OF EXISTING SEWER LINE ELEVATIONS.

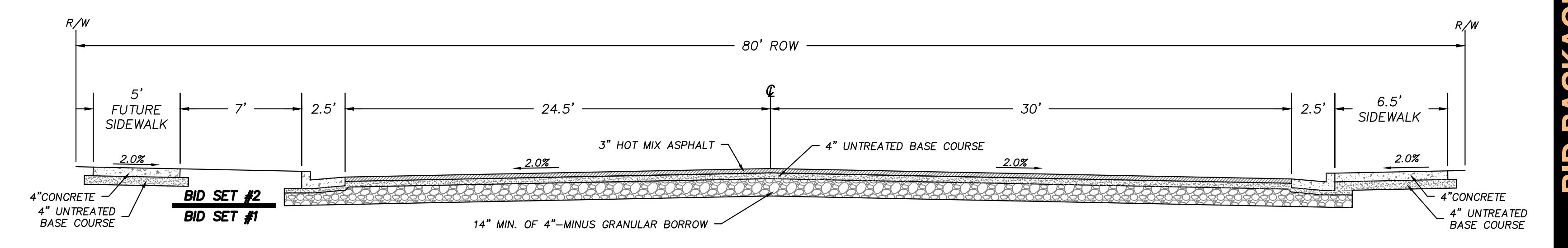
**200 SOUTH STREET PLAN VIEW**  
 SCALE: 1"=40'



**ROADWAY PROFILE VIEW**  
 SCALE: H: 1"=40' V: 1"=4'



**80' ROW ROADWAY SECTION-TYPICAL**  
 SCALE: NTS

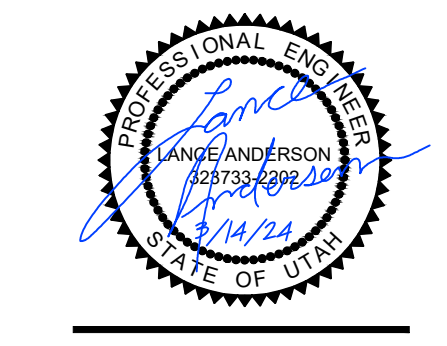


**ROADWAY SECTION STA. 1+40.25 TO 2+27.47**  
 SCALE: NTS

**HYDE PARK MIDDLE SCHOOL**  
 250 W 200 S HYDE PARK, UT  
 CACHE COUNTY SCHOOL DISTRICT

MARK	DATE	DESCRIPTION

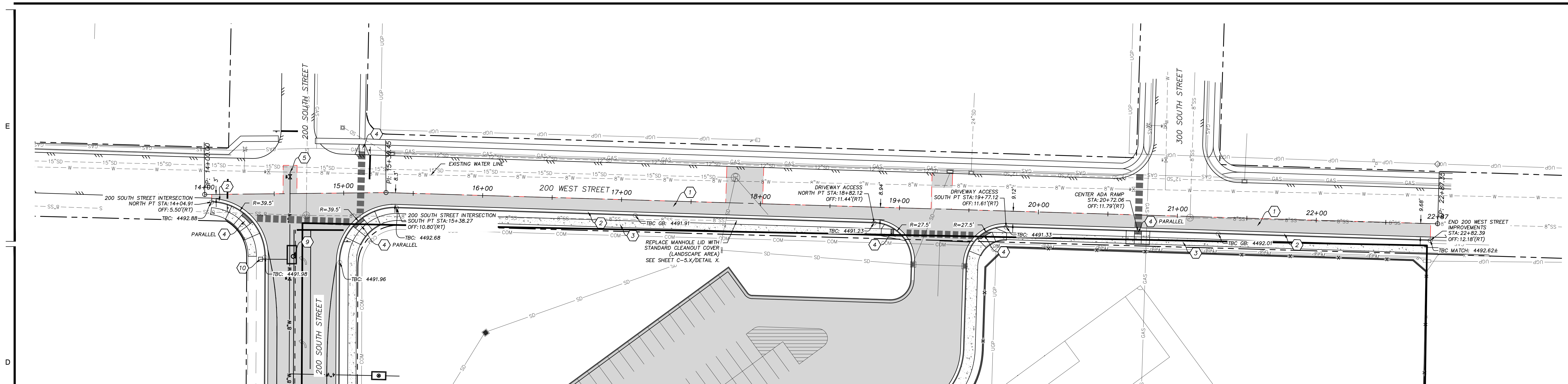
PROJECT #: 123005  
 DRAWN BY: J. JENSEN  
 CHECKED BY: L. ANDERSON  
 ISSUED: 03.14.2024



CIVIL- ROADWAY  
 PLAN & PROFILE  
 200 SOUTH

**BID PACKAGE #1**





**GENERAL NOTES**

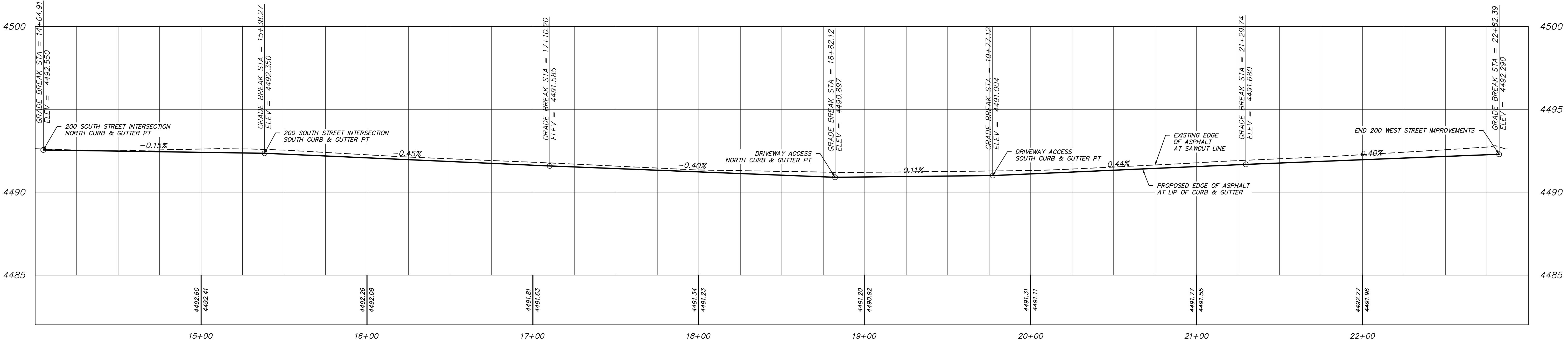
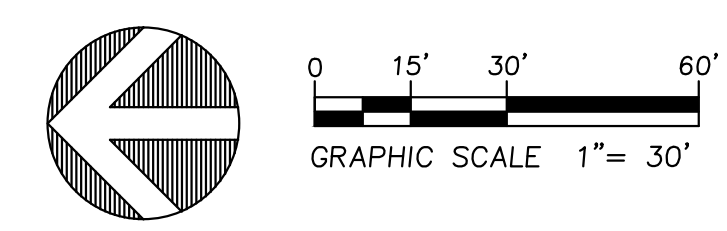
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2. PLACE THRUST BLOCKS PER HYDE PARK CITY STANDARDS.
3. WATER MAIN TO HAVE A MINIMUM COVER OF 5 FEET. FIRE HYDRANTS TO BE 6.5-FT BURY MINIMUM.
4. PROVIDE TRACER WIRE ON ALL MAINS AND SERVICES PER HYDE PARK CITY STANDARDS.
5. ALL EXISTING HARDSCAPE FEATURES, LANDSCAPING AND IRRIGATION SYSTEMS THAT ARE DAMAGED OR DISTURBED DURING CONSTRUCTION ARE TO BE REPAIRED OR REPLACED AT CONTRACTORS EXPENSE.
6. ALL IMPROVEMENTS TO COMPLY WITH ADA STANDARDS.
7. ALL PAYMENT MARKINGS TO CONFORM WITH THE LATEST EDITION OF THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD).

**CONSTRUCTION NOTES**

1. CONSTRUCT ASPHALT SECTION. SEE ROADWAY SECTION HEREON.
2. CONSTRUCT 30" ROADWAY CURB & GUTTER. SEE SHEET C-501/DETAIL 4.
3. CONSTRUCT CONCRETE SIDEWALK SECTION. SEE ROADWAY SECTION HEREON.
4. CONSTRUCT ADA RAMP. SEE SHEET C-501.
5. INSTALL 8"x8" HOT TAPPING TEE. INSTALL 8" GATE VALVE ON WATER LINE.
6. INSTALL 11.25" ELBOW.
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8. INSTALL FIRE HYDRANT. SEE UTILITY PLAN.
9. INSTALL PRESSURE REDUCING VALVE AND VAULT. SEE SHEET C-503/DETAIL 4. INSTALL 8" GATE VALVE WITHIN 20' OF WEST SIDE OF VAULT.
10. INSTALL VENT FOR PRV VAULT. SEE SHEET C-503/DETAIL 4.

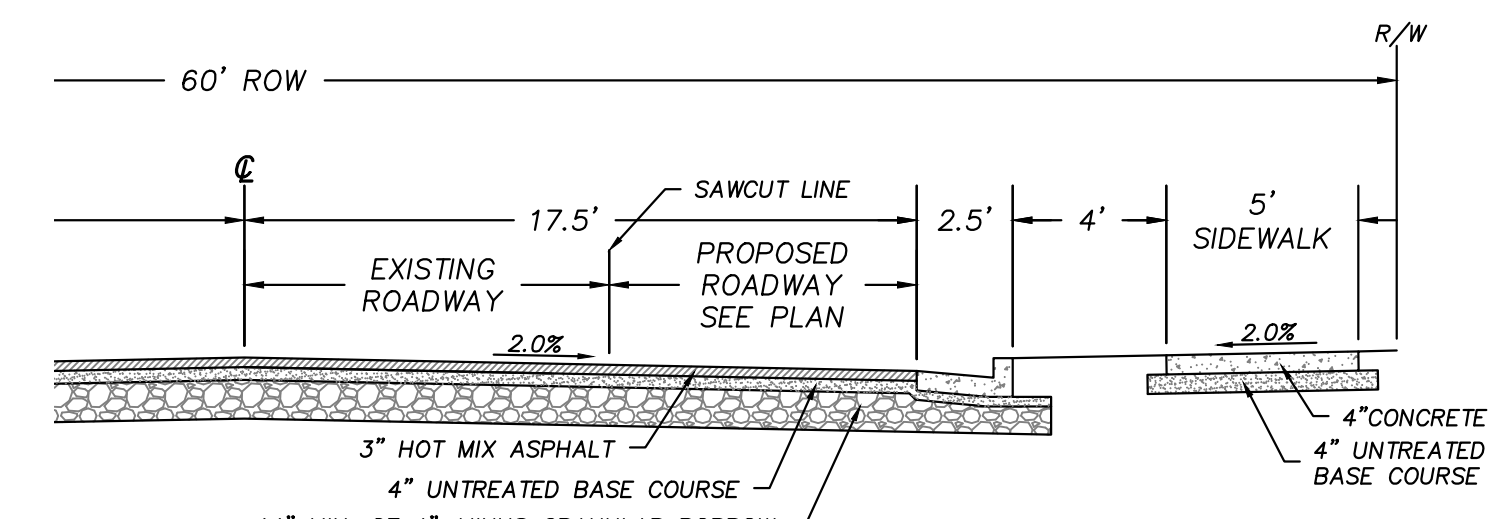
**200 WEST STREET PLAN VIEW**

SCALE: 1"=30'



**ROADWAY PROFILE VIEW**

SCALE: H: 1"=30' V: 1"=3'



**200 WEST ROADWAY SECTION**

SCALE: NTS

**FOR REFERENCE ONLY**

**BID PACKAGE #1**

**HYDE PARK MIDDLE SCHOOL**

250 W 200 S HYDE PARK, UT  
 CACHE COUNTY SCHOOL DISTRICT

MARK	DATE	DESCRIPTION

PROJECT #: 123005  
 DRAWN BY: J. JENSEN  
 CHECKED BY: L. ANDERSON  
 ISSUED: 03.14.2024



CIVIL- ROADWAY  
 PLAN & PROFILE  
 200 WEST

C-602



STRUCTURAL NOTES:

A. GENERAL

- 1. THE STRUCTURAL NOTES ARE INTENDED TO COMPLEMENT THE PROJECT SPECIFICATIONS WHICH ARE PART OF THE CONSTRUCTION DOCUMENTS. SPECIFIC NOTES AND DETAILS ON THE DRAWINGS SHALL GOVERN OVER THE STRUCTURAL NOTES AND TYPICAL DETAILS.
2. THESE DRAWINGS (AND, WHERE APPLICABLE, ACCOMPANYING WRITTEN SPECIFICATIONS) ARE THE ONLY CONTRACT DOCUMENTS PROVIDED BY ARW ENGINEERS FOR THE PROJECT REPRESENTED HEREIN. NOTHING IN ANY DIGITAL MODEL OR DIGITAL FILE RELATED TO THIS PROJECT SHALL BE TAKEN TO SUPERSEDE ANY INFORMATION SHOWN IN THESE DRAWINGS (INCLUDING, BUT NOT LIMITED TO, DIMENSIONS, SIZES, ETC.).
3. THE ARCHITECTURAL DRAWINGS ARE THE PRIME CONTRACT DRAWINGS. THE STRUCTURAL DRAWINGS ARE SUPPLEMENTARY TO AND MUST BE USED IN CONJUNCTION WITH THE ARCHITECTURAL DRAWINGS AND OTHER CONSULTANTS' DRAWINGS. ALL OMISSIONS OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND STRUCTURAL ENGINEER BEFORE PROCEEDING WITH ANY WORK INVOLVED. IN CASE OF CONFLICT, FOLLOW THE MOST STRINGENT REQUIREMENT AS DIRECTED BY THE ARCHITECT AT NO ADDITIONAL COST TO THE OWNER.
4. SEE SPECIFICATIONS FOR REQUIRED SUBMITTALS. SUBMITTALS SHALL BE MADE IN A TIMELY MANNER AS INDICATED IN SPECIFICATIONS. REVIEW OF SUBMITTALS BY ARW ENGINEERS IS FOR GENERAL COMPLIANCE ONLY AND IS NOT INTENDED AS APPROVAL. CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL SIZES, DIMENSIONS, AND ELEVATIONS ON SUBMITTALS AS RELATED TO DESIGN DOCUMENTS. PREPARATION OF SHOP DRAWINGS FOR STRUCTURAL ELEMENTS WILL REQUIRE INFORMATION (I.E. DIMENSIONS, ETC.) FOUND IN THE ARCHITECTURAL, STRUCTURAL, AND OTHER CONSULTANTS' DRAWINGS.
5. THE CONTRACTOR SHALL VERIFY ALL CONDITIONS AND DIMENSIONS AT THE SITE. IF ACTUAL CONDITIONS DIFFER FROM THOSE SHOWN ON CONTRACT DOCUMENTS, CONTRACTOR SHALL NOTIFY ARCHITECT PRIOR TO FABRICATION OR CONSTRUCTION OF ANY AFFECTED ELEMENTS.
6. THE CONTRACTOR SHALL COORDINATE AND VERIFY ALL LOCATIONS AND SIZES OF MECHANICAL EQUIPMENT OR OTHER EQUIPMENT BEFORE FABRICATING AND ERECTING STRUCTURAL ELEMENTS. SIZES AND LOCATIONS THAT DIFFER FROM THOSE SHOWN ON THE CONTRACT DOCUMENTS SHALL BE REPORTED TO THE ARCHITECT.
7. THE CONTRACTOR SHALL SUBMIT A WRITTEN REQUEST TO THE ARCHITECT FOR ARCHITECT AND/OR ENGINEER APPROVAL BEFORE PROCEEDING WITH ANY CHANGES, MODIFICATIONS, OR SUBSTITUTIONS.
8. DURING AND AFTER CONSTRUCTION, BUILDER AND/OR OWNER SHALL KEEP LOADS ON STRUCTURE WITHIN THE LIMITS OF DESIGN LOADS AS NOTED IN THESE DOCUMENTS.
9. TYPICAL OR SIMILAR DETAILS AND SECTIONS SHALL APPLY WHERE SPECIFIC DETAILS ARE NOT SHOWN. TYPICAL OR SIMILAR DETAILS REFER TO THE CONDITION ADDRESSED AND ARE NOT NECESSARILY DETAILS LABELED 'TYPICAL' OR 'SIMILAR' IN THE PLANS AND SECTION REQUIREMENTS AS PRESCRIBED BY OSHA OR OTHER REGULATORY AGENCIES REGARDLESS OF INDICATIONS IN THESE DOCUMENTS.
10. NOTICE OF COPYRIGHT: THESE STRUCTURAL DRAWINGS ARE HEREBY COPYRIGHTED BY ARW ENGINEERS. ALL RIGHTS RESERVED. THESE DOCUMENTS DEFINE A STRUCTURE AND ARE INSTRUMENTS OF SERVICE. FOR ONE USE ONLY. REPRODUCTION AND DISTRIBUTION OF THESE DRAWINGS IS ONLY ALLOWED FOR REGULATORY AGENCIES AND FOR CONVEYANCE OF INFORMATION TO PARTIES INVOLVED IN THE CONSTRUCTION OF THIS PROJECT. THESE DOCUMENTS SHALL NOT BE REPRODUCED OR COPIED, IN PART OR WHOLE BY ANY PARTY FOR USE IN PREPARATION OF SHOP DRAWINGS OR OTHER SUBMITTALS.
11. WHERE THE WORD 'SHALL' OCCURS IN THESE DRAWINGS AND ANY ACCOMPANYING SPECIFICATIONS, IT IS CONSIDERED A MANDATORY OBLIGATION AND SYNONYMOUS WITH THE PHRASE 'HAS DUTY TO'.

B. STATEMENT OF SPECIAL INSPECTIONS AND SPECIAL INSPECTIONS

- 1. THE DESIGNATED SEISMIC/WIND SYSTEMS AND SEISMIC/WIND-FORCE-RESISTING SYSTEMS THAT ARE SUBJECT TO SPECIAL INSPECTIONS IN ACCORDANCE WITH IBC SECTION 1705.12 AND 1705.13 ARE IDENTIFIED ON THESE DOCUMENTS WITH A CIRCLE 'L'. ALL OTHER ITEMS REQUIRING SPECIAL INSPECTION ARE IDENTIFIED IN THE SPECIAL INSPECTION SCHEDULE ON SHEETS S-012 AND S-013.
2. SPECIAL INSPECTIONS AND TESTING ARE TO BE PROVIDED AS REQUIRED BY IBC SECTIONS 1704 THROUGH 1705 AND OTHER APPLICABLE SECTIONS OF THE IBC. THE TYPE AND FREQUENCY OF TESTING AND SPECIAL INSPECTIONS SHALL BE AS NOTED IN THE SPECIAL INSPECTION SCHEDULE. JOB SPECIFICATIONS, AND ACCORDANCE WITH IBC SECTION 110 AND CHAPTER 17, CONTRACTOR SHALL COORDINATE AND COOPERATE WITH REQUIRED INSPECTIONS.
3. ALL TESTING AND SPECIAL INSPECTIONS SHALL BE PROVIDED BY A QUALIFIED INDEPENDENT SPECIAL INSPECTION AGENCY IN ACCORDANCE WITH IBC 1704 AND AS OUTLINED IN THE JOB SPECIFICATIONS. REPORTS OF FINDINGS OR DISCREPANCIES SHALL BE NOTED AND FORWARDED TO THE CONTRACTOR, ARCHITECT, ENGINEERS, AND BUILDING OFFICIAL IN A TIMELY MANNER.
4. STRUCTURAL OBSERVATION VISITS SHALL BE PERFORMED BY A REPRESENTATIVE FROM ARW ENGINEERS IN ACCORDANCE WITH THE CONTRACT AS NEEDED TO OBSERVE THE CONSTRUCTION OF CRITICAL BUILDING ELEMENTS (I.E. FOOTINGS, BRACED FRAMES, MOMENT FRAMES, DRAG STRUTS AND THEIR CONNECTIONS, COLLECTORS, AND ROOF AND FLOOR DIAPHRAGMS). STRUCTURAL OBSERVATION REPORTS FOR EACH VISIT SHALL BE SENT DIRECTLY TO THE ARCHITECT FOR DISTRIBUTION TO THE CONTRACTOR AND BUILDING OFFICIAL. STRUCTURAL OBSERVATION VISITS SHALL NEITHER BE CONSTRUED AS SPECIAL INSPECTION NOR APPROVAL OF COMPLETED CONSTRUCTION.
5. IN ACCORDANCE WITH IBC 1704.4, THE CONTRACTOR SHALL SUBMIT A WRITTEN CONTRACTOR'S STATEMENT OF RESPONSIBILITY TO THE BUILDING OFFICIAL AND OWNER. THE STATEMENT SHALL BE SUBMITTED PRIOR TO THE CONSTRUCTION OF ANY SEISMIC/WIND-FORCE-RESISTING SYSTEM, DESIGNATED SEISMIC/WIND SYSTEM, OR COMPONENT IDENTIFIED IN THESE DOCUMENTS WITH A CIRCLE 'L'.

C. BASIS OF DESIGN

- 1. GOVERNING BUILDING CODE: INTERNATIONAL BUILDING CODE (IBC) 2021
2. RISK CATEGORY: II
3. SUSPENDED FLOOR LOADS
a. LIVE LOAD = 40 PSF AT CLASSROOMS, 80 PSF AT CORRIDORS (UNREDUCED)
b. DEAD LOAD = 70 PSF
3. ROOF LOADS
a. FLAT-ROOF SNOW LOAD, P<sub>s</sub>: 33 PSF
1. GROUND SNOW LOAD, P<sub>g</sub>: 45 PSF
b. SNOW EXPOSURE FACTOR, C<sub>e</sub>: 1.0
c. SNOW LOAD IMPORTANCE FACTOR, I<sub>s</sub>: 1.1
d. THERMAL FACTOR, C<sub>t</sub>: 1.0
e. SLOPE FACTOR, C<sub>s</sub>: 1.0
f. SNOW DRIFT: SHOWN ON PLANS WHERE APPLICABLE.
g. LIVE LOAD = 20 PSF
c. DEAD LOAD = 20 PSF
4. WIND DESIGN
a. BASIC WIND SPEED (3 SECOND GUST): 110 MPH
b. WIND EXPOSURE: C
c. INTERNAL PRESSURE COEFFICIENT, GC<sub>i</sub>: 0.18
d. COMPONENT AND CLADDING DESIGN WIND PRESSURE SHALL BE AS REQUIRED PER ASCE 7-16.
5. SEISMIC DESIGN
a. SEISMIC IMPORTANCE FACTOR, I<sub>e</sub>: 1.25
b. SITE CLASS: E
c. MAPPED SPECTRAL RESPONSE ACCELERATIONS: S<sub>s</sub> = 1.079, S<sub>1</sub> = 0.363
d. SPECTRAL RESPONSE COEFFICIENTS: S<sub>DS</sub> = 0.863, S<sub>1D</sub> = 0.617
e. SEISMIC DESIGN CATEGORY: D
f. BASIC SEISMIC-FORCE-RESISTING SYSTEM: SPECIAL REINFORCED CONCRETE/MASONRY SHEARWALLS
g. SEISMIC RESPONSE COEFFICIENT, C<sub>s</sub>: 216
h. RESPONSE MODIFICATION FACTOR, R: 5
i. ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE

D. FOUNDATION

- 1. GENERAL
a. DESIGN SOIL PRESSURE: 3000 PSF
b. SOILS REPORTS BY: A CACHE CORP
REPORT # 1220008 (HYDE PARK SITE) DATED NOVEMBER 12, 2022
REPORT # 1230024 (NIBLEY SITE) DATED DECEMBER 11, 2023
c. SOIL PREPARATION UNDER FOUNDATIONS AND SLABS-ON-GRADE SHALL BE IN ACCORDANCE WITH THE SOILS REPORT
d. TOP OF FOOTING ELEVATIONS SHOWN ON THE FOOTING AND FOUNDATION PLAN ARE BASED ON PRELIMINARY GRADING INFORMATION AND SHALL BE VERIFIED PRIOR TO CONSTRUCTION. STEPS WHERE SHOWN ARE AT APPROXIMATE LOCATIONS. ACTUAL STEP LOCATIONS SHALL BE AT THE CONTRACTOR'S DISCRETION BASED UPON FIELD CONDITIONS. ALL EXTERIOR FOUNDATIONS SHALL BEAR A MINIMUM OF 30" INCHES BELOW LOWEST ADJACENT FINAL GRADE.
e. ALL WALLS (EXCEPT CANTILEVER RETAINING WALLS) SHALL BE FULLY BRACED AGAINST LATERAL MOVEMENT PRIOR TO BACKFILLING. DESIGN AND ERECTION OF BRACING/SHORING SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR. BRACING SHALL REMAIN IN PLACE UNTIL SUPPORTING SITS ARE IN PLACE AND HAVE OBTAINED FULL STRENGTH.
f. UNLESS NOTED OTHERWISE, ALL FOOTINGS AT COLUMNS SHALL BE CENTERED BELOW COLUMNS. UNLESS NOTED OTHERWISE, ALL FOOTINGS SHALL HAVE VERTICAL FACES FORMED WITH STANDARD FORMING MATERIALS (WOOD, METAL, ETC.) WITH PRIOR APPROVAL OF ARCHITECT AND ENGINEER. CONCRETE FOR FOOTINGS CAN BE PLACED IN EXCAVATED SOIL "FORMS" PROVIDED THAT THE DIMENSIONS ARE INCREASED 3" ON ALL SIDES.

E. CONCRETE

- 1. ALL CONCRETE MIX DESIGNS SHALL COMPLY WITH THE PROJECT SPECIFICATIONS AND THE REQUIREMENTS LISTED BELOW
a. INTERIOR GRADE BEAMS, FOUNDATION WALLS:
1. WHERE THE TOP OF THE ELEMENT IS EXPOSED OR IS LOCATED WITHIN 30" OF THE LOWEST ADJACENT GRADE (EXPOSURE CATEGORY F2):
a. 28 DAY COMPRESSIVE STRENGTH: 4000 PSI
b. MAXIMUM W/C RATIO: 0.45
c. MAXIMUM AGGREGATE SIZE: 1"
2. WHERE THE TOP OF THE ELEMENT IS NOT EXPOSED OR IS NOT LOCATED WITHIN 30" OF THE LOWEST ADJACENT GRADE (EXPOSURE CATEGORY F0):
a. 28 DAY COMPRESSIVE STRENGTH: 3000 PSI
b. INTERIOR SLABS ON GRADE (EXPOSURE CATEGORY F0):
1. 28 DAY COMPRESSIVE STRENGTH: 3000 PSI
c. INTERIOR SUSPENDED SLABS (EXPOSURE CATEGORY F0):
1. 28 DAY COMPRESSIVE STRENGTH: 3000 PSI
d. ICF WALLS
1. 28 DAY COMPRESSIVE STRENGTH: 4500 PSI
e. TOTAL AIR CONTENT FOR CONCRETE EXPOSED TO CYCLES OF FREEZING AND THAWING SHALL BE DETERMINED IN ACCORDANCE WITH THIS SCHEDULE. TOLERANCE ON AIR CONTENT AS DELIVERED SHALL BE +/- 1.5 PERCENT
NOMINAL MAXIMUM TARGET AIR CONTENT, PERCENT
AGGREGATE SIZE, IN F1 F2 AND F3
3/8 6 7.5
1/2 5.5 7
3/4 5 8
1 -1/2 4.5 6
2 4 5.5
2 4 5
2. WATER USED IN MIXING CONCRETE SHALL CONFORM TO ASTM C1602.
3. NO CONDUIT, PIPES, DUCTS, SLEEVES, ETC. SHALL BE PLACED IN STRUCTURAL CONCRETE UNLESS SPECIFICALLY DETAILED AND APPROVED BY THE STRUCTURAL ENGINEER. NO ALUMINUM PRODUCTS SHALL BE EMBEDDED IN CONCRETE. PENETRATIONS THRU STRUCTURAL CONCRETE ELEMENTS MUST BE APPROVED BY THE ENGINEER AND SHALL BE BUILT INTO THE ELEMENT PRIOR TO CONCRETE PLACEMENT.
4. REFER TO ARCHITECTURAL DRAWINGS FOR MOLDS, GROOVES, ORNAMENTS, ETC. TO BE CAST IN TO CONCRETE, AND FOR EXTENT AND LOCATION OF DEPRESSIONS, CURBS, RAMPS, ETC.
5. UNLESS NOTED OTHERWISE, MINIMUM REINFORCING IN ALL CONCRETE FOUNDATION WALLS SHALL BE AS FOLLOWS:

Table with 4 columns: THICKNESS, TOP & BOTTOM BARS, VERTICAL, HORIZONTAL. Rows include 8" and 14" wall thicknesses with corresponding bar counts and spacing requirements.

F. ANCHOR BOLTS/EMBEDDED BOLTS

- 1. ALL ANCHOR BOLTS SHALL HAVE ASTM A-363 HEAVY HEX NUT AND ASTM F-436 WASHERS AT STANDARD OR OVERSIZED HOLES PER AISC SPECIFICATION TABLE J3.3, WHERE HOLE SIZES DO NOT COMPLY WITH THE LIMITATIONS FOR OVERSIZED HOLES THE STRUCTURAL ENGINEER SHALL BE NOTIFIED TO DETERMINE STEEL PLATE WASHER REQUIREMENTS. ANCHOR BOLTS SHALL COMPLY WITH THE FOLLOWING:
a. AT ALL ANCHOR BOLTS (UNLESS NOTED OTHERWISE) - ASTM F1554 GRADE 36 HEADED BOLTS. (ASTM A36 THREADED ROD MAY BE USED WITH DOUBLE NUT AND WASHER.)
2. EMBEDDED BOLTS IN MASONRY SHALL BE (UNLESS NOTED OTHERWISE) ASTM A-307 GRADE HEADED BOLTS.
3. SEE TYPICAL ANCHOR BOLT DETAIL FOR DEFINITIONS OF EMBEDMENT LENGTH, ETC.
4. BRUSH TEMPLATES AND OTHER DEVICES AS NECESSARY FOR PRESETTING ALL BOLTS PRIOR TO PLACING CONCRETE AND/OR GROUT.
5. IF THREADED RODS ARE USED AS PERMITTED ABOVE, THEY SHALL BE CLEAR OF SOIL AND DIRT. WHERE REQUIRED FOR ERECTION, HOLES LARGER THAN OVERSIZED MAY BE PERMITTED WITH THE USE OF STEEL PLATE WASHERS AT THE DISCRETION OF THE STRUCTURAL ENGINEER.

G. ADHESIVE/MECHANICAL ANCHORS

- 1. WITHOUT WRITTEN APPROVAL OF THE ENGINEER, CONTRACTOR SHALL NOT SUBSTITUTE POST-INSTALLED ANCHORS WHERE CAST-IN-PLACE ANCHORS ARE SPECIFIED IN THE DRAWINGS.
2. WHERE STRUCTURAL DETAILS SPECIFY SPECIFIC BRANDS AND/OR TYPES OF ADHESIVES OR ANCHORS, SUBSTITUTIONS OF OTHER BRANDS AND/OR TYPES IS NOT ALLOWED, WITHOUT WRITTEN APPROVAL OF THE ENGINEER.
3. SUBSTITUTION REQUESTS FOR ALTERNATE PRODUCTS SHALL BE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER OF RECORD PRIOR TO USE. SUBSTITUTION REQUESTS SHALL INCLUDE AN ICC ESR OR IAPMO REPORT AND SUPPORTING CALCULATIONS INDICATING COMPLIANCE WITH DESIGN INTENT.
4. ALL ADHESIVE/MECHANICAL ANCHORS SHALL BE INSTALLED, INCLUDING HOLE DRILLING AND PREPARATION, IN ACCORDANCE WITH AN APPROVED INDEPENDENT EVALUATION REPORT (ICC-ES, IAPMO, OR APPROVED EQUAL), AS INDICATED BELOW, AND IN ACCORDANCE WITH ALL MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII).
5. INSTALLERS SHALL BE, AT A MINIMUM, TRAINED FOR THE SPECIFIC APPLICATION INSTALLATION TECHNIQUE FOR THE SPECIFIC PRODUCT BY THE PRODUCT MANUFACTURERS FIELD EMPLOYEE OR SHALL POSSESS A TRAINING CARD OBTAINED BY THE MANUFACTURERS ONLINE TRAINING PROGRAM.
6. ADHESIVE ANCHORS SHALL BE INSTALLED IN CONCRETE HAVING A MINIMUM AGE OF 21 DAYS AT TIME OF ANCHOR INSTALLATION. ADHESIVE ANCHORS SHALL NOT BE FULLY LOADED UNTIL CONCRETE HAS REACHED DESIGN STRENGTH.
7. ADHESIVE ANCHORS SHALL CONSIST OF REINFORCING BAR OR THREADED RODS AS INDICATED IN THESE DOCUMENTS.
8. UNLESS APPROVED BY THE ENGINEER OF RECORD, CONCRETE AND DRILLED ANCHOR HOLES SHALL BE CLEAN AND FREE OF WATER FOR 14 DAYS PRIOR TO ADHESIVE INSTALLATION. CONTACT THE ENGINEER OF RECORD FOR GUIDANCE IF THE CONTRACTOR CHOOSES TO INSTALL IN DAMP, WATER-SATURATED, OR WATER-FILLED HOLES.
9. CONCRETE TEMPERATURE AT THE TIME OF INSTALLATION SHALL BE MONITORED BY THE CONTRACTOR. CONTRACTOR SHALL COMPLY WITH ALL MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII) RELATIVE TO SUBSTRATE TEMPERATURE.
10. INSTALLATION OF ADHESIVE ANCHORS HORIZONTALLY OR UPWARDLY INCLINED TO SUPPORT SUSTAINED TENSION LOADS SHALL BE PERFORMED BY PERSONNEL CERTIFIED BY AN APPLICABLE CERTIFICATION PROGRAM. CERTIFICATION SHALL INCLUDE WRITTEN AND PERFORMANCE TESTS IN ACCORDANCE WITH THE ANCHORS' ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM, OR EQUIVALENT IN ACCORDANCE WITH ACI 318-19 26.7.2 (a) PROOF OF CURRENT CERTIFICATION SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO INSTALLATION. CONTINUOUS SPECIAL INSPECTION SHALL BE PROVIDED FOR THESE ANCHORS.
11. UNLESS NOTED OTHERWISE, ALL ADHESIVE ANCHORS INTO CONCRETE SHALL BE:
a. HILTI HIT-RE 500V3 (ESR-361), OR HILTI HIT-HY 200-V3 (ESR-466),
b. SIMPSON SET-36 (ESR-4057), OR AT-XP (ER-265).
c. DEWALT PURE 110+ (ESR-3298), OR AC208+ GOLD (ESR-4027-COLD WEATHER).
12. UNLESS NOTED OTHERWISE, ALL ADHESIVE ANCHORS INTO GROUTED MASONRY (CMU) SHALL BE:
a. HILTI HIT-HY 270 (ESR-4145),
b. SIMPSON SET-XP (ER-265), OR AT-XP (ER-261),
c. DEWALT AC108+ GOLD (ESR-3293).
13. UNLESS NOTED OTHERWISE, ALL MECHANICAL ANCHORS INTO CONCRETE SHALL BE:
a. HILTI KWIK BOLT-T22 (ESR-4266),
b. SIMPSON STRONG-BOLT 2 (ESR-3037).
14. UNLESS NOTED OTHERWISE, ALL MECHANICAL ANCHORS INTO GROUTED MASONRY (CMU) SHALL BE:
a. HILTI KWIK BOLT-T22 (ESR-4561),
b. SIMPSON STRONG BOLT 2 (ER-240),
c. DEWALT SCREWBOLT+ (ESR-4042).
15. UNLESS NOTED OTHERWISE, ALL SCREW ANCHORS INTO CONCRETE SHALL BE:
a. SIMPSON TITEN HD (ESR-2713),
b. DEWALT SCREWBOLT+ (ESR-3889),
c. HILTI KH-EZ (ESR-3027).
16. UNLESS NOTED OTHERWISE, ALL SCREW ANCHORS INTO GROUTED MASONRY (CMU) SHALL BE:
a. SIMPSON TITEN HD (ESR-1056),
b. DEWALT SCREWBOLT+ (ESR-1678),
c. HILTI KH-EZ (ESR-3058).
17. ALL MASONRY CELLS WITHIN 8" OF THE ANCHOR SHALL BE SOLID GROUTED.
18. THE TESTING LABORATORY WILL PERFORM VISUAL INSPECTION OF ANCHORS AND DOWELS AS SPECIFIED IN THE SPECIAL INSPECTION SCHEDULE AND THE APPROVED INDEPENDENT EVALUATION REPORT. TENSION TESTING CAN BE REQUIRED AT THE DIRECTION OF THE STRUCTURAL ENGINEER OF RECORD OR THE SPECIAL INSPECTOR.
19. IF REINFORCEMENT IS ENCOUNTERED DURING DRILLING, ABANDON THAT HOLE AND SHIFT THE ANCHOR LOCATION TO AVOID THE REINFORCEMENT. PROVIDE A MINIMUM SPACE OF (2) ANCHOR HOLE DIAMETERS OR 2 INCHES, WHICH EVER IS LARGER, OF SOUND CONCRETE/MASONRY BETWEEN THE ANCHOR AND THE ABANDONED HOLE. FILL THE ABANDONED HOLE WITH NON-SHRINK GROUT OR AN APPROVED ANCHORING ADHESIVE. AT CONTRACTORS OPTION, LOCATE EXISTING REINFORCEMENT PRIOR TO DRILLING/CORING. IF THE ANCHOR OR DOWEL CANNOT BE SHIFTED AS NOTED ABOVE, THE ENGINEER WILL DETERMINE A NEW LOCATION.
20. LOCATE REINFORCEMENT AND CONFIRM FINAL ANCHOR LOCATIONS PRIOR TO FABRICATING PLATES, MEMBERS, OR OTHER STEEL ASSEMBLIES ATTACHED WITH MECHANICAL ANCHORS.

H. SUSPENDED CONCRETE SLABS / SLABS ON METAL DECK

- 1. UNLESS NOTED OTHERWISE, ALL CONCRETE SLABS ON METAL DECK SHALL BE 5 1/2" TOTAL THICKNESS NORMAL WEIGHT CONCRETE WITH A WEIGHT LESS THAN 145 POUNDS PER CUBIC FOOT REINFORCED WITH 6 X 6 - W14 X W14 WELDED WIRE FABRIC. REINFORCING STEEL SHALL BE CHAINED TO 1" TOP COVER AT ALL BEAM LOCATIONS, EXCEPT WHERE SPECIFICALLY DETAILED. FIBER MESH MAY BE USED IN PLACE OF REINFORCEMENT IN SLABS ON DECK WHEN USED IN ACCORDANCE WITH AN APPROVED ICC RESEARCH REPORT AND WHERE APPROVED BY THE ENGINEER. WHERE THE SLAB CONSTRUCTION IS USED TO OBTAIN A UL FIRE RATING, THE PROPOSED FIBER MESH SHALL HAVE UL ACCEPTANCE AS AN APPROVED ALTERNATIVE TO WELDED WIRE FABRIC.
2. AROUND OPENINGS IN SUSPENDED CONCRETE SLABS, ADD REINFORCING BARS EQUIVALENT TO BARS CUT BY OPENING WITH HALF ON EACH SIDE OF OPENING. BARS PARALLEL TO PRINCIPAL REINFORCING SHALL RUN FULL LENGTH OF SPAN. BARS PARALLEL TO TEMPERATURE REINFORCING SHALL RUN 24" BEYOND OPENING.
3. SLAB PENETRATIONS LESS THAN 6" IN ALL DIRECTIONS WITH A CLEAR SPACING OF AT LEAST 3 TIMES THE LONGEST DIMENSION, DO NOT REQUIRE SUPPLEMENTAL REINFORCING. OTHERWISE, THE PENETRATIONS SHALL BE FRAMED ON 4 SIDES WITH STEEL ANGLES OR BENT PLATES (SEE TYPICAL DETAIL) UNLESS NOTED OTHERWISE.
4. EVERY EFFORT SHALL BE MADE TO PROVIDE A LEVEL FINISHED FLOOR WHILE MAINTAINING THE MINIMUM INDICATED SLAB THICKNESS.
5. CONTROL JOINTS IN SUSPENDED CONCRETE SLABS AND CONCRETE SLABS ON DECK SHALL NOT BE USED UNLESS SPECIFICALLY APPROVED AND DETAILED BY THE ENGINEER.
6. SEE TYPICAL DETAILS WHEN SLABS ARE MADE COMPOSITE WITH STEEL BEAMS.
7. NO CONDUIT IS ALLOWED IN CONCRETE SLABS ON METAL DECK.
8. WHERE CONDUIT IS CLUSTERED TOGETHER TO RISE ABOVE SLAB OR PENETRATE SLAB, PENETRATION IN SLAB MUST BE SUPPORTED AS NOTED IN NOTE H-3 ABOVE.
9. CONTRACTOR SHALL PROVIDE ALL TEMPORARY SHORING, BRACING, AND GUYING AS REQUIRED DURING ERECTION AND PLACEMENT OF SUSPENDED CONCRETE SLABS ON METAL DECK.
10. IN ALL SLABS NOT COVERED BY CARPET INSTALL #4 REINFORCING STEEL @ 12" O.C. EACH WAY.

I. REINFORCING STEEL

- 1. REINFORCING BAR STRENGTH REQUIREMENTS:
a. ALL REINFORCING BARS SHALL CONFORM TO ASTM STANDARD A-615 GRADE 60 AND ALL WELDED WIRE FABRIC SHALL CONFORM TO ASTM STANDARD A-108 AND SHALL BE SUPPLIED IN FLAT SHEETS, ADEQUATELY TIE AND SUPPORT ALL REINFORCING STEEL AS SPECIFIED BY ACI 117, TO MAINTAIN EXACT REQUIRED POSITION.
2. HEADED SHEAR STUD ASSEMBLIES SHALL CONFORM TO ASTM A1044.
3. STEEL DISCONTINUOUS FIBER REINFORCEMENT SHALL BE DEFORMED AND CONFORM TO ASTM A820 AND SHALL HAVE A LENGTH TO DIAMETER RATIO NOT SMALLER THAN 50 AND NOT GREATER THAN 100.
4. HEADED DEFORMED BARS SHALL CONFORM TO ASTM A675. OBSTRUCTIONS OR INTERRUPTIONS OF THE BAR DEFORMATIONS, IF ANY, SHALL NOT EXTEND MORE THAN 2 BAR DIAMETERS FROM THE BEARING FACE OF THE HEAD.
5. ALL REINFORCING STEEL SHALL BE TIED IN PLACE AND ADEQUATELY SUPPORTED PRIOR TO PLACING CONCRETE. WELD STABBING OF ANY REINFORCING STEEL IS NOT PERMITTED, UNLESS SPECIFICALLY DETAILED OTHERWISE OR APPROVED BY THE ENGINEER.
6. ALL FIELD BENT DOWELS SHALL BE GRADE 40 WITH SPACING INDICATED REDUCED BY 1/3.
7. UNLESS NOTED OTHERWISE, REINFORCEMENT SHALL HAVE THE FOLLOWING CONCRETE COVERAGE:
a. CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH: ... 3"
b. EXPOSED TO EARTH OR WEATHER:
1. #6 & LARGER: ... 2"
2. #5 & SMALLER: ... 1-1/2"
c. NOT EXPOSED TO WEATHER OR EARTH:
1. SLABS, WALLS, JOISTS, #11 & SMALLER: ... 3/4"
2. BEAMS, COLUMNS: MAIN REINFORCING OR TIES: ... 1-1/2"
d. SLAB ON GRADE:
1. PLACE REINFORCEMENT AT CENTER OF SLAB UNLESS INDICATED OTHERWISE.
8. EXCEPT WHERE NOTED ON PLANS OR DETAILS CONTINUOUS REINFORCEMENT SHALL BE SPLICED AT POINTS OF MINIMUM STRESS BY LAPPING PER THE REBAR LAP SCHEDULE.
9. REINFORCING STEEL MAY BE SPLICED WITH MECHANICAL COUPLERS THAT HAVE A TENSION CAPACITY OF AT LEAST 125% OF THE STRENGTH OF THE BAR. MECHANICAL COUPLERS SHALL BE A POSITIVE CONNECTING TYPE COUPLER, AND SHALL BE INSTALLED IN ACCORDANCE WITH AN APPROVED ICC RESEARCH REPORT. WHERE THESE ARE USED, SPLICES ON ADJACENT BARS SHALL BE STAGGERED AT LEAST 24 INCHES ALONG THE LENGTH OF THE BARS.
10. ALL VERTICAL REINFORCING IN STRUCTURAL ELEMENTS ABOVE SHALL BE SPLICED WITH MATCHING DOWELS EMBEDDED WITHIN THE FOOTINGS OR STRUCTURE BELOW. SPLICE LENGTHS SHALL COMPLY WITH REBAR LAP SCHEDULE. DOWELS INTO FOOTINGS SHALL TERMINATE WITH A STANDARD HOOK, AND SHALL EXTEND TO WITHIN 4" OF THE BOTTOM OF THE FOOTING, BUT NEED NOT EXCEED MORE THAN 20" INTO FOOTING. FOR MASONRY CONSTRUCTION SEE STRUCTURAL NOTE L.6.A.
11. DO NOT WELD REINFORCING EXCEPT AS NOTED ON PLANS, WHERE REINFORCING IS WELDED, USE ASTM A-706 REINFORCING.
12. REINFORCING BARS, TIES, AND TENDONS SHALL BE SUPPORTED BY NYLON CONES, PLASTIC-COATED TIE-WIRES, OR PLASTIC-COATED CHAIRS. REINFORCING IN FOOTINGS IS PERMITTED TO BE SUPPORTED ON CONCRETE DOBIES.
13. UNLESS NOTED OTHERWISE, HOOKS, STIRRUPS, TIES, AND OTHER BENDS IN REINFORCING STEEL SHALL MEET THE STANDARDS SET FORTH IN ACI 318-19-19. UNLESS OTHERWISE PERMITTED BY THE ENGINEER, ALL REINFORCEMENT SHALL BE BENT COLD. REINFORCEMENT PARTIALLY EMBEDDED IN CONCRETE SHALL NOT BE FIELD BENT, EXCEPT AS SHOWN ON THESE DRAWINGS OR OTHERWISE PERMITTED BY THE ENGINEER.
14. UNLESS SPECIFICALLY NOTED AND/OR DETAILED IN THE STRUCTURAL DRAWINGS CONDUIT SHALL NOT BE IN CONTACT WITH REINFORCING STEEL.

(STRUCTURAL NOTES CONTINUED ON SHEET S-002)

LEGEND OF SYMBOLS AND ABBREVIATIONS

Table mapping symbols to abbreviations and descriptions. Includes symbols for footing marks, section numbers, sheet numbers, foundation walls, shear walls, length of shear wall, footing step, masonry wall, deck bearing elevation, edge nailing, edge of deck, foundation, footing, finished floor elevation, near side, far side, headed stud anchor, joist bearing elevation, kicker brace, maximum, masonry beam, masonry column, mechanical, mezzanine, minimum, masonry jamb, masonry wall, masonry wall, near side, far side, approved equal, opposite, powder actuated fastener, plate, reinforcing, reinforced, similar, steel stud header, steel stud wall, steel stud wall, top of beam elevation, top of concrete slab, top of footing, top of girder elevation, top of masonry, top of steel elevation, typical, unless noted otherwise, framing angle, framing channel, items, details, & systems, braced frame, moment resisting connections, moment resisting cantilever connections, kicker brace.

Structural Sheet Index

Table with 2 columns: SHEET NUMBER and SHEET NAME. Lists sheets S-001 through S-401 and their corresponding titles, such as Structural Notes, Schedules, Typical Details, Footing & Foundation Details, Floor Framing Plans, Roof Framing Plans, Sports Storage Footing, etc.

Table with 2 columns: DATE and DESCRIPTION. Includes project start and end dates, and descriptions of project phases.

PROJECT #: 123005
DRAWN BY: BLP
CHECKED BY: J. Blazzard
ISSUED: 03.14.2024



J. STRUCTURAL STEEL

- 1. STRUCTURAL STEEL SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF THE FOLLOWING:
a. ANS/AISC 360-16 "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS" WITH "COMMENTARY" AND "SUPPLEMENTS" AS REQUIRED BY BUILDING CODE.
b. AISC 303-16 "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" EXCLUDING THE FOLLOWING SECTIONS: 4.4, 4.1, AND 4.2.
c. AISI "SPECIFICATIONS FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS".
d. AISI "SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS".
e. AISC D1.1 AND 1.3, "STRUCTURAL WELDING CODE" (EXCEPT SPECIFIC ITEMS DO NOT APPLY IF THEY CONFLICT WITH AISC).
f. ANS/AISC 341-16 "SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS".
2. STRUCTURAL STEEL SHALL COMPLY WITH THE FOLLOWING:
a. WIDE FLANGE SHAPES, CHANNELS, AND WT SHAPES - ASTM A992 (Fy = 50 ksi)
b. OTHER SHAPES, PLATES, ANGLES, AND BARS - ASTM A572 (Fy = 50 ksi) (UNO)
c. HOLLOW STRUCTURAL SECTIONS (HSS) - ASTM A500, GRADE C (Fy = 50 ksi)
d. STAINLESS STEEL SHAPES, PLATES, AND FASTENERS - ASTM 304
e. DEFORMED BAR ANCHORS (DBA) - ASTM A-496, WELDED IN ACCORDANCE WITH AWS D1.1
f. HEADED STUD ANCHORS (HSA) - ASTM A-108, GRADE 1015 STEEL AND WELDED IN ACCORDANCE WITH AWS D1.1 FOR TYPE "B". USE 3/4" DIAMETER STUDS, UNLESS NOTED OTHERWISE.
g. THREADED ROD - ASTM A-449
h. NON-SHRINK GROUT - ASTM C1107. NON-SHRINK GROUT SHALL BE PRE-PACKAGED, NON-METALLIC, WITH A 28-DAY COMPRESSIVE STRENGTH OF 6,000 PSI.
3. CONNECTIONS SHALL COMPLY WITH THE STRUCTURAL DRAWINGS UNLESS WRITTEN APPROVAL TO CHANGE IS GIVEN BY THE STRUCTURAL ENGINEER.
4. ALL SHOP FABRICATIONS SHALL BE PERFORMED BY AN APPROVED FABRICATOR IN ACCORDANCE WITH SECTIONS 1702 AND 1704 OF THE IBC OR WITH SHOP INSPECTION BY AN INDEPENDENT AGENCY IN ACCORDANCE WITH SECTION 1704.2.5 OF THE IBC.
5. WELDING
a. ALL WELDING AND CUTTING SHALL BE PERFORMED BY AWS QUALIFIED WELDERS IN ACCORDANCE WITH ANS/AWS D1.1 (LATEST EDITION).
b. USE E-70XX ELECTRODES UNLESS NOTED OTHERWISE. E-60XX MAY BE USED FOR WELDING STEEL DECKS.
c. ALL INTERSECTING STEEL SHAPES WHICH ARE NOT CONNECTED WITH BOLTS SHALL BE WELDED TOGETHER WITH A FILLET WELD ALL AROUND UNLESS NOTED OTHERWISE. WHERE WELD SIZES ARE NOT SHOWN, USE THE FOLLOWING:
1. WHERE THE THICKNESS OF THE CONNECTED PARTS IS EQUAL TO OR THICKER THAN 1/4", WELD SIZE SHALL BE 1/16" LESS THAN THE THICKNESS OF THE THINNEST PART.
2. WHERE ANY PARTS OF THE CONNECTED PARTS IS LESS THAN 1/4" THICK, WELD SIZE SHALL BE THE SAME AS THE THICKNESS OF THE THINNEST PART.
d. WELDING OF HSAs (HEADED STUD ANCHORS) AND DBAs (DEFORMED BAR ANCHORS) SHALL CONFORM TO THE MANUFACTURER'S SPECIFICATIONS AND DBA'S REINFORCING BARS SHALL NOT BE SUBSTITUTED FOR HSAs OR DBAs.
e. WHEREVER POSSIBLE, WELDS SHALL BE SHOP WELDS. SPECIAL CONSIDERATIONS, SUCH AS ITEMS WHICH MAY NEED ADJUSTMENT AT THE SITE, WELDS THAT REQUIRE FIELD WELDS WHERE QUESTIONS OR DISCREPANCIES OCCUR THE CONTRACTOR SHALL COORDINATE THE WORK BETWEEN THE SHOP FABRICATOR AND THE STEEL ERECTOR.
f. SPECIAL PROVISIONS FOR SEISMIC (SEISMIC RESISTING SYSTEM):
1. ALL WELDS DESIGNATED AS DEMAND CRITICAL WELDS SHALL BE MADE WITH FILLER METALS MEETING THE REQUIREMENTS SPECIFIED IN CLAUSES 6.1, 6.2, AND 6.3 OF AWS D1.8.
2. ALL OTHER WELDS THAT ARE PART OF THE SEISMIC RESISTING SYSTEM SHALL BE MADE WITH FILLER METALS MEETING THE REQUIREMENTS SPECIFIED IN CLAUSE 6.1 OF AWS D1.8.
3. BUTT WELDS IN MEMBERS WITH DIFFERENT THICKNESSES, SUCH AS COLUMN SPLICES, SHALL BE TAPERED AND MADE IN SUCH A MANNER THAT THE TRANSITION DOES NOT EXCEED 1 IN 2-1/2 INCHES. THE TRANSITION SHALL BE ACCOMPLISHED BY CHAMFERING THE THICKER PART, TAPERING THE WIDER PART, SLOPING THE WELD METAL OR BY A COMBINATION OF THESE.
6. BOLTING
a. UNLESS NOTED OTHERWISE, ALL STRUCTURAL STEEL TO STEEL CONNECTIONS SHALL USE HIGH STRENGTH BOLTS CONFORMING TO ASTM F3125 GR. A525.
b. UNLESS NOTED OTHERWISE, ALL BOLTING CLASSIFIED AS NON-SLIP CRITICAL BEARING TYPE CONNECTIONS WITH THREADS INCLUDED IN SHEAR PLANE, TIGHTEN BOLTS TO A SNUG TIGHT CONDITION, WITH ALL PILES OF THE JOINT IN FIRM CONTACT.
c. WHERE OVERSIZED OR SLOTTED HOLES OCCUR IN THE OUTER PLY, AN ASTM F436 WASHER OR 5/16" THICK COMMON PLATE WASHER SHALL BE USED AS REQUIRED TO COMPLETELY COVER THE HOLE.
d. BOLTS SHALL BE CENTERED IN SLOTTED HOLES, UNLESS NOTED OTHERWISE.
e. WHERE A STEEL BEAM TO BEAM CONNECTION IS NOT SHOWN, PROVIDE AN AISC STANDARD FRAMED CONNECTION SIZED FOR 1/2 OF THE TOTAL LOAD CAPACITY OF THE BEAM FOR THE SPAN AND STEEL SPECIFIED.
7. METAL DECKING
a. UNLESS NOTED OTHERWISE, METAL ROOF DECK SHALL BE 22 GAUGE TYPE B STEEL DECK. SEE ROOF DECK SCHEDULE FOR ATTACHMENTS.
b. UNLESS NOTED OTHERWISE, METAL FLOOR DECK SHALL BE 20 GAUGE TYPE W3 COMPOSITE UNVENTED STEEL DECK UNLESS NOTED OTHERWISE. ATTACH TO SUPPORTING STRUCTURE WITH 3/4" DIAMETER WELDS AT 6" MAXIMUM SPACING. ATTACH SIDE BEAMS WITH BUTT ON PUNCH OR SIDE BEAM SCREWS AT 24" MAXIMUM SPACING. AN HSA FIELD-WELDED THROUGH THE DECK MAY SUBSTITUTE FOR A PUNCH WELD.
c. ALL DECK SHALL BE CONTINUOUS OVER 3 SPANS. WHERE NOT POSSIBLE, THE DECK SUPPLIER/CONTRACTOR SHALL PROVIDE HEAVIER GAUGE DECK AS NEEDED TO PROVIDE THE EQUIVALENT PERFORMANCE OF THE SPECIFIED DECK WITH 3 SPAN CONTINUITY.
d. SEE TYPICAL DETAILS FOR SUPPORT OF DECK AT OPENINGS.
e. PROVIDE L2"x3/16" FOR DECK SUPPORT AT LOCATIONS WHERE COLUMNS EXTEND THROUGH DECK.
f. PAINTED STEEL DECK SHALL CONFORM TO EITHER ASTM A1008 OR A1039, GRADE 50 STEEL AND GALVANIZED STEEL DECK SHALL CONFORM TO EITHER ASTM A653 OR A1063, GRADE 50 STEEL, WITH A ZINC COATING DESIGNATION OF G90.
g. BUILDING ELEMENTS MAY BE SUPPORTED BY HANGING DIRECTLY FROM METAL DECKING, PROVIDED THAT THE TOTAL WEIGHT PER CONNECTION IS LESS THAN 50 LBS AND THAT THE ATTACHMENT TO THE DECKING IS DISTRIBUTED ACROSS AT LEAST TWO RIBS AND SPACED AT LEAST 6 FEET APART IN ANY DIRECTION.
8. ALL COLUMNS EMBEDDED IN OR LOCATED AT THE END OF MASONRY WALLS SHALL HAVE (2) 1/2" DIAMETER X 24" LONG DBA EXTENDING EACH WAY AT ALL WALL BOND BEAMS.
9. PROVIDE FULL DEPTH WEB STIFFENER PLATES AT EACH END OF STEEL BEAMS AT ALL BEARING (EXCEPT SECONDARY FRAMING) POINTS. STIFFENER PLATES SHALL BE THICKNESS SHOWN UNLESS NOTED OTHERWISE AND SHALL BE WELDED BOTH SIDES WITH FILLET WELDS ALL AROUND. FLANGE WIDTH = STIFFENER THICKNESS WELD THICKNESS
< 8 1/4" 1/4" 3/16"
8 1/4" < BF < 12 1/2" 3/8" 1/4"
12 1/2" < BF < 18" 1/2" 5/16"
10. FABRICATORS AND SUPPLIERS SHALL COORDINATE PAINT FINISHES WITH REQUIREMENTS FOR DIRECT APPLIED INSULATION, FIREPROOFING, ETC. AS NOTED IN THE PROJECT SPECIFICATIONS.
11. WHEN DETERMINING THE FIRE RESISTANCE OF ASSEMBLIES, USE THE FOLLOWING: STEEL ROOF MEMBERS ARE CONSIDERED UN-RESTRAINED AND STEEL FLOOR FRAMING MEMBERS ARE CONSIDERED RESTRAINED.
12. UNLESS NOTED OTHERWISE, ALL HORIZONTAL FRAMING MEMBERS SHALL BE ERECTED WITH THE NATURAL CROWN UP.
13. UNLESS OTHERWISE SHOWN OR DETAILED IN THE PLANS, ALL STEEL COLUMNS, BEAMS, BRACES, STRUTS, ETC. SHALL BE CONTINUOUS BETWEEN CONNECTIONS OR SUPPORTS. SPLICES IN MEMBERS SHALL NOT BE PERMITTED WITHOUT WRITTEN APPROVAL BY THE ENGINEER OF RECORD.

K. OPEN WEB JOISTS AND GIRDERS

- 1. ALL OPEN WEB STEEL JOISTS AND GIRDERS SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF "STANDARD SPECIFICATIONS AND CODE OF STANDARD PRACTICE" OF THE STEEL JOIST INSTITUTE.
2. (#####) DENOTES APPLIED TOTAL AND LIVE UNIFORMLY DISTRIBUTED LOADS IN POUNDS PER LINEAR FOOT OF JOIST, RESPECTIVELY.
3. SEE JOIST LOAD PROFILES FOR SPECIALLY LOADED JOISTS.
4. CONCENTRATED POINT LOADS, NOT SPECIFICALLY SHOWN ON THE PLANS, OF LESS THAN 100 POUNDS FOR MECHANICAL UNITS, FIRE SPRINKLER MAINS, AND OTHER EQUIPMENT SHALL BE ALLOWED WITHOUT REQUIRING ADDITIONAL WEB MEMBERS TO BE INSTALLED. WHERE THE LOAD EXCEEDS 100 POUNDS, THE LOAD SHALL BE SUPPORTED WITHIN 6" OF A CHORD PANEL POINT. SUPPORT BEYOND 6" FROM PANEL POINTS CAN BE PROVIDED BY ADDING (2) L2 x 2 x 1/4 DIAGONALS TO THE NEAREST OPPOSITE CHORD PANEL POINT PER TYPICAL DETAIL. CONNECTIONS SHALL BE MADE CONCENTRIC TO THE CHORD ANGLES. BEAM CLAMPS, OR SIMILAR ECCENTRIC ATTACHMENTS, ARE NOT ALLOWED, EXCEPT AS INDICATED BELOW. BEAM CLAMPS, OR SIMILAR ATTACHMENTS THAT ARE NOT CENTERED ON THE CHORD ANGLES MAY ONLY BE USED FOR LOADS LESS THAN 10 POUNDS. SEE JOIST SUBMITTAL FOR ADDITIONAL REQUIREMENTS. ALL LOADS PROVIDED FOR IN THIS NOTE SHALL BE ACCOUNTED FOR IN THE SPECIFIED DESIGN LOADS.
5. ANY BRACINGS REQUIRED FOR MISCELLANEOUS ITEMS (I.E. DUCTWORK, PIPING, ETC.) MUST CONNECT TO THE TOP CHORD OF THE JOIST OR GIRDER. BRACING TO THE BOTTOM CHORD IS NOT ALLOWED UNLESS SPECIFICALLY DETAILED THAT WAY ON THE PLANS.
6. PROVIDE SPECIAL BEARING ENDS AS REQUIRED AT SLOPED BEARING CONDITIONS. CONTRACTOR SHALL COORDINATE WITH OTHER STRUCTURAL ELEMENTS.
7. ALL JOISTS SHALL BE CAMBERED PER SJI SPECIFICATIONS, UNLESS NOTED OTHERWISE.
8. FIELD MODIFICATIONS (INCLUDING HOLES IN THE CHORD OR WEB MEMBERS) SHALL NOT BE MADE TO ANY JOIST OR GIRDER WITHOUT PRIOR APPROVAL BY THE MANUFACTURER.
9. FABRICATORS AND SUPPLIERS SHALL COORDINATE PAINT FINISHES WITH REQUIREMENTS FOR DIRECT APPLIED INSULATION, FIREPROOFING, ETC. AS NOTED IN THE PROJECT SPECIFICATIONS.
10. JOIST BRIDGING SHALL BE PROVIDED AS REQUIRED BY THE JOIST MANUFACTURER AND SJI STANDARDS. BRIDGING WHERE SHOWN ON THE STRUCTURAL DRAWINGS IS A SCHEMATIC REPRESENTATION ONLY. SEE JOIST MANUFACTURER FOR BRIDGING SIZE, CONNECTIONS, TYPE AND QUANTITY.
11. WHERE ADDED LOADS ARE SHOWN ON THE JOISTS BUT NOT SPECIFICALLY DIMENSIONED, THE JOIST DESIGNER SHALL PLACE THOSE LOADS ON THE JOIST AT A LOCATION THAT RESULTS IN THE HIGHEST STRESS IN THE MEMBERS. THE DESIGNER MAY ASSUME THAT THE LOAD OCCURS WITHIN 10 FEET OF A SCALED DIMENSION.
12. FABRICATOR MUST SUBMIT A CERTIFICATE OF COMPLIANCE TO THE BUILDING OFFICE PER IBC 2207.5 STATING THAT WORK WAS PERFORMED IN ACCORDANCE WITH APPROVED CONSTRUCTION DOCUMENTS AND WITH SJI SPECIFICATIONS.
13. UNLESS NOTED OTHERWISE, ROOF JOISTS AND GIRDERS SHALL BE DESIGNED FOR A NET WIND UPLIFT OF 10 PSF.
14. ROOF JOISTS BEARING ON EXTERIOR WALLS SHALL BE DESIGNED TO TRANSFER 3.75 KIPS (ULTIMATE) TOP CHORD AXIAL FORCE THROUGH THE BEARING SCHEDULE.
15. ALL JOISTS AT GRID LINES SHALL BE DESIGNED TO RESIST A 10 KIP TOP CHORD AXIAL FORCE (ULTIMATE).
16. ALL STANDARD AND NON STANDARD SJI JOISTS SHALL BE DESIGNED FOR THE FOLLOWING DEFLECTION CRITERIA:
a. LIVE LOAD : L/240 TOTAL LOAD : L/180
17. JOIST MANUFACTURER SHALL APPLY ADDITIONAL POINT OR LINE LOAD AS REQUIRED TO SUPPORT FIRE PROTECTION MAIN LINES 4" DIAMETER OR GREATER. JOIST MANUFACTURER SHALL COORDINATE WITH GENERAL CONTRACTOR TO OBTAIN LOCATIONS AND WEIGHTS OF THESE LINES. SEISMIC BRACING LOADS FOR FIRE PROTECTION MAIN LINES SHALL ALSO BE ACCOUNTED FOR IN THE JOIST MANUFACTURER'S DESIGN.

L. MASONRY

- 1. ALL HOLLOW MASONRY UNITS SHALL CONFORM TO ASTM C-90.
1m (MINIMUM FACTORED) 2,000 PSI
MINIMUM UNIT STRENGTH 2,000 PSI (TESTED IN ACCORDANCE WITH ASTM C-140)
2. ACCEPTABLE RANGE OF UNIT WEIGHT : 105 PCF TO 125 PCF
3. ALL GROUT (SITE MIXED OR PRE-MIXED) SHALL CONFORM TO ASTM C-476 OR SECTION 2.2A OF TMS 602-16. GROUT SHALL BE PLACED WITH SUFFICIENT WATER FOR POURING WITHOUT SEGREGATION. DO NOT USE MORTAR FOR GROUT. MECHANICALLY VIBRATE ALL GROUT.
4. GROUT STOPS SHALL BE AN APPROVED PRODUCT DESIGNED AND MANUFACTURED FOR USE AS A GROUT STOP. GROUT STOP SUBMITTALS SHALL BE SUBMITTED TO THE ARCHITECT AND ENGINEER FOR REVIEW. OTHER GROUT STOP MATERIALS SUCH AS ASPHALT IMPREGNATED MATERIALS ARE NOT PERMITTED.
5. MORTAR SHALL BE TYPE S AND SHALL CONFORM TO ASTM C-270.
6. ALL MASONRY WORK SHALL CONFORM TO CHAPTER 21 OF THE IBC. UNLESS NOTED OTHERWISE, MINIMUM REINFORCING IN ALL 8" MASONRY WALLS SHALL BE AS FOLLOWS:
a. VERTICAL : #5 BARS IN GROUTED CELLS ADJACENT TO ALL OPENINGS GREATER THAN 24 INCHES WIDE, ON EACH SIDE OF CONTROL JOINTS, AT ENDS OF WALL, AND AT A MAXIMUM SPACING OF 32" THROUGHOUT THE WALL. AT CORNERS, PROVIDE A MINIMUM OF (4) VERTICAL BARS AT "T" JOINTS AND (3) VERTICAL BARS AT "L" JOINTS. SEE THE TYPICAL DETAIL / SCHEDULE FOR MORE INFORMATION. ALL VERTICAL REINFORCING SHALL BE DOVELETTED INTO THE FOUNDATION WALL UNLESS SPECIFICALLY DETAILED OTHERWISE.
b. HORIZONTAL : (2) #4 BARS IN 8" DEEP "H" BLOCK BOND BEAM UNITS AT 48" O.C. AND AT FLOORS, ROOF, BELOW OPENINGS, AND TOP OF WALL. BOND BEAMS AT ROOF SHALL SLOPE TO MATCH SLOPING ROOF. SEE THE MASONRY REINFORCING SCHEDULE FOR MASONRY BEAMS ABOVE OPENINGS.
7. SEE THE MASONRY REINFORCING SCHEDULE FOR OPENINGS WHICH EXCEED 32 INCHES IN EITHER DIRECTION.
8. ALL BLOCK CELLS CONTAINING REINFORCING, BOLTS, OR ANCHORS SHALL BE GROUTED SOLID. PROVIDE (1) #5 (MINIMUM) IN GROUTED SPACE ON ALL SIDES AND ADJACENT TO EVERY OPENING WHICH EXCEEDS 24" IN EITHER DIRECTION. HORIZONTAL BARS SHALL EXTEND 24" BEYOND THE CORNERS OF THE OPENING AND VERTICAL BARS SHALL EXTEND TO TOP OF WALL. VERTICAL REINFORCING SHALL BE PROVIDED AT ENDS, CORNERS AND EACH SIDE OF CONTROL JOINTS. SEE TYPICAL DETAILS FOR OPENINGS WHICH EXCEED 32" IN EITHER DIRECTION.
9. SOLID GROUTING OF MASONRY IS UNACCEPTABLE EXCEPT AS SPECIFICALLY NOTED ON PLANS AND SCHEDULES.
10. WHERE WALLS ARE NOT GROUTED SOLID, EACH GROUT POUR SHALL TERMINATE FLUSH WITH THE TOP OF THE UPPERMOST UNIT EXCEPT AT CELLS WITH VERTICAL REINFORCING WHERE GROUT SHALL BE 1-1/2" BELOW TOP OF UNIT TO PROVIDE CONSTRUCTION KEY. WHERE WALLS ARE GROUTED SOLID, EACH GROUT POUR SHALL TERMINATE 1-1/2" BELOW TOP OF UNIT.
11. GROUT POURS SHALL NOT EXCEED 5'-0" UNLESS HIGH LIFT GROUTING PROCEDURES ARE FOLLOWED. THE USE OF HIGH LIFT GROUTING PROCEDURES REQUIRE THE APPROVAL OF THE ARCHITECT AND ENGINEER AND SHALL NOT EXCEED THE MAXIMUM HEIGHTS GIVEN IN TABLE 3.2.1 OF TMS 402-16. GROUT DEMONSTRATION PANELS, AS PRESCRIBED BY THE ARCHITECT AND ENGINEER, SHALL BE REQUIRED WHERE REQUESTED GROUTING PROCEDURES DO NOT MEET THE LIMITS OF TABLE 3.2.1. ADDITIONALLY, ALL HIGH LIFT GROUTING SHALL REQUIRE SPECIAL INSPECTION PROCEDURES NEEDED TO VERIFY GROUT PLACEMENT DURING CONSTRUCTION. DURING THE SUBMITTAL FOR APPROVAL PROCESS, SUBMITTAL SHALL INCLUDE, BUT NOT BE LIMITED TO: STATEMENT OF PROCEDURE FOR MECHANICAL VIBRATION OF HIGH LIFT GROUT; NEW MIX DESIGNS FOR HIGH SLUMP, HIGH LIFT GROUT; FOR SELF-CONSOLIDATING GROUT; SUBMIT MIX DESIGNS, SLUMP FLOW RATES, VISUAL STABILITY INDEX (VSI), AND QUANTITIES OF ADMIXTURES BEING USED.
12. ALL MASONRY BEAMS SHALL BE BUILT INTEGRAL WITH SUPPORT. NO TOOTHING OR DOWELING PERMITTED. UNITS WITH ONE END OPEN SHALL BE USED FOR ALL MASONRY BEAMS.
13. PROVIDE VERTICAL CONTROL JOINTS AT MAXIMUM SPACINGS NOTED BELOW UNLESS NOTED OTHERWISE IN THE SPECIFICATIONS AND/OR ON ARCHITECTURAL ELEVATIONS AND AT ALL CHANGES IN WALL ELEVATION AND MASONRY THICKNESS. CONTROL JOINTS SHALL NOT BE LOCATED DIRECTLY OVER OR CLOSER THAN 24" TO WALL OPENINGS (DOORS, WINDOWS, MECHANICAL OPENINGS, ETC.), OR WITHIN MASONRY JAMBS.
REINFORCED MASONRY : 40 FT VENEER : 30 FT AND AT INTERFACE BETWEEN VENEER SUPPORTED BY FOUNDATIONS AND SUSPENDED STRUCTURAL ELEMENTS.
14. HORIZONTAL REINFORCEMENT SHALL TERMINATE AT EACH SIDE OF CONTROL JOINTS EXCEPT AT FLOOR AND ROOF LEVEL BOND BEAMS AND AT TOP OF PARAPET.
15. CONTROL JOINTS SHALL BE PROVIDED AT THE MASONRY SIDE OF EMBEDDED STEEL COLUMNS TO CONTROL CRACKING OF FACE SHELLS.
16. SUPPORT NON-BEARING, NON-STRUCTURAL WALLS AT TOP OF MASONRY AS PER TYPICAL DETAILS AT LOCATIONS WHERE INTERSECTING OR PERPENDICULAR WALLS ARE 12'-0" OR MORE APART OR WHERE END OF WALL OCCURS 6'-0" OR MORE FROM INTERSECTING WALL.
17. EMBED CHANNELS AND PLATES TO BE PLACED SO AS TO CREATE FLUSH SURFACE WITH FACE OF MASONRY. FLANGES ON CHANNEL EMBEDS SHALL BE HORIZONTAL.
18. ALL VERTICAL REINFORCING SHALL BE SECURED IN PLACE PRIOR TO GROUTING USING WIRE POSITIONERS OR OTHER ACCEPTABLE DEVICES. REINFORCING SHALL BE SECURED AT BAR-SPLICE LOCATIONS AND AT A SPACING NOT MORE THAN 120 BAR DIAMETERS.
19. UNLESS NOTED OTHERWISE, MASONRY WALLS SHALL BE CONSTRUCTED UTILIZING COMMON RUNNING-BOND WITH FULLY MORTARED BED JOINTS AROUND GROUTED CELLS.
20. MASONRY VENEER SHALL BE ANCHORED USING THE HOHMANN AND BARNARD VENEER ANCHOR ASSEMBLY SYSTEM, OR AN APPROVED EQUAL, REGARDLESS OF BACK-UP SYSTEM. PROVIDE A CONTINUOUS HORIZONTAL 9 GAUGE WIRE AT 18" O.C. IN VENEER MORTAR JOINTS FOR ANCHOR ATTACHMENT. POSITIVE ANCHORAGE TO THE WIRE USING THE SEISMICLIP INTERLOCK SYSTEM SHALL BE PROVIDED TO SUPPORT NOT MORE THAN 2 SQUARE FEET OF WALL, WITH A HORIZONTAL SPACING NOT EXCEEDING 18".
a. METAL STUDS, USE HOHMANN AND BARNARD HB-213 S.I.S. (SEISMICLIP INTERLOCK SYSTEM) HEAVY DUTY ANCHORS OR AN APPROVED EQUAL. THE HB-213 ASSEMBLY SHALL BE ATTACHED TO WOOD STUDS USING A # 12 X 7 WOOD SCREWS OR TO METAL STUDS USING #10 SCREWS.
b. BRICK AND BLOCK WALLS: USE HOHMANN AND BARNARD 270-M.L.S.I.S. (SEISMICLIP INTERLOCK SYSTEM) MIGHTY-LOK SEISMIC ANCHORS OR AN APPROVED EQUAL. AT SPACINGS NOTED ABOVE, INSTALL A 2 GAUGE WIRE LADDER TYPE JOINT REINFORCEMENT AT 18" O.C. IN THE BACK-UP WALL FOR ANCHORAGE ATTACHMENT.
c. CONCRETE WALLS: USE HOHMANN AND BARNARD HB-305S SEISMIC NOTCH DOVE TAIL ANCHOR SYSTEM OR AN APPROVED EQUAL AT SPACINGS NOTED ABOVE.
21. ELECTRICAL CONDUIT SHALL NOT BE PLACED IN CELLS THAT CONTAIN REBAR. CONDUIT IS ALLOWED TO PASS THROUGH REINFORCED CELLS WHEN IT OCCURS PERPENDICULAR TO THE REBAR. CONDUIT SHALL NOT CONTACT REBAR AS IT PASSES. THERE SHALL BE 1" CLEAR BETWEEN CONDUIT AND REBAR.

M. STRUCTURAL DELEGATED DESIGNS AND DEFERRED SUBMITTALS

- 1. STRUCTURAL DELEGATED DESIGNS AND SUBSEQUENT DEFERRED SUBMITTALS ARE FOR ELEMENTS, PARTS, OR PORTIONS OF THE OVERALL STRUCTURAL SYSTEM THAT ARE INDICATED OR REFERRED TO ON THESE DRAWINGS AND THAT ARE CRITICAL TO THE PERFORMANCE OF THE OVERALL STRUCTURAL SYSTEM. DESIGN CRITERIA HAS BEEN PROVIDED FOR THESE ITEMS IN THE STRUCTURAL NOTES, PLANS, AND DETAILS.
2. STRUCTURAL DEFERRED SUBMITTALS ARE COMPLETE PACKAGES TO BE SUBMITTED FOR REVIEW THAT INCLUDE DRAWINGS AND CALCULATIONS FOR ALL DELEGATED DESIGN ITEMS AND THEIR CONNECTIONS. DEFERRED SUBMITTALS SHALL BEAR THE STAMP AND SIGNATURE OF THE DESIGN PROFESSIONAL RESPONSIBLE FOR THEIR DESIGN.
3. ARW ENGINEERS WILL REVIEW STRUCTURAL DEFERRED SUBMITTALS TO VERIFY DESIGN CRITERIA IS COMPLIANT WITH THE APPROVED CONSTRUCTION DOCUMENTS.
4. STRUCTURAL DELEGATED DESIGN COMPONENTS SHALL NOT BE INSTALLED UNTIL APPROVED BY THE BUILDING OFFICIAL.
5. STRUCTURAL DELEGATED DESIGN ITEMS REQUIRING DEFERRED SUBMITTALS INCLUDE, BUT ARE NOT LIMITED TO:
a. OPEN WEB JOISTS & GIRDERS, BRIDGING, BRACING, CONNECTIONS, AND RELATED COMPONENTS.

N. NON-STRUCTURAL DELEGATED DESIGNS AND DEFERRED SUBMITTALS

- 1. NON-STRUCTURAL DELEGATED DESIGNS AND SUBSEQUENT DEFERRED SUBMITTALS ARE FOR ITEMS NOT INCLUDED IN THE STRUCTURAL DELEGATED DESIGN SECTION. THESE ARE ITEMS THAT ARE NOT CRITICAL TO THE OVERALL PERFORMANCE OF THE STRUCTURAL SYSTEM BUT THAT IMPART LOADS AND FORCES TO THE STRUCTURAL SYSTEM.
2. NON-STRUCTURAL DEFERRED SUBMITTALS SHALL BEAR THE STAMP AND SIGNATURE OF THE DESIGN PROFESSIONAL RESPONSIBLE FOR THE DESIGN.
3. ARW ENGINEERS WILL REVIEW NON-STRUCTURAL DEFERRED SUBMITTALS TO VERIFY DESIGN CRITERIA IS COMPLIANT WITH THE APPROVED CONSTRUCTION DOCUMENTS.
4. IF THE STRUCTURAL DRAWINGS INCLUDE LOADS TO ACCOMMODATE NON-STRUCTURAL ELEMENTS, THE CONTRACTOR SHALL SUBMIT DOCUMENTATION INDICATING THAT THE NON-STRUCTURAL ELEMENTS COMPLY WITH THE LOADING CRITERIA PROVIDED HEREIN. SUCH DOCUMENTATION SHALL BEAR THE STAMP AND SIGNATURE OF THE DESIGN PROFESSIONAL RESPONSIBLE FOR THE DESIGN.
5. WHEN THE NON-STRUCTURAL DEFERRED SUBMITTAL INDICATES THAT THE ELEMENT WILL IMPART FORCES IN EXCESS OF LOADS THAT ARE INDICATED ON THE STRUCTURAL DRAWINGS, THE CONTRACTOR SHALL SUBMIT A DETAILED GRAPHICAL REPRESENTATION OF THOSE DESIGN LOADS, INCLUDING MAGNITUDE, AND LOCATION. THE GRAPHIC SHALL BE ACCOMPANIED BY DOCUMENTATION INDICATING THAT THE NON-STRUCTURAL ELEMENT DESIGN COMPLIES WITH THE LOADING CRITERIA PROVIDED HEREIN. THE LETTER SHALL BEAR THE STAMP AND SIGNATURE OF THE DESIGN PROFESSIONAL RESPONSIBLE FOR THE DESIGN.
6. NON-STRUCTURAL DELEGATED DESIGN ITEMS REQUIRING DEFERRED SUBMITTALS SHALL INCLUDE, BUT ARE NOT LIMITED TO:
a. COLD FORMED STEEL STUDS / JOISTS / HEADERS / JAMBS / TRUSSES
b. SEISMIC BRACING OF ALL ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL ITEMS WHERE REQUIRED BY THE MOST RECENT VERSION OF ASCE 7 AND THE PROJECT CONTRACT DOCUMENTS.
c. STRUCTURAL STEEL STAIRS.

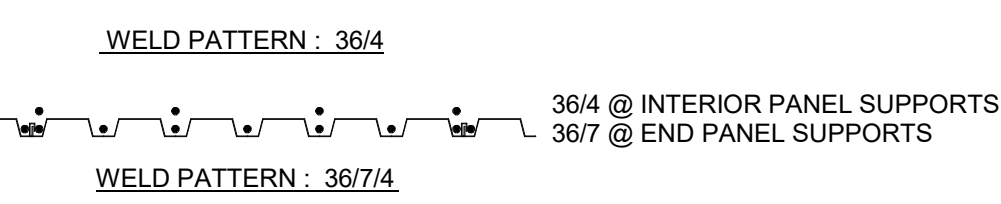
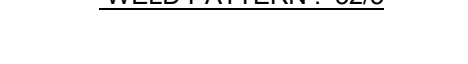
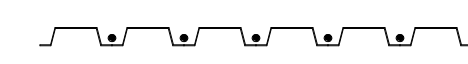
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Table with columns: MARK, DATE, DESCRIPTION. Includes project information: PROJECT #: 123005, DRAWN BY: BLP, CHECKED BY: J. Blazzard, ISSUED: 03.14.2024.



ROOF DECK SCHEDULE												
AREA	DECK		ATTACHMENT		SIDE SEAMS			SUPPORTS PARALLEL TO FLUTES		MIN. SHEAR CAPACITY (ASD)	MAX. FLEXIBILITY FACTOR	
	DEPTH	TYPE	GA.	DIA. WELD	PATTERN	#12 TEK SCREWS	TOP SEAM WELD	PUNCH LOCK #	Ø WELD			SPA.
A	3"	PLN3	20	3/4"	32/5	---	---	12"o.c.	3/4"	12"o.c.	795 PLF	14.3
B	1 1/2"	PLB	20	3/4"	36/7/4	---	---	24"o.c.	3/4"	12"o.c.	764 PLF	13.2
C	1 1/2"	PLB	20	3/4"	36/7/4	---	---	18"o.c.	3/4"	12"o.c.	908 PLF	12.1
D	1 1/2"	PLB	20	3/4"	36/7/4	---	---	12"o.c.	3/4"	12"o.c.	1168 PLF	11
E	1 1/2"	PLB	20	3/4"	36/7/4	---	---	8"o.c.	3/4"	12"o.c.	1499 PLF	10.1

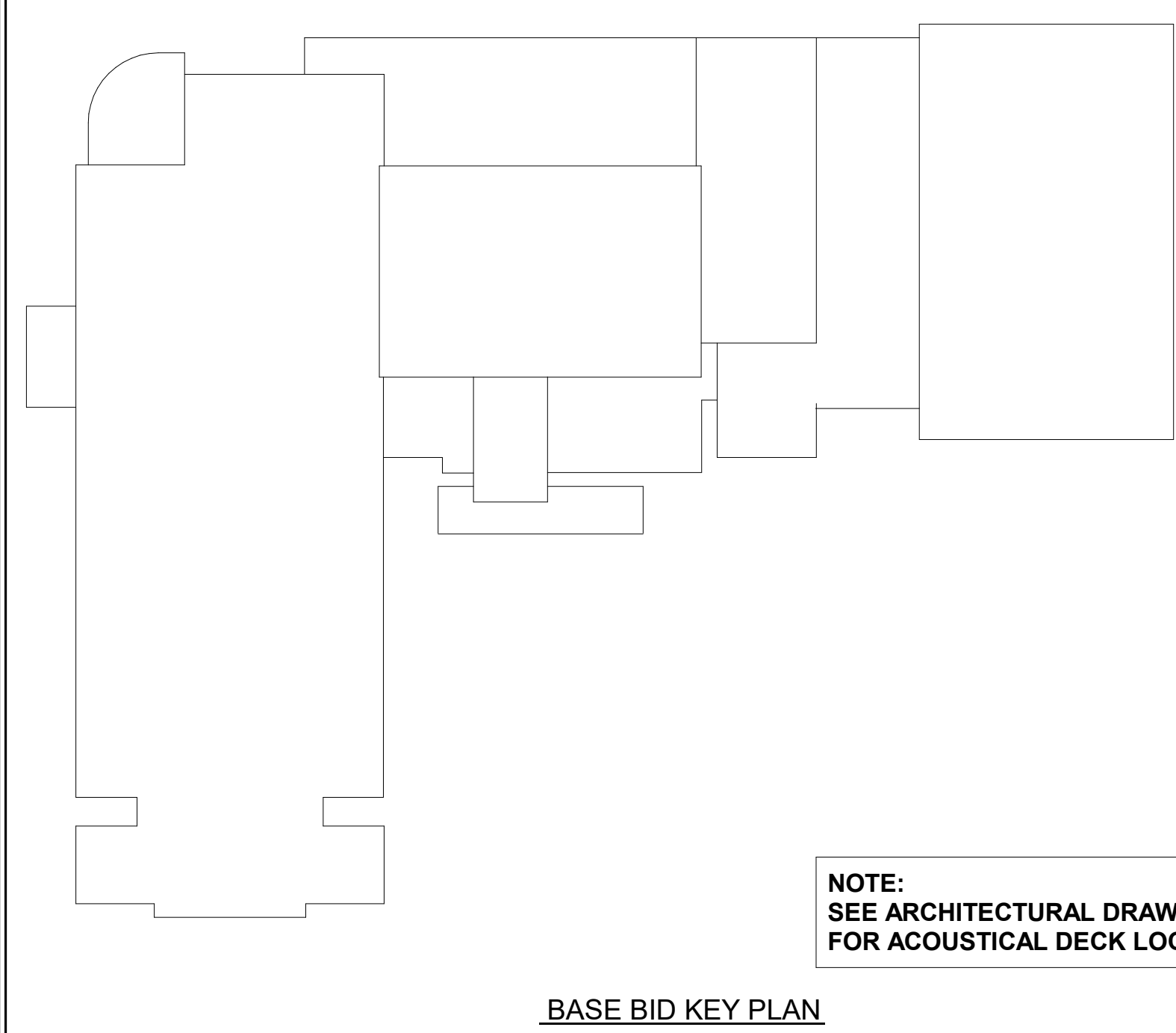
**FASTENING PATTERNS**



36/4 @ INTERIOR PANEL SUPPORTS  
36/7 @ END PANEL SUPPORTS

**NOTES:**

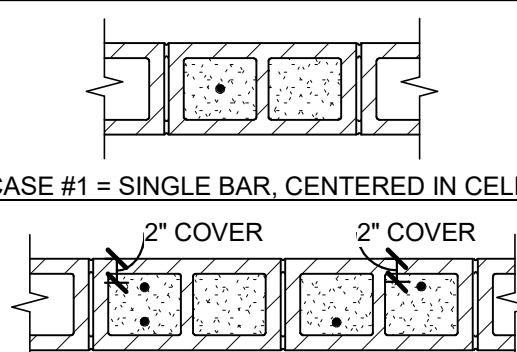
- TOP SEAM WELDS SHALL BE 1-1/2" LONG AND SHALL BE ACCORDING TO SDI STANDARDS.
- USE NESTABLE (OVERLAPPING) SIDE SEAMS AT SCREW ATTACHMENTS AND INTERLOCKING SIDE SEAMS AT WELDS.
- IF N DECK IS NOT NESTABLE, IN DECK END SUTT. JOINTS OVER STEEL JOISTS SHALL USE 16 GA. x 6' CONTINUOUS SHEET BETWEEN DECK AND JOIST TOP CHORD ANGLES. DECK WELDS TO PENETRATE SHEET AND ENGAGE JOIST CHORD.
- ALL DECK WITH A PROFILE DEPTH OF 2" OR LESS SHALL HAVE NESTED OR TELESCOPED END LAPS.
- SUBMIT CURRENT ICC APPROVAL FOR ALL DECKS.
- ALTERNATE SYSTEMS SHALL MEET OR EXCEED THE MINIMUM SHEAR CAPACITY AND SHALL PROVIDE LESS THAN OR EQUAL TO THE MAXIMUM FLEXIBILITY FACTOR LISTED IN THE SCHEDULE.
- ALL ALTERNATE SYSTEMS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW AND APPROVAL.



**NOTE:**  
SEE ARCHITECTURAL DRAWINGS FOR ACOUSTICAL DECK LOCATIONS.

**2021 IBC MASONRY REBAR LAP SPLICE SCHEDULE**

FOR MASONRY APPLICATIONS (TMS 402/602 - 16)



CASE #2 = WHEN REINFORCING BAR IS PLACED ADJACENT TO FACE SHELL

BAR LOCATION	MASONRY REINFORCING & SPLICE LENGTHS (IN) (f'm = 1500psi)									
	BAR SIZE #3		BAR SIZE #4		BAR SIZE #5		BAR SIZE #6		BAR SIZE #7	
	CASE #	CASE #	CASE #	CASE #	CASE #	CASE #	CASE #	CASE #	CASE #	CASE #
BEAM / WALL HORIZONTAL	19"	26"	32"	38"	45"					
WALL VERTICAL COLUMN AND JAMB	12"	15"	15"	26"	23"	40"	43"	54"	60"	63"

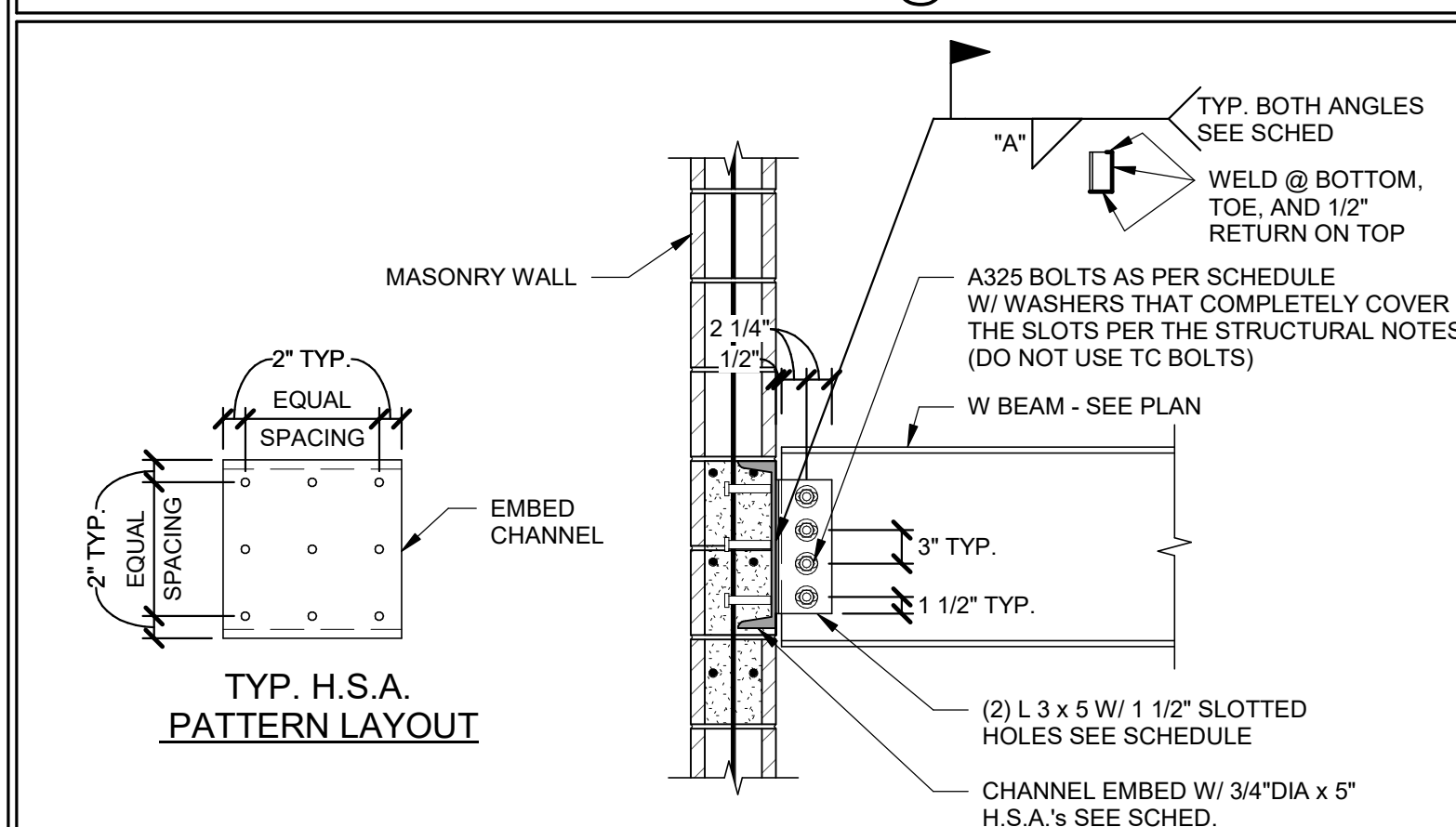
BAR LOCATION	MASONRY REINFORCING & SPLICE LENGTHS (IN) (f'm = 2000psi)									
	BAR SIZE #3		BAR SIZE #4		BAR SIZE #5		BAR SIZE #6		BAR SIZE #7	
	CASE #	CASE #	CASE #	CASE #	CASE #	CASE #	CASE #	CASE #	CASE #	CASE #
BEAM / WALL HORIZONTAL	19"	26"	32"	38"	45"					
WALL VERTICAL COLUMN AND JAMB	12"	13"	13"	22"	20"	35"	38"	54"	52"	63"

BAR LOCATION	MASONRY REINFORCING & SPLICE LENGTHS (IN) (f'm = 2500psi)									
	BAR SIZE #3		BAR SIZE #4		BAR SIZE #5		BAR SIZE #6		BAR SIZE #7	
	CASE #	CASE #	CASE #	CASE #	CASE #	CASE #	CASE #	CASE #	CASE #	CASE #
BEAM / WALL HORIZONTAL	19"	26"	32"	38"	45"					
WALL VERTICAL COLUMN AND JAMB	12"	12"	20"	18"	31"	34"	54"	47"	63"	

**NOTES:**

- MECHANICAL COUPLERS MAY BE USED IN LIEU OF LAP SPLICES SHOWN. SEE STRUCTURAL NOTES FOR MINIMUM COUPLER CAPACITY. WHERE MECHANICAL COUPLERS ARE USED, STAGGER ADJACENT SPLICES A MINIMUM OF 24" AS INDICATED ABOVE.
- DEVELOPMENT LENGTHS SHALL BE INCREASED BY 50% WHERE EPOXY COATED REBAR IS USED.
- WHEN SPLICING BARS OF DIFFERENT SIZES, USE LAP SPLICE LENGTH OF LARGER BARS UNLESS NOTED OTHERWISE.
- ALL REBAR #5 AND LARGER IN MASONRY SHALL BE SPLICED USING MECHANICAL SPLICES. SEE STRUCTURAL NOTES FOR MINIMUM COUPLER CAPACITY.

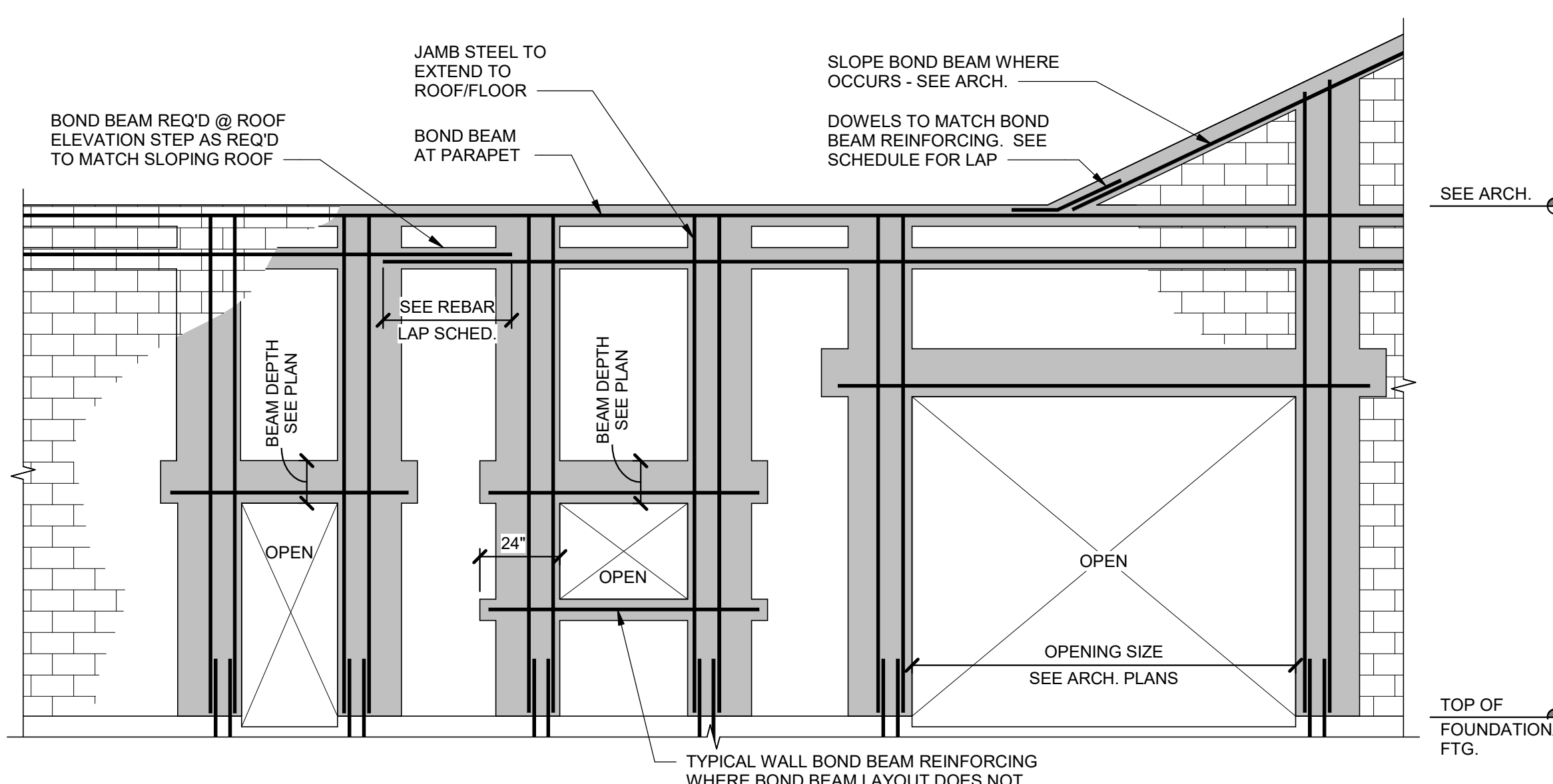
**BEAM CONNECTION SCHEDULE @ MASONRY WALL**



BEAM SIZE	ANGLE SIZE (EA. SIDE)	WELD 'A'	BOLTS	EMBED CHANNEL	# OF H.S.A.	H.S.A. PATTERN
W8, C8	3 x 5 x 1/4"	3/16"	(2) 3/4"Ø	C8 x 13.75 x 0-8"	4	..
W10, C10	3 x 5 x 5/16"	1/4"	(2) 3/4"Ø	C12 x 20.7 x 1-4"	6	...
W12, C12	3 x 5 x 5/16"	1/4"	(3) 3/4"Ø	C12 x 20.7 x 1-4"	6	...
W14	3 x 5 x 5/16"	1/4"	(3) 3/4"Ø	C15 x 33.9 x 1-4"	9	...
W16	3 x 5 x 5/16"	1/4"	(4) 3/4"Ø	C15 x 33.9 x 2-0"	12	...
W18	3 x 5 x 5/16"	1/4"	(5) 3/4"Ø	(2) C12 x 20.7 x 2-0"	12	...

- AT THE CONTRACTORS OPTION, (2) C8 x 13.75 MAY BE SUBSTITUTED FOR THE C15 x 33.9. H.S.A. PATTERN SHALL BE RECONFIGURED & EQUALLY DISTRIBUTED BETWEEN THE TWO CHANNELS.
- DOUBLE CHANNELS SHALL BE WELDED TOGETHER WITH A 2" STITCH WELD EVERY 4" AT THE FLANGE TOE.

**TYPICAL MASONRY / JAMB REINFORCING SCHEDULE**



**NOTES:**

- USE OPEN-END UNITS AT INTERSECTIONS OF BEAMS AND JAMBS.
- TYPICAL HORIZONTAL BOND BEAMS MAY BE ADJUSTED UP OR DOWN BY ONE COURSE PROVIDED THE OVERALL NUMBER OF REQUIRED BOND BEAMS ARE INSTALLED.
- TYPICAL HORIZONTAL AND VERTICAL WALL REINFORCING NOT SHOWN FOR CLARITY. SEE PLAN AND SCHEDULE FOR TYPICAL WALL REINFORCING.
- JAMB REINFORCING SHOWN IS SCHEMATIC. SEE SCHEDULE & DETAILS FOR ACTUAL JAMB REINFORCING.
- ALL VERTICAL WALL REINFORCING SHALL BE CONTINUOUS BETWEEN THE LEVELS IN WHICH THE WALL OCCURS.

**MASONRY BEAM SCHEDULE**

MARK	NOMINAL THICKNESS	BOTTOM REINF.	TOP REINF.	VERTICAL REINF.	MIN. GROUT DEPTH	OPENING Ø	COMMENTS
MB-1	8"	(2) #5	(2) #4	#4 @ 16"o.c.	16"	UP TO 5'-0"	
MB-2	8"	(2) #5	(2) #4	#4 @ 16"o.c.	32"	5'-1" TO 9'-0"	
MB-3	12"	(2) #5	(2) #4	#4 @ 16"o.c.	24"	UP TO 5'-0"	
MB-4	12"	(2) #5	(2) #4	#4 @ 16"o.c.	32"	5'-1" TO 7'-0"	

**NOTES:**

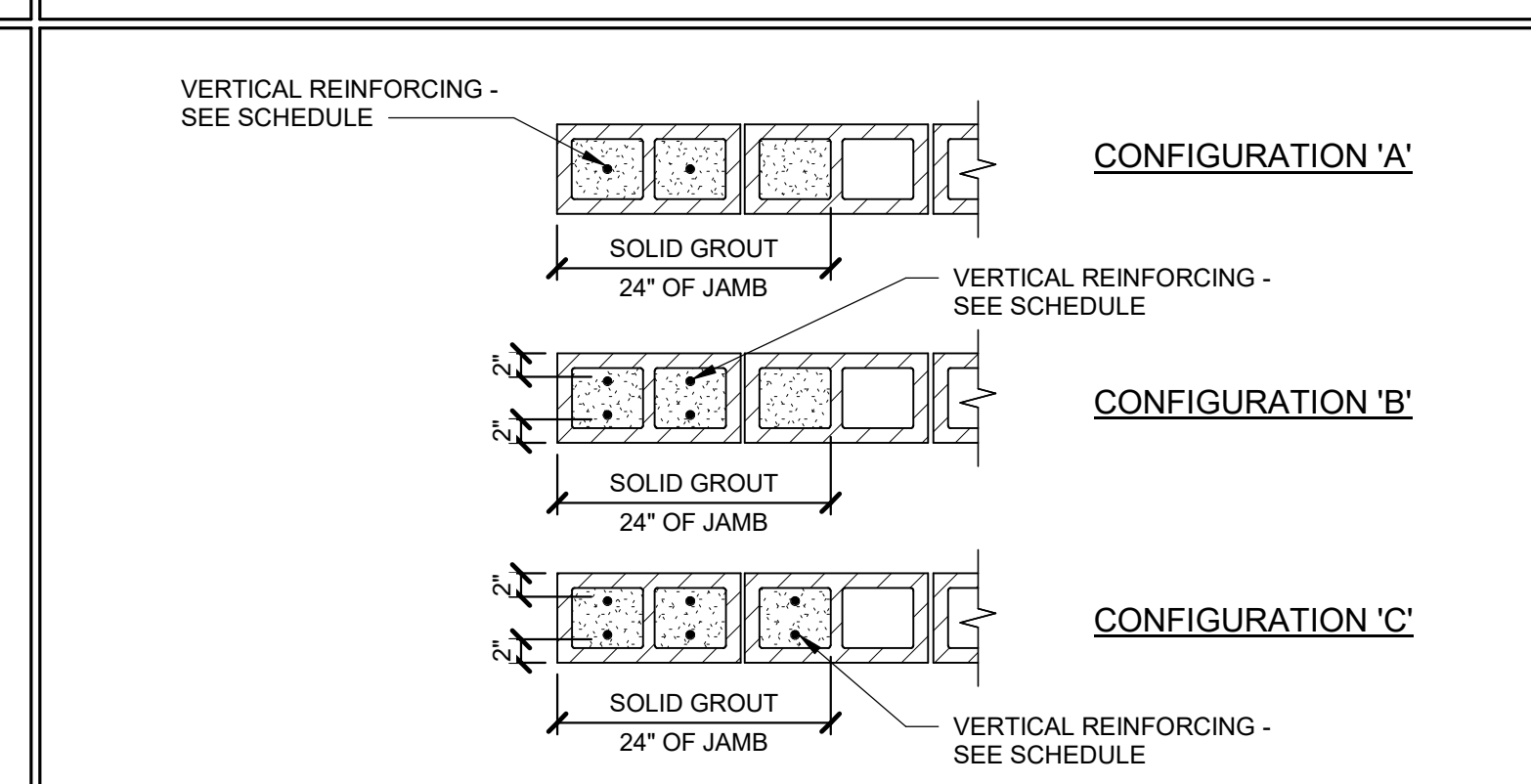
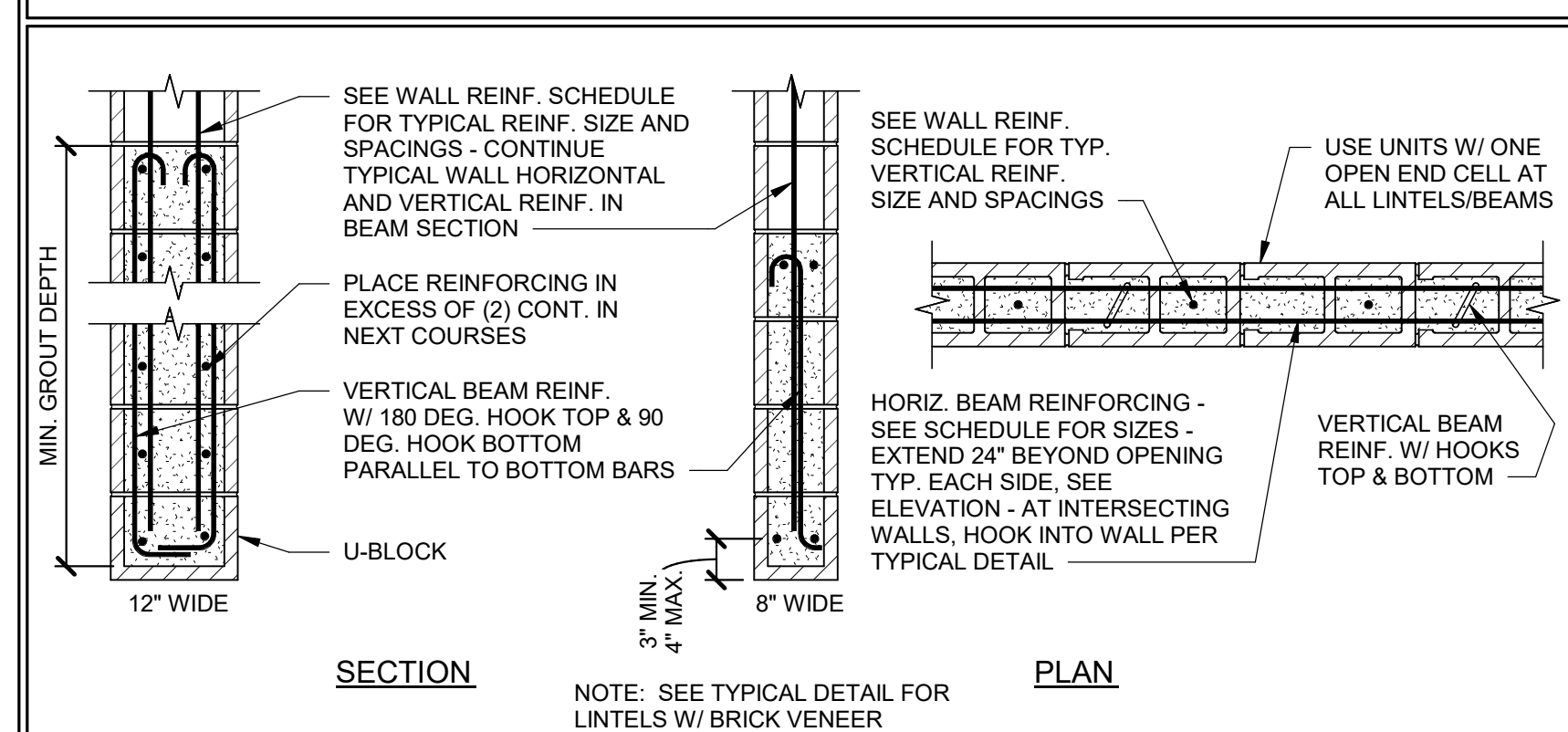
- WHERE SPECIFIC BEAMS ARE NOT NOTED ON THE PLANS - REFER TO OPENING SIZE FOR REQUIRED BEAM DEPTH AND REINFORCING.
- FIRST VERTICAL BAR TO BE WITHIN 8" OF END OF BEAM.
- SEE TYPICAL ELEVATION - VIEW OF BEAM.
- VERTICAL REINFORCING SHALL HAVE HOOKS TOP AND BOTTOM.

**MASONRY JAMB SCHEDULE**

MARK	NOMINAL THICKNESS	VERTICAL REINF.	TIES	CONFIG.	OPENING Ø	COMMENTS
MJ-1	8"	(2) #5	---	A	2'-8" TO 5'-0"	
MJ-2	8"	(4) #5	---	B	5'-1" TO 7'-0"	
MJ-3	8"	(6) #5	---	C	SEE PLAN	
MJ-4	8"	(8) #5	---	SEE PLAN	SIMILAR TO CONFIG. C - SOLID GROUT 32"	
MJ-5	12"	(4) #5	---	B	2'-8" TO 5'-0"	
MJ-6	12"	(6) #5	---	C	5'-1" TO 7'-0"	

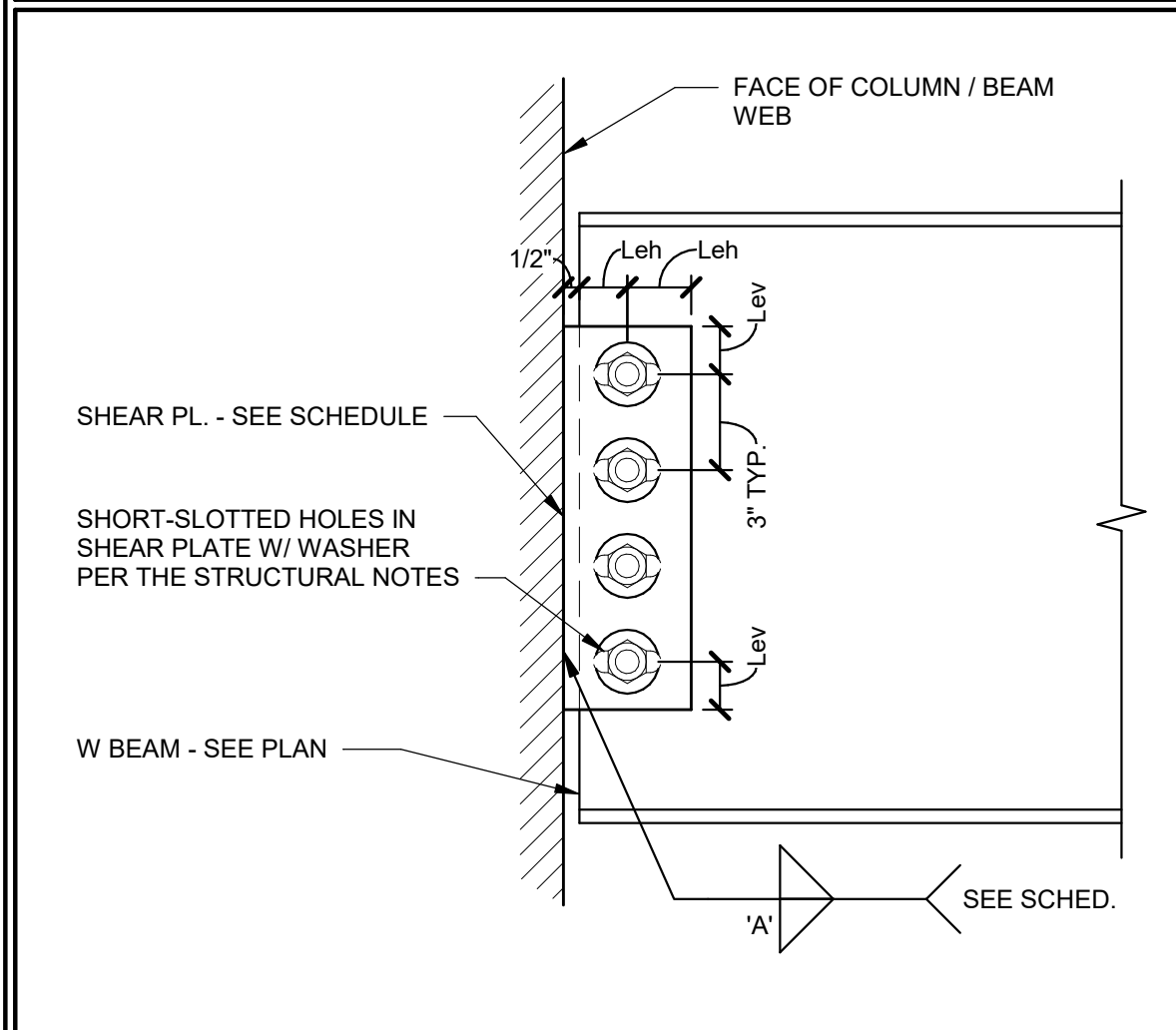
**NOTES:**

- WHERE SPECIFIC JAMBS ARE NOT NOTED ON THE PLANS - REFER TO OPENING SIZE FOR REQUIRED REINFORCING AND CONFIGURATION.
- ALL VERTICAL REINFORCING SHALL HAVE MATCHING DOWELS CAST INTO FOUNDATIONS.
- HORIZONTAL REINFORCING NOT SHOWN.
- JAMBS TO BE GROUTED SOLID.



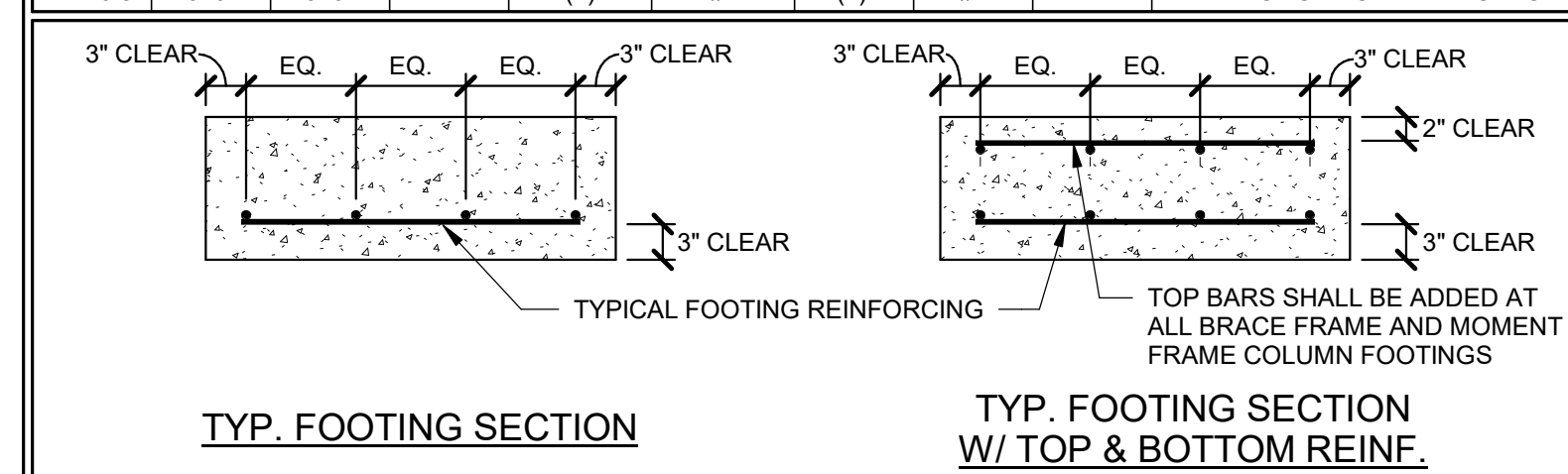
**BEAM CONNECTION SCHEDULE**

BEAM DEPTH	SHEAR PLATE INFORMATION				BOLTS W/ STANDARD WASHERS OVER SLOTS		WELD 'A'	COMMENTS
	PL. DIMENSIONS W/ SHORT SLOTTED HOLES	Lev	Leh	No.	SIZE			
W8, W10	PL. 1/4" x REQ'D	1 1/2"	2"	2	3/4"Ø	3/16"		
W12, W14	PL. 5/16" x REQ'D	1 1/2"	2"	3	3/4"Ø	1/4"		
W16	PL. 5/16" x REQ'D	1 1/2"	2"	4	3/4"Ø	1/4"		
W18	PL. 5/16" x REQ'D	1 1/2"	2"	5	3/4"Ø	1/4"		
W21	PL. 5/16" x REQ'D	1 1/2"	2"	6	3/4"Ø	1/4"		
W24	PL. 3/8" x REQ'D	1 1/2"	2"	7	7/8"Ø	1/4"		
W27	PL. 3/8" x REQ'D	1 1/2"	2"	7	7/8"Ø	1/4"		
W30	PL. 1/2" x REQ'D	1 3/4"	2"	8	1"Ø	5/16"		
W33	PL. 1/2" x REQ'D	1 3/4"	2"	9	1"Ø	5/16"		
W36	PL. 1/2" x REQ'D	2"	2 1/4"	10	1-1/8"Ø	5/16"		



**FOOTING SCHEDULE**

MARK	WIDTH	LENGTH	THICK	LENGTHWISE REINF.			CROSSWISE REINF.			REMARKS
				NO.	SIZE	SPA.	NO.	SIZE	SPA.	
FC2	2'-0"	CONT.	12"	(2)	#5	--	--	--		
FC2.5	2'-6"	CONT.	12"	(3)	#5	--	--	--		
FC3	3'-0"	CONT.	12"	(3)	#5	--	--	--		
FC4	4'-0"	CONT.	14"	(4)	#5	(1)	#5	12"o.c.		
FC5	5'-0"	CONT.	16"	(5)	#6	(1)	#6	12"o.c.		
FC6	6'-0"	CONT.	20"	(6)	#6	(1)	#6	12"o.c.	REINFORCE TOP & BOTTOM	
FC7	7'-0"	CONT.	22"	(7)	#7	(1)	#7	12"o.c.	REINFORCE TOP & BOTTOM	
FC8	8'-0"	CONT.	24"	(8)	#8	(1)	#8	12"o.c.	REINFORCE TOP & BOTTOM	
F3	3'-0"	3'-0"	12"	(3)	#5	(3)	#5	--		
F3.5	3'-6"	3'-6"	12"	(3)	#5	(3)	#5	--		
F4	4'-0"	4'-0"	12"	(4)	#5	(4)	#5	--		
F4.5	4'-6"	4'-6"	12"	(5)	#5	(5)	#5	--		
F5	5'-0"	5'-0"	14"	(5)	#5	(5)	#5	--		
F5.5	5'-6"	5'-6"	14"	(6)	#6	(6)	#6	--		
F6	6'-0"	6'-0"	16"	(6)	#6	(6)	#6	--		
F6.5	6'-6"	6'-6"	18"	(7)	#6	(7)	#6	--		
F7	7'-0"	7'-0"	18"	(7)	#6	(7)	#6	--		
F7.5	7'-6"	7'-6"	20"	(7)	#7	(7)	#7	--		
F8	8'-0"	8'-0"	22"	(8)	#7	(8)	#7	--		
F8.5	8'-6"	8'-6"	22"	(8)	#7	(8)	#7	--		
F9	9'-0"	9'-0"	24"	(9)	#7	(9)	#7	--		
F9.5	9'-6"	9'-6"	26"	(10)	#7	(10)	#7	--		
F10	10'-0"	10'-0"	26"	(10)	#8	(10)	#8	--		
FS36	3'-0"	6'-0"	12"	(3)	#5	(6)	#5	--		
FS6.5	6'-6"	6'-6"	24"	(7)	#7	(7)	#7	--	REINFORCE TOP AND BOTTOM	



**MASONRY WALL SCHEDULE**

MARK	THICK	VERT. REINF. SIZE	SPACE	HORIZ. BOND BEAM REINF.			COMMENTS	
				NO.	SIZE	SPACE		
MW-8A	8"	#5	32"o.c.	(2)	#4	48"	(2) #4	--
MW-8B	8"	#5	32"o.c.	(2)	#5	32"	(2) #5	(2) #5
MW-12A	12"	(2) #5	18"o.c.	(2)	#5	48"	--	SOLID GROUTED
MW-12B	12"	(2) #5	24"o.c.	(2)	#5	48"	(4) #5	(4) #5

**NOTES:**

- FOR ANY CMU WALLS NOT SPECIFICALLY CALLED OUT IN PLANS, USE MW-8A.
- VERTICAL REINFORCING TO BE @ CENTERLINE OF WALL WHERE SINGLE BAR IS NOTED IN SCHEDULE. POSITION BARS 2" FROM EACH FACE OF WALL WHERE (2) BARS ARE SPECIFIED.
- SOLID GROUTING OF WALLS IS UNACCEPTABLE EXCEPT WHERE SPECIFICALLY NOTED.
- SEE STRUCTURAL NOTES FOR ADDITIONAL INFORMATION.
- A BOND BEAM SHALL BE LOCATED IN THE FIRST COURSE ABOVE THE FOUNDATION IF VERTICAL DOWELS HAVE BEEN BENT TO ALIGN WITH VERTICAL CELLS, WHETHER OR NOT MASONRY WEBS HAVE BEEN CUT.

**BID PACKAGE 1**









































































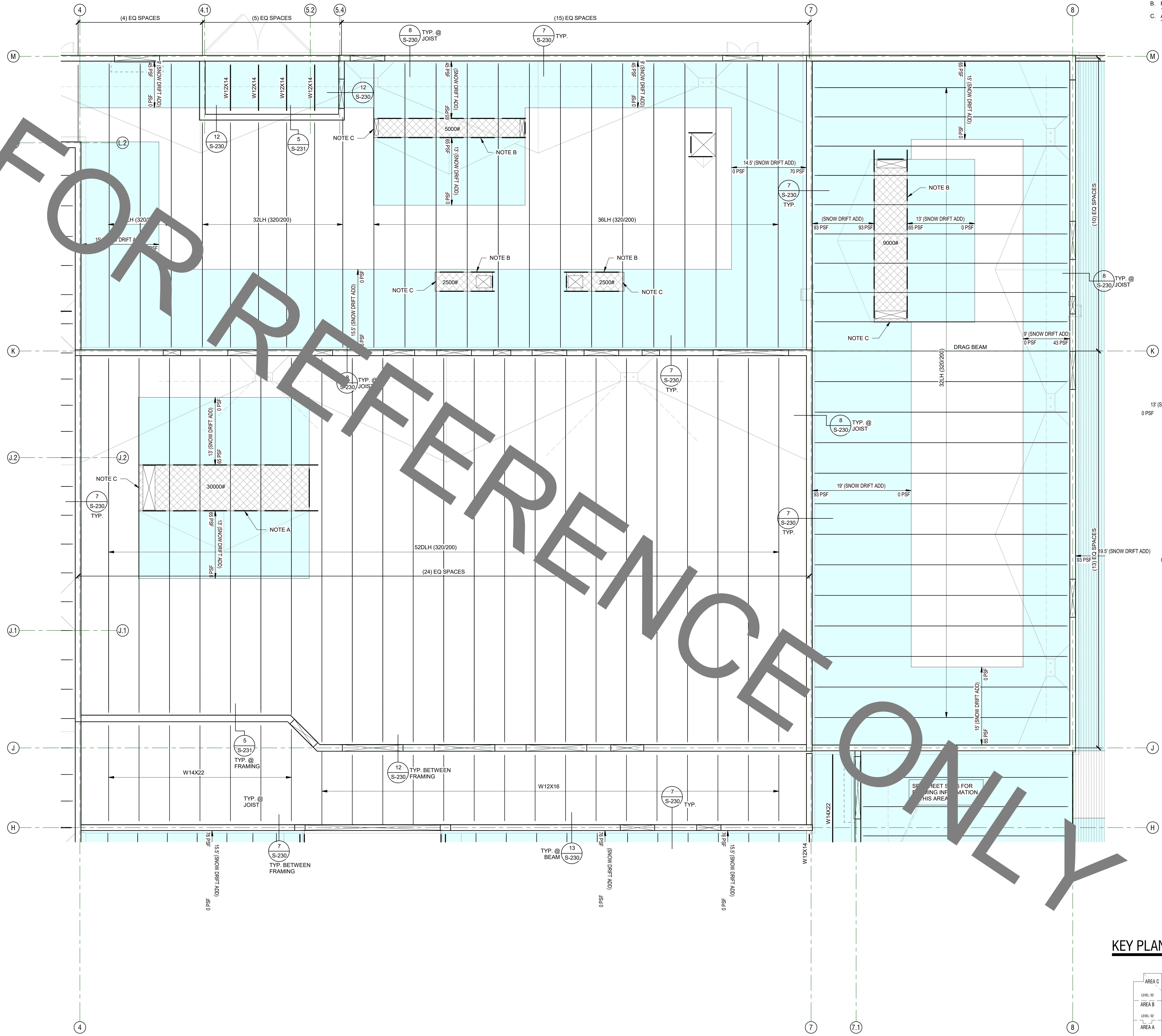






FOR REFERENCE ONLY

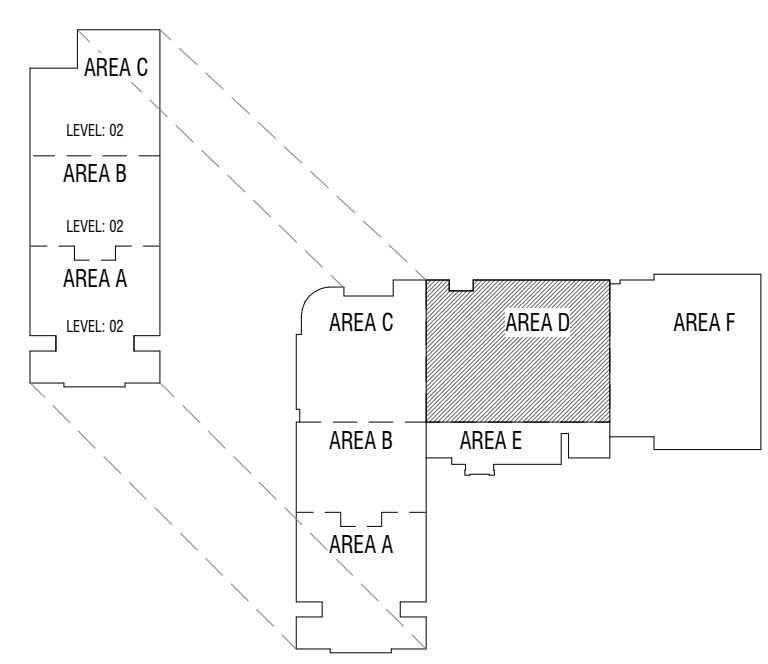
**MECHANICAL UNIT KEYED NOTES:**  
 A. PROVIDE FRAME BELOW UNIT AND AROUND OPENINGS PER DETAIL 9'S-202. PROVIDE LIGHTWEIGHT CONCRETE PAD BELOW UNIT AS INDICATED IN DETAIL.  
 B. PROVIDE FRAME BELOW UNIT AND AROUND OPENINGS PER DETAIL 12'S-202. DO NOT PROVIDE CONCRETE PAD BELOW UNIT.  
 C. ALIGN EDGE OF MECHANICAL UNIT OVER CENTER OF FRAMING AT THIS LOCATION.



**ROOF FRAMING PLAN - AREA D**  
 SCALE: 1/8" = 1'-0"  
 SEE SHEET S-130 FOR ROOF FRAMING NOTES

A  
S-134

**KEY PLAN**



**BID PACKAGE 1**

MARK	DATE	DESCRIPTION

PROJECT #: 123005  
 DRAWN BY: BLP  
 CHECKED BY: J. Blazzard  
 ISSUED: 03.14.2024















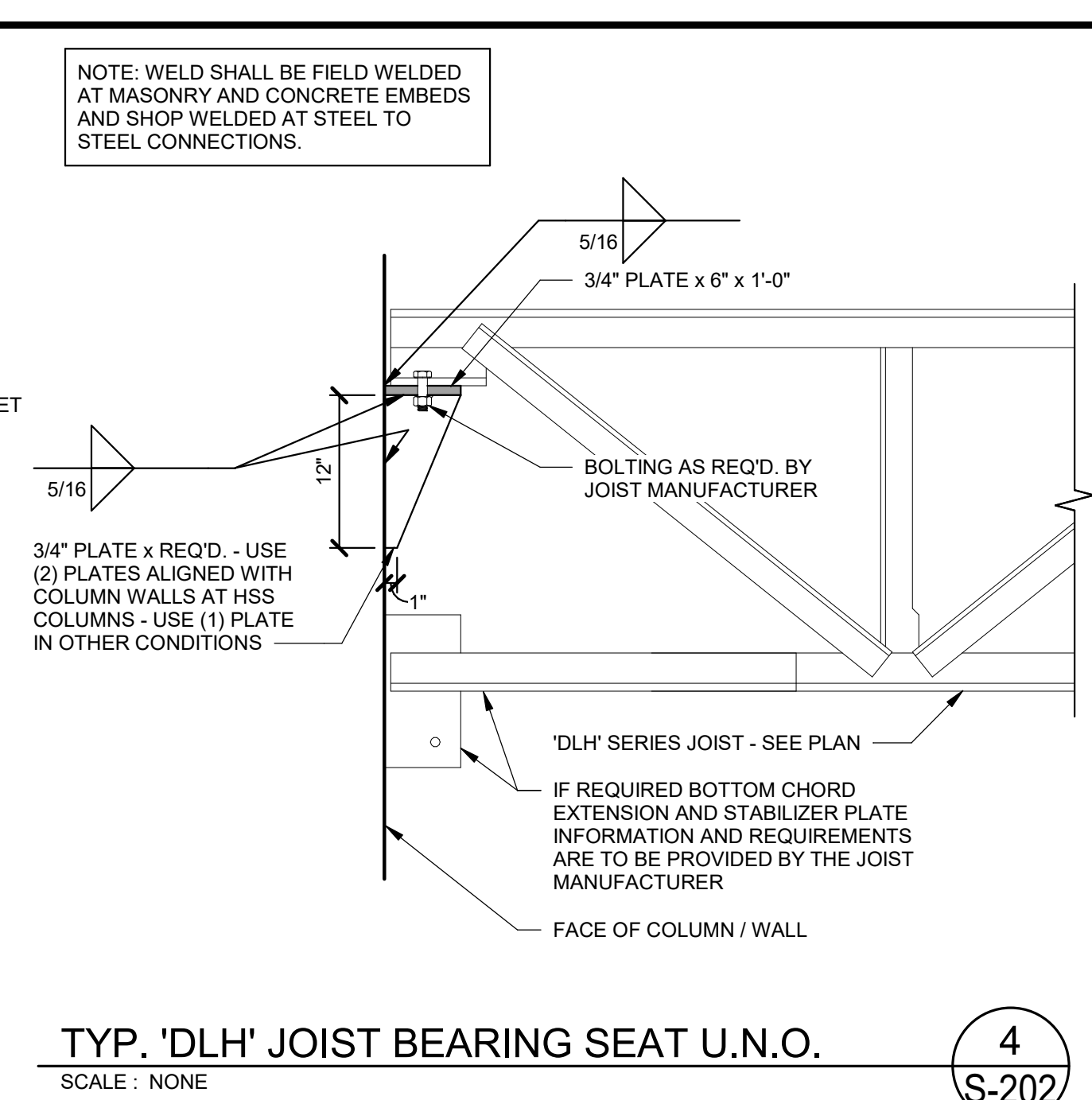




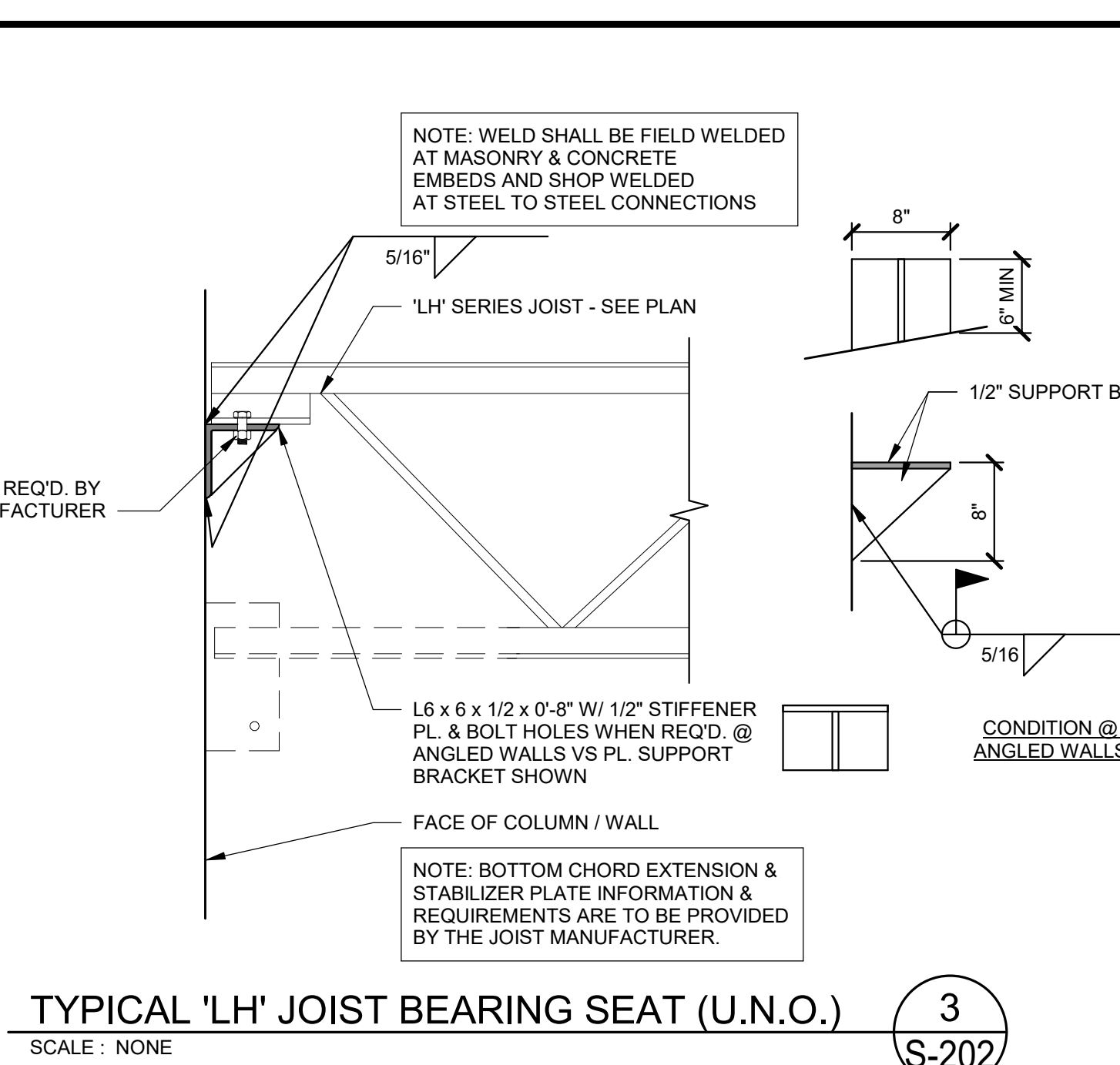
MARK	DATE	DESCRIPTION

PROJECT #: 123005  
 DRAWN BY: BLP  
 CHECKED BY: J. Blazzard  
 ISSUED: 03.14.2024

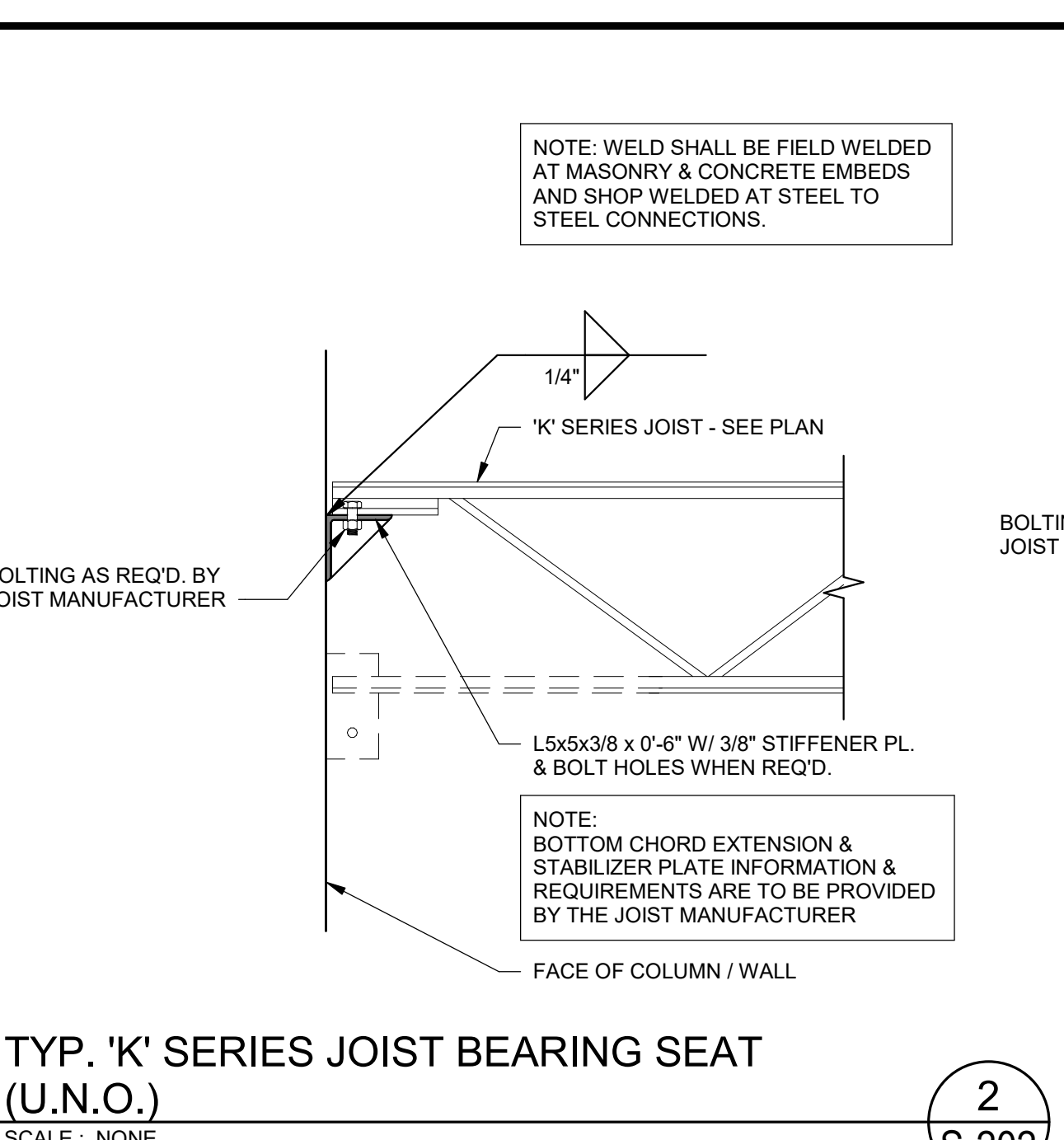
BID PACKAGE 1



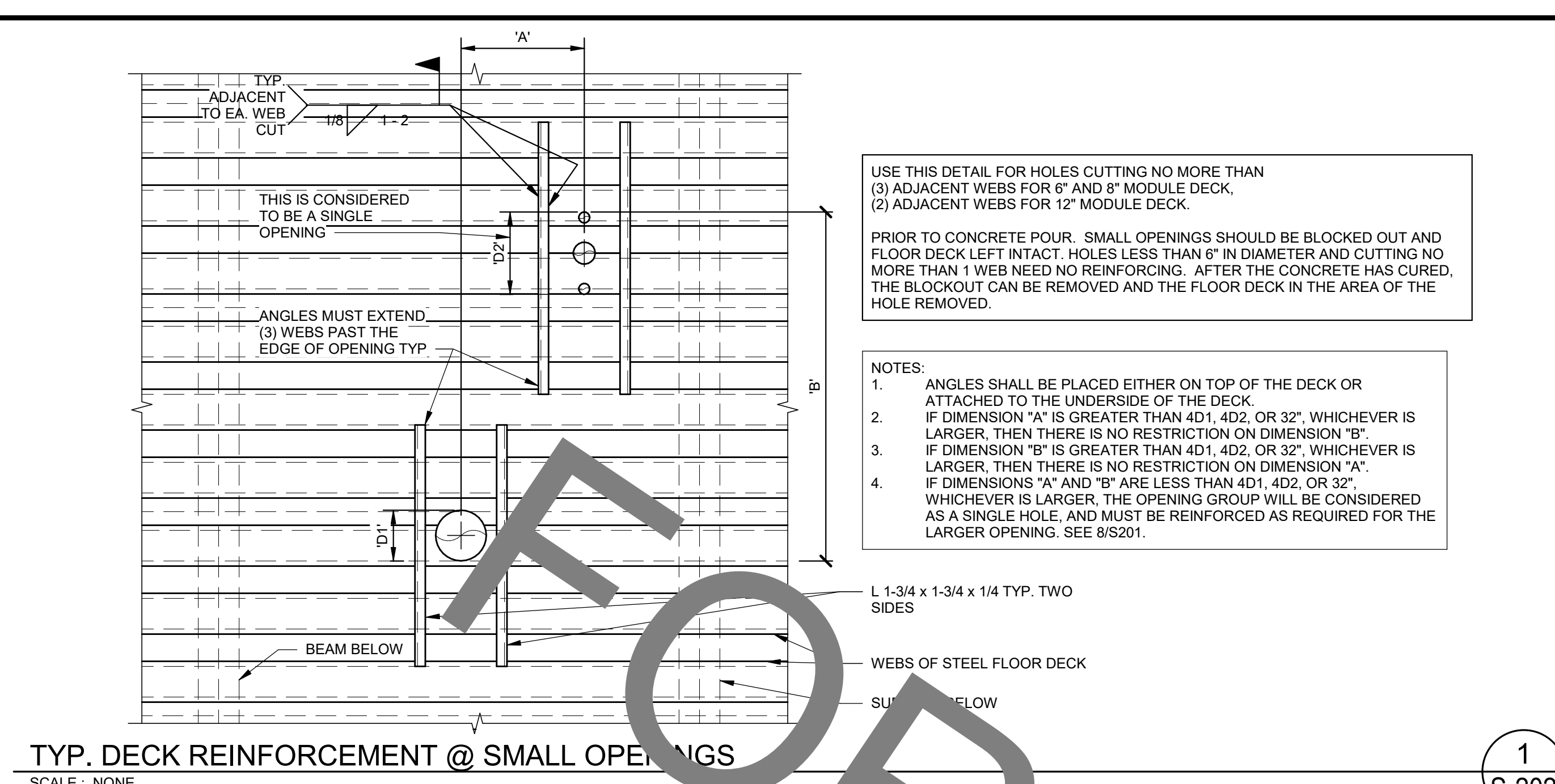
4 TYP. 'DLH' JOIST BEARING SEAT U.N.O. SCALE: NONE



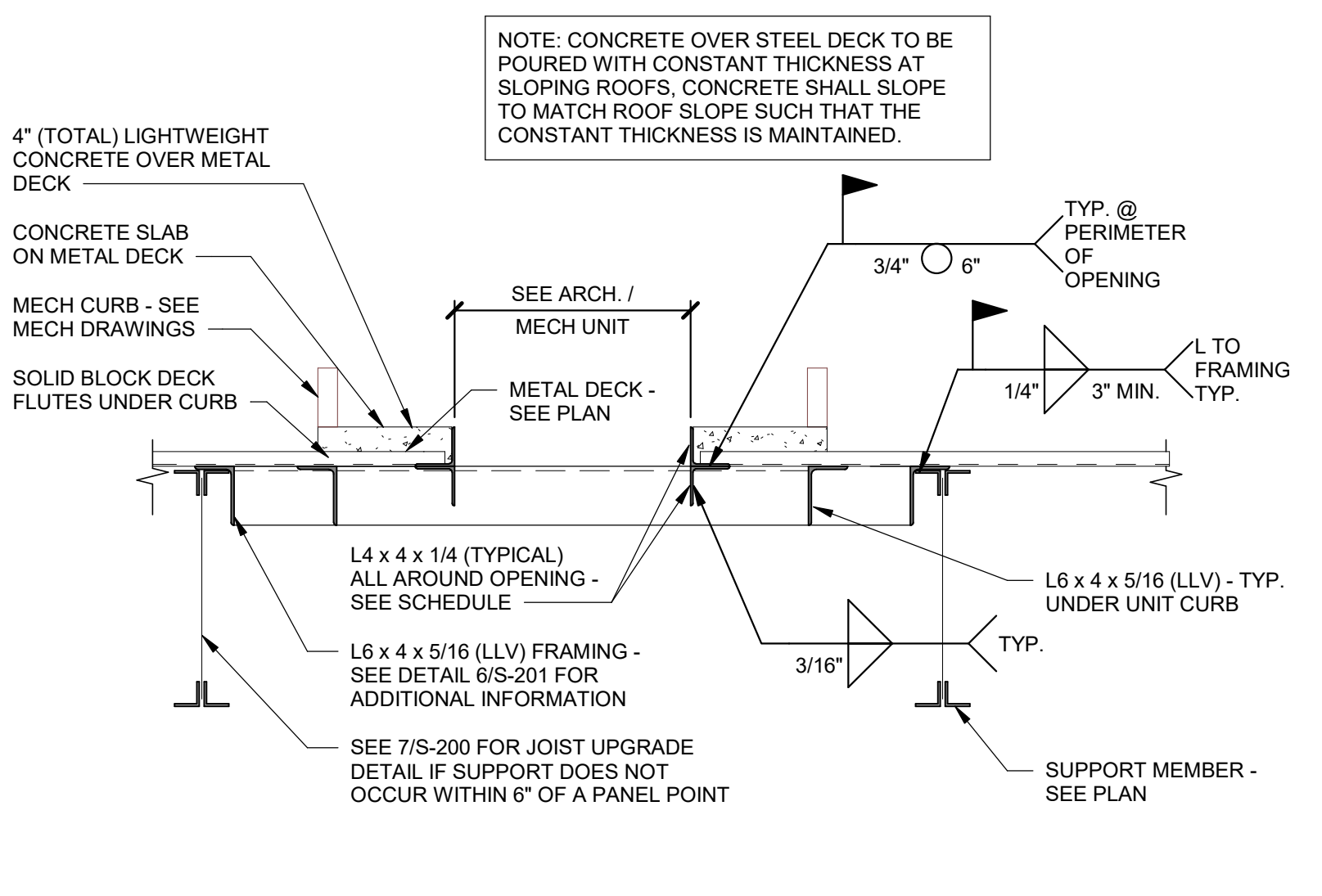
3 TYPICAL 'LH' JOIST BEARING SEAT (U.N.O.) SCALE: NONE



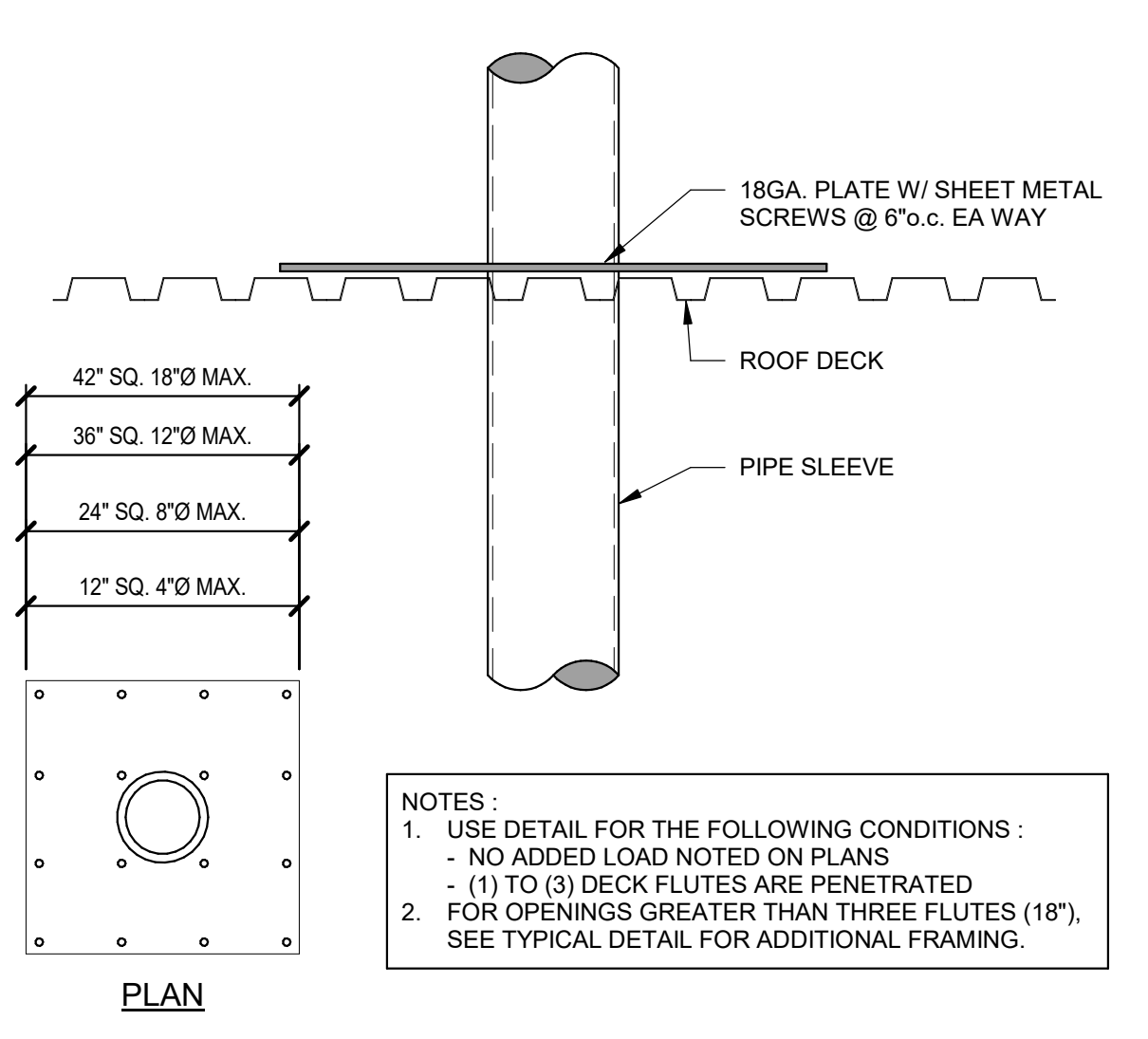
2 TYP. 'K' SERIES JOIST BEARING SEAT (U.N.O.) SCALE: NONE



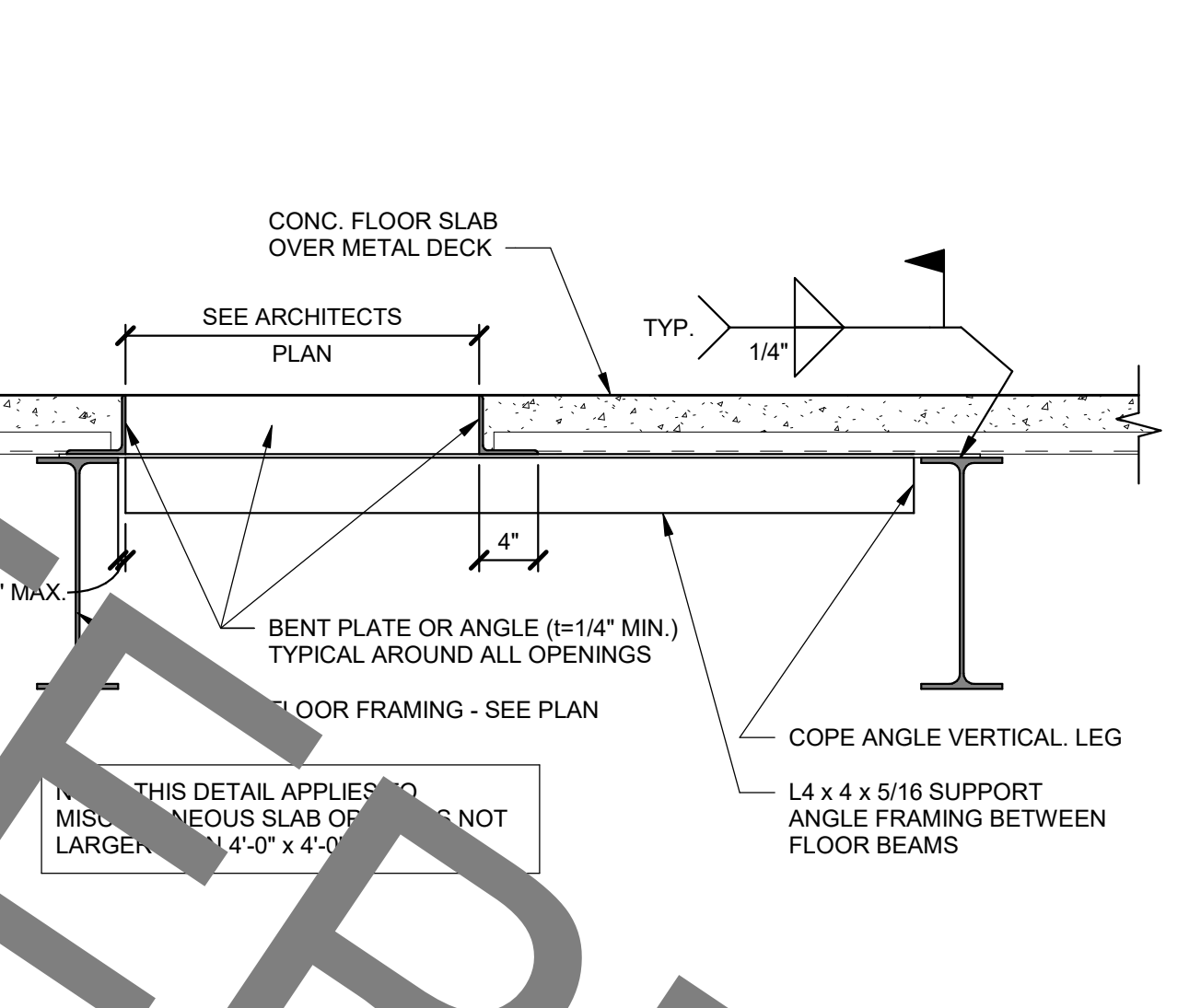
1 TYP. DECK REINFORCEMENT @ SMALL OPENINGS SCALE: NONE



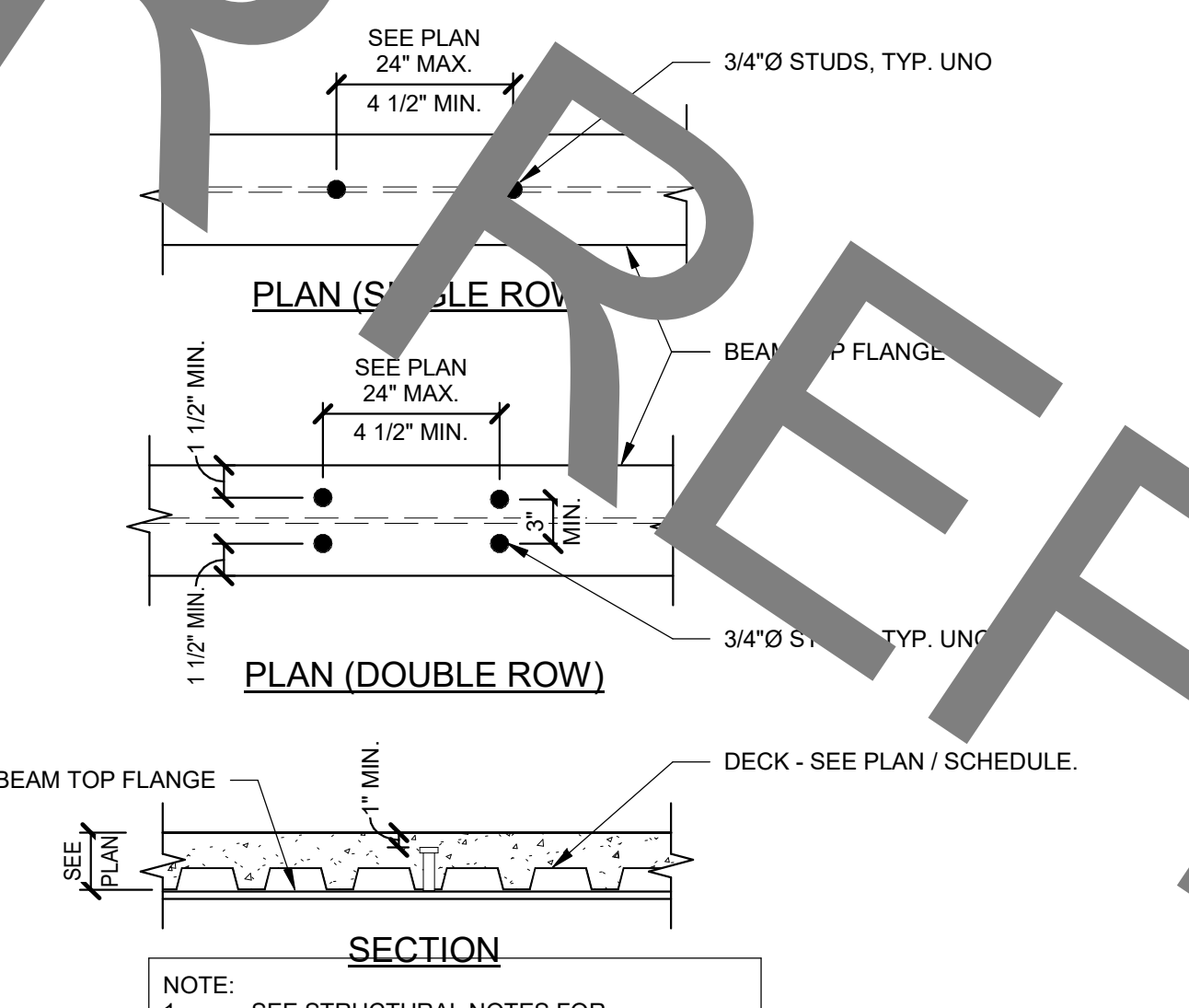
9 TYP. FRAMING @ ROOF OPN'GS & ROOFTOP EQUIPMENT DETAIL SCALE: NONE



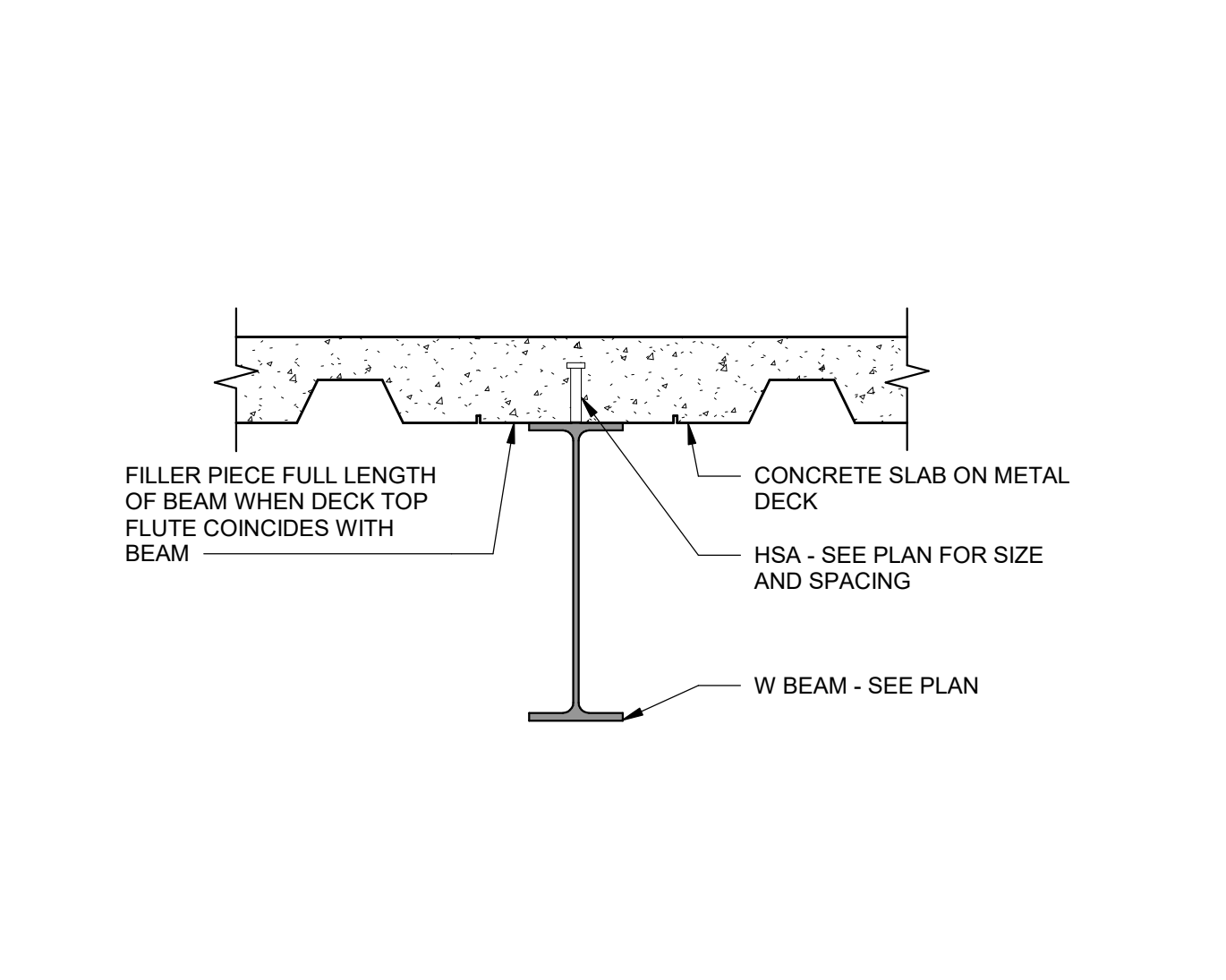
8 TYP. FRAMING @ SMALL ROOF OPENINGS DETAIL SCALE: NONE



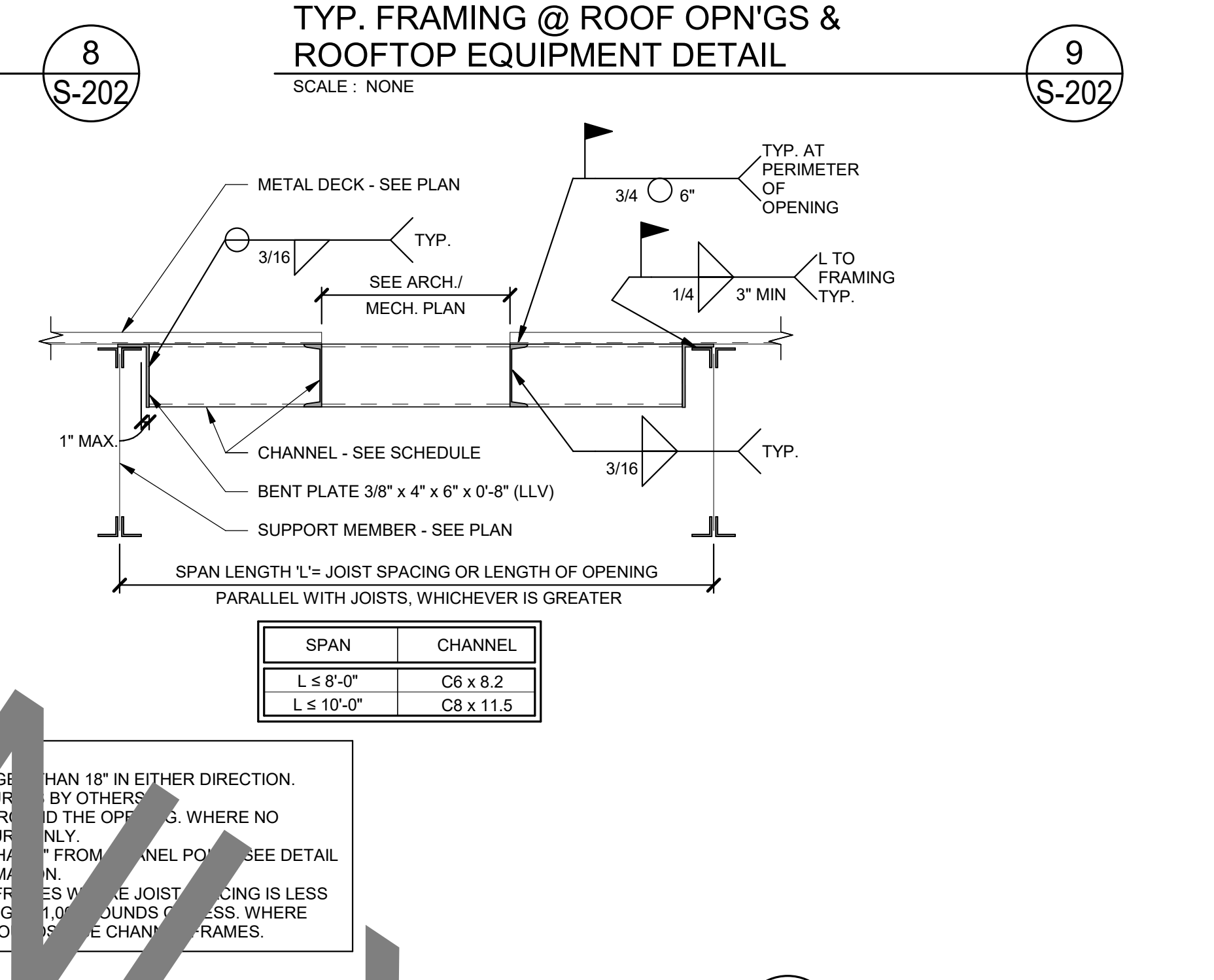
7 TYP. FRAMING @ LOOP OPENING SCALE: NONE



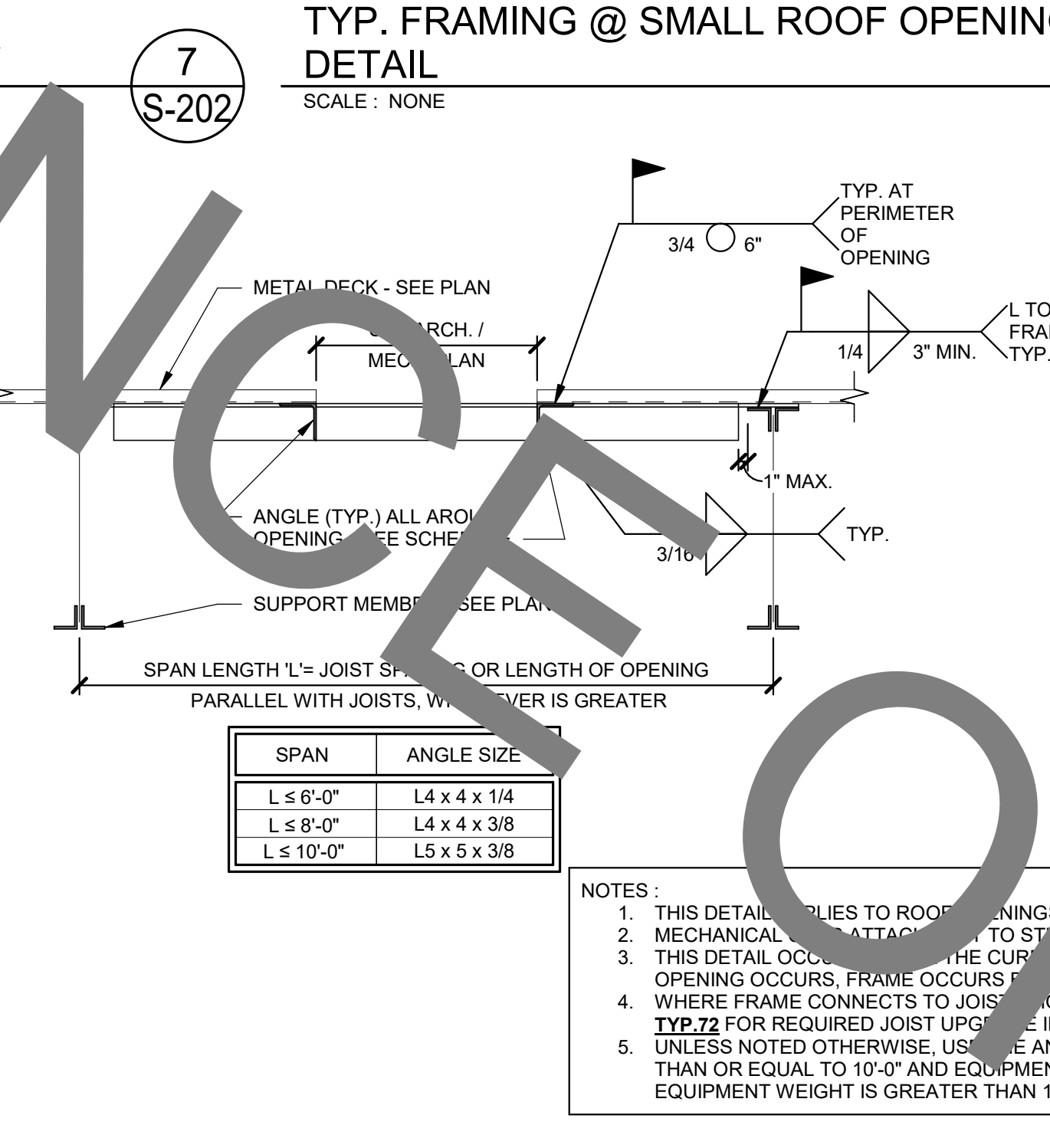
6 TYP. SHEAR STUD CONNECTION DETAIL SCALE: NONE



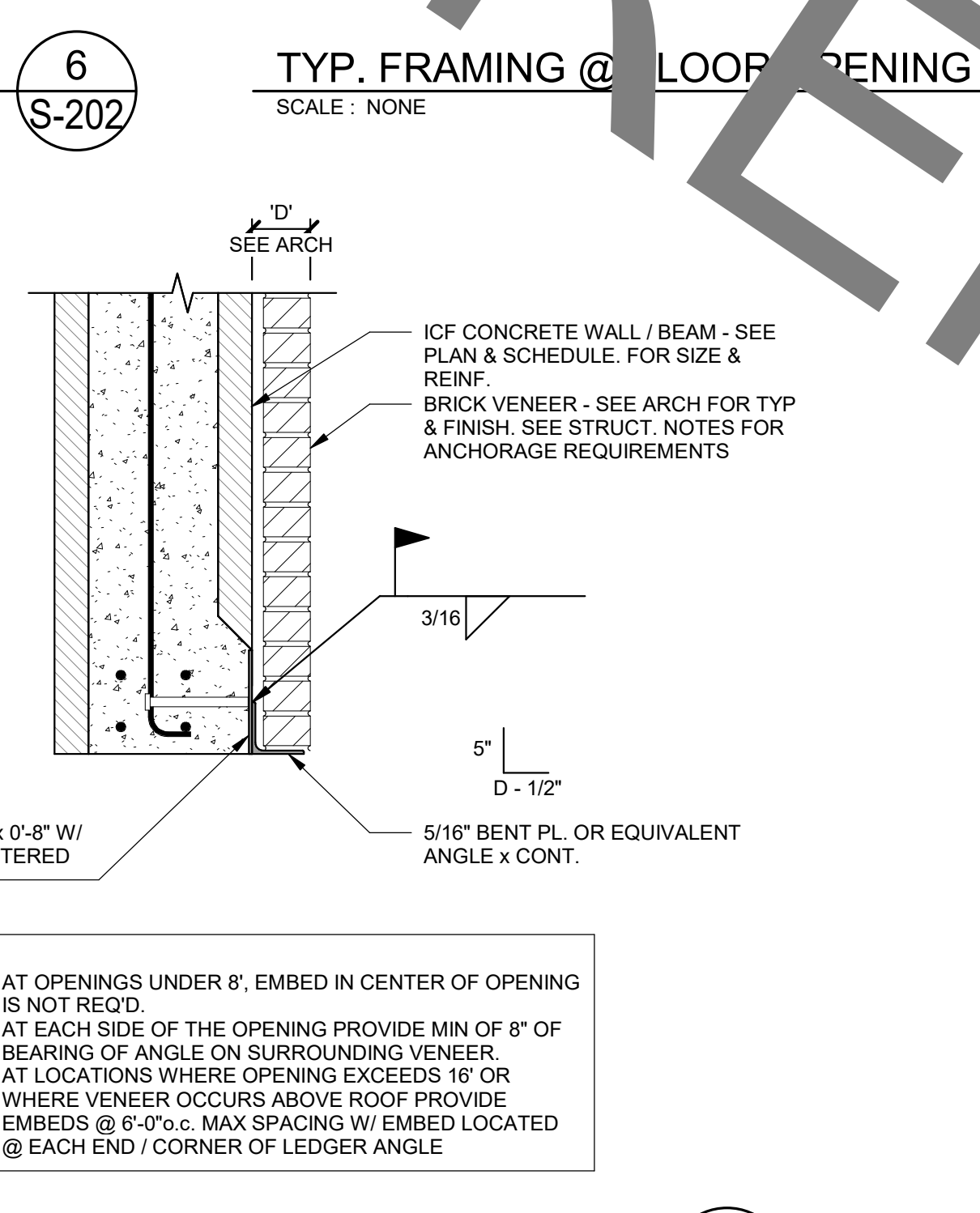
5 TYP. FORMLOK COMPOSITE FLOOR DECK SCALE: NONE



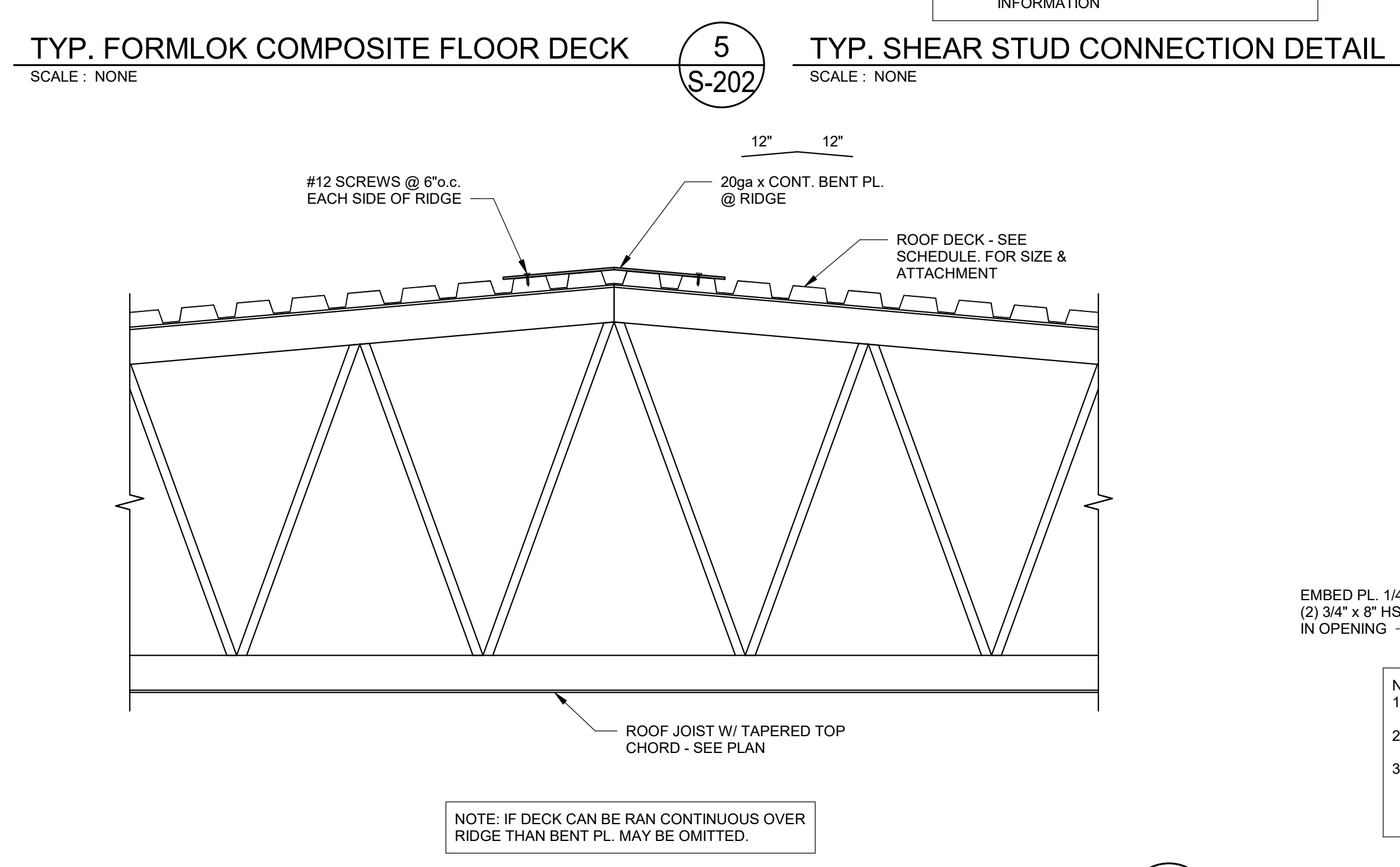
12 TYP. FRAMING @ ROOF OPENINGS & ROOFTOP EQUIPMENT DETAIL SCALE: NONE



11 TYP. FRAMING @ ROOF OPENINGS & ROOFTOP EQUIPMENT DETAIL SCALE: NONE



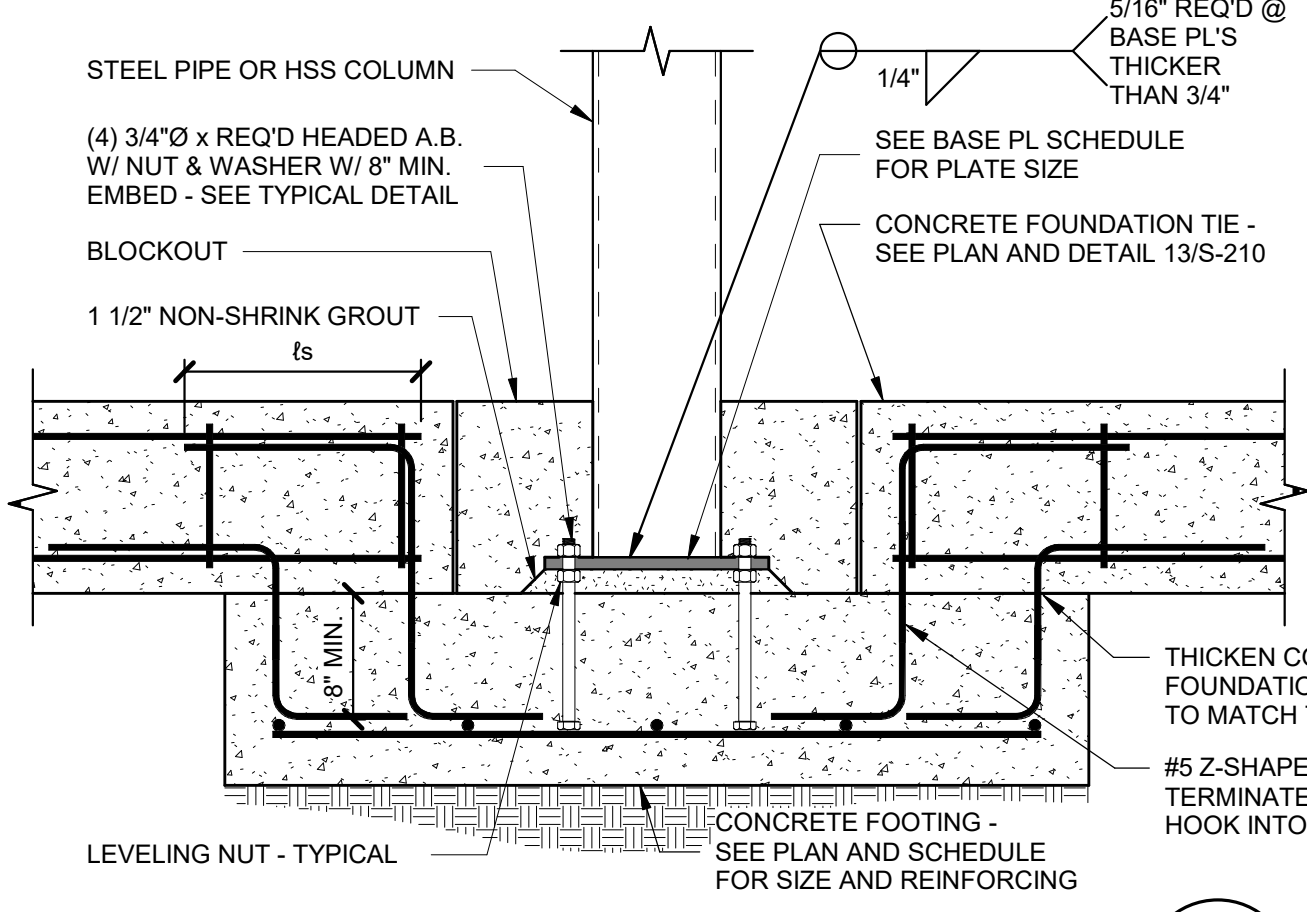
10 TYP. BRICK VENEER LINTEL DETAIL SCALE: NONE



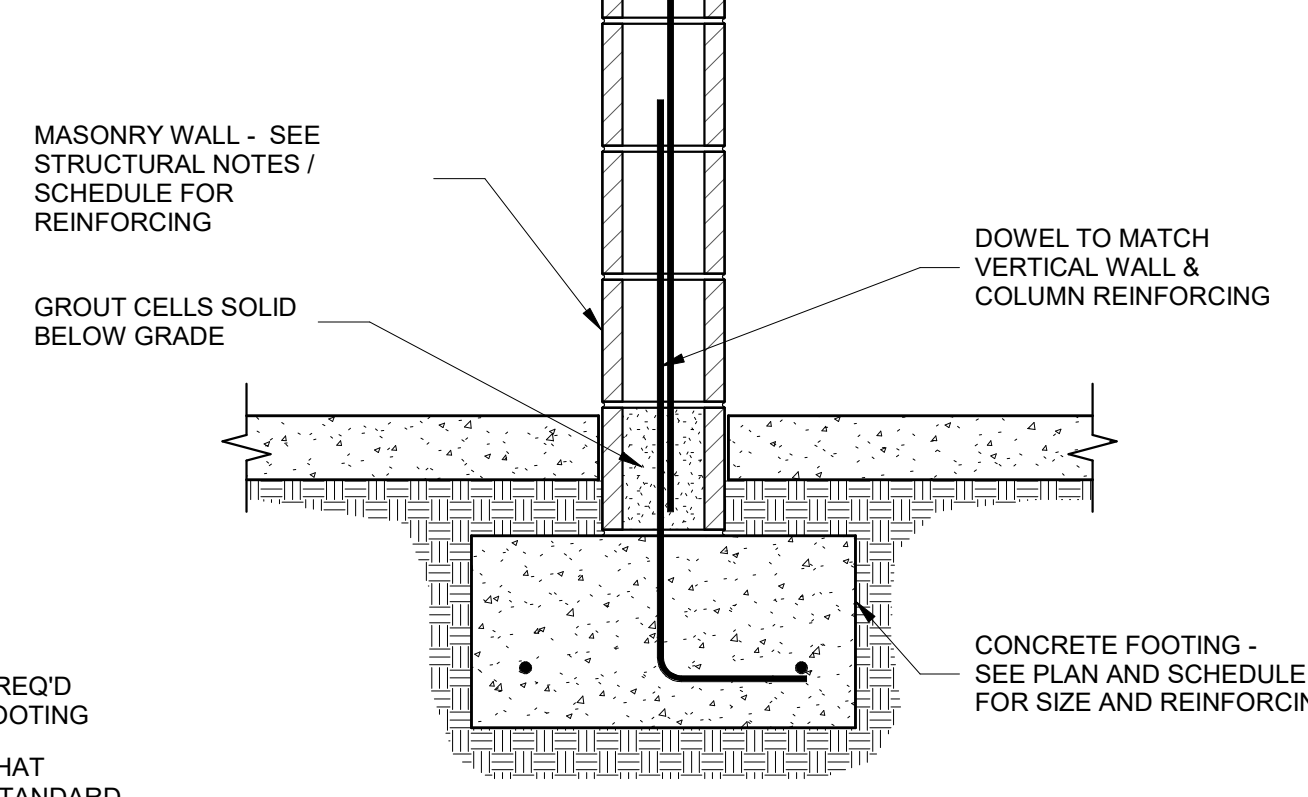
DETAIL SCALE: NONE



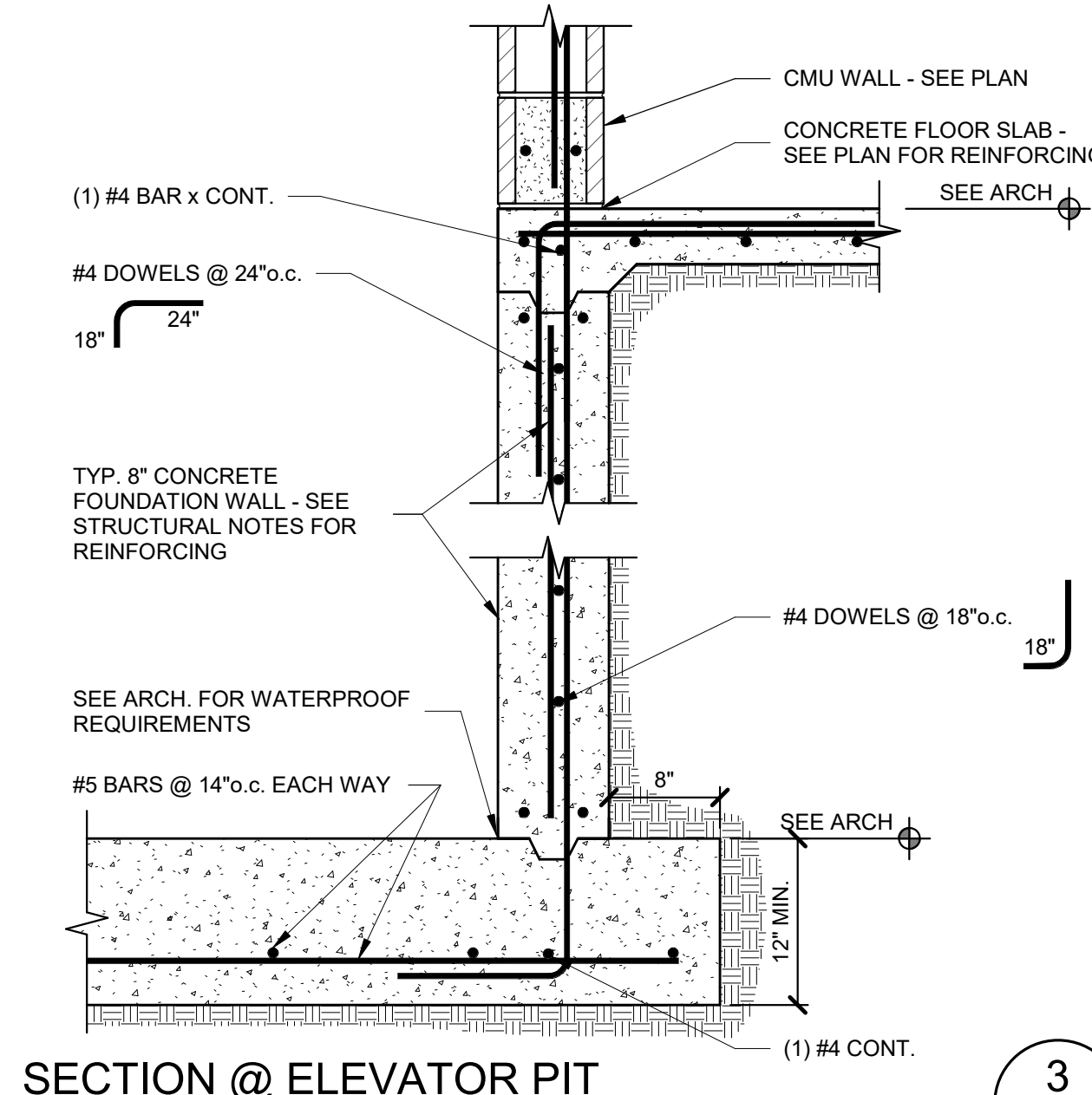
BASE PLATE SCHEDULE	
COLUMN SIZE	BASE PLATE SIZE
8" SQ, 10" Ø	PL 3/4" x 14" x 1'-2"
7" SQ	PL 3/4" x 13" x 1'-1"
6" SQ, 8" Ø	PL 3/4" x 12" x 1'-0"
5" SQ	PL 3/4" x 12" x 1'-0"
4" SQ	PL 5/8" x 10" x 0'-10"
3" Ø, 3" SQ	PL 1/2" x 9" x 0'-9"



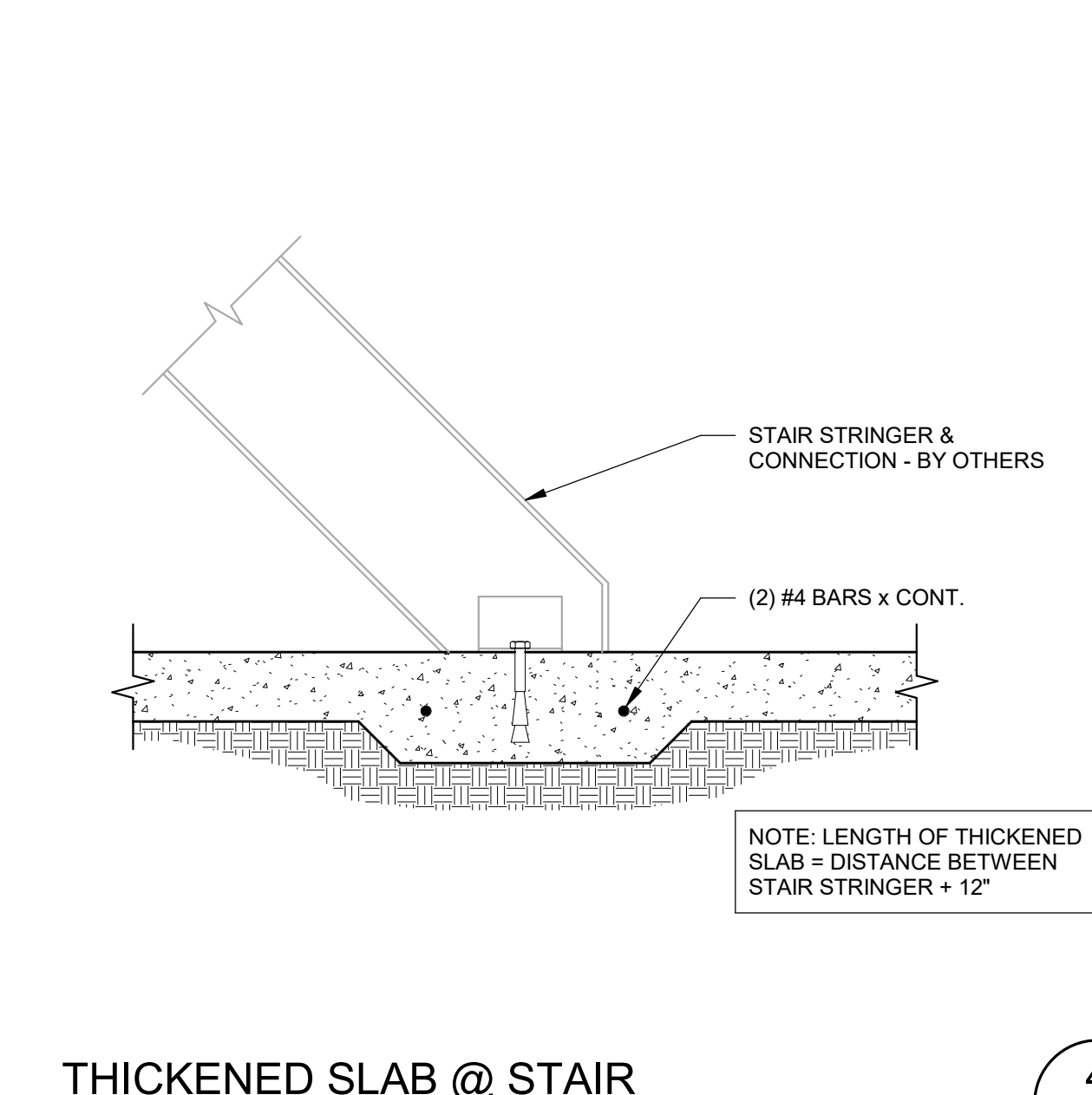
**1** COLUMN TO SPOT FOOTING  
SCALE: NONE



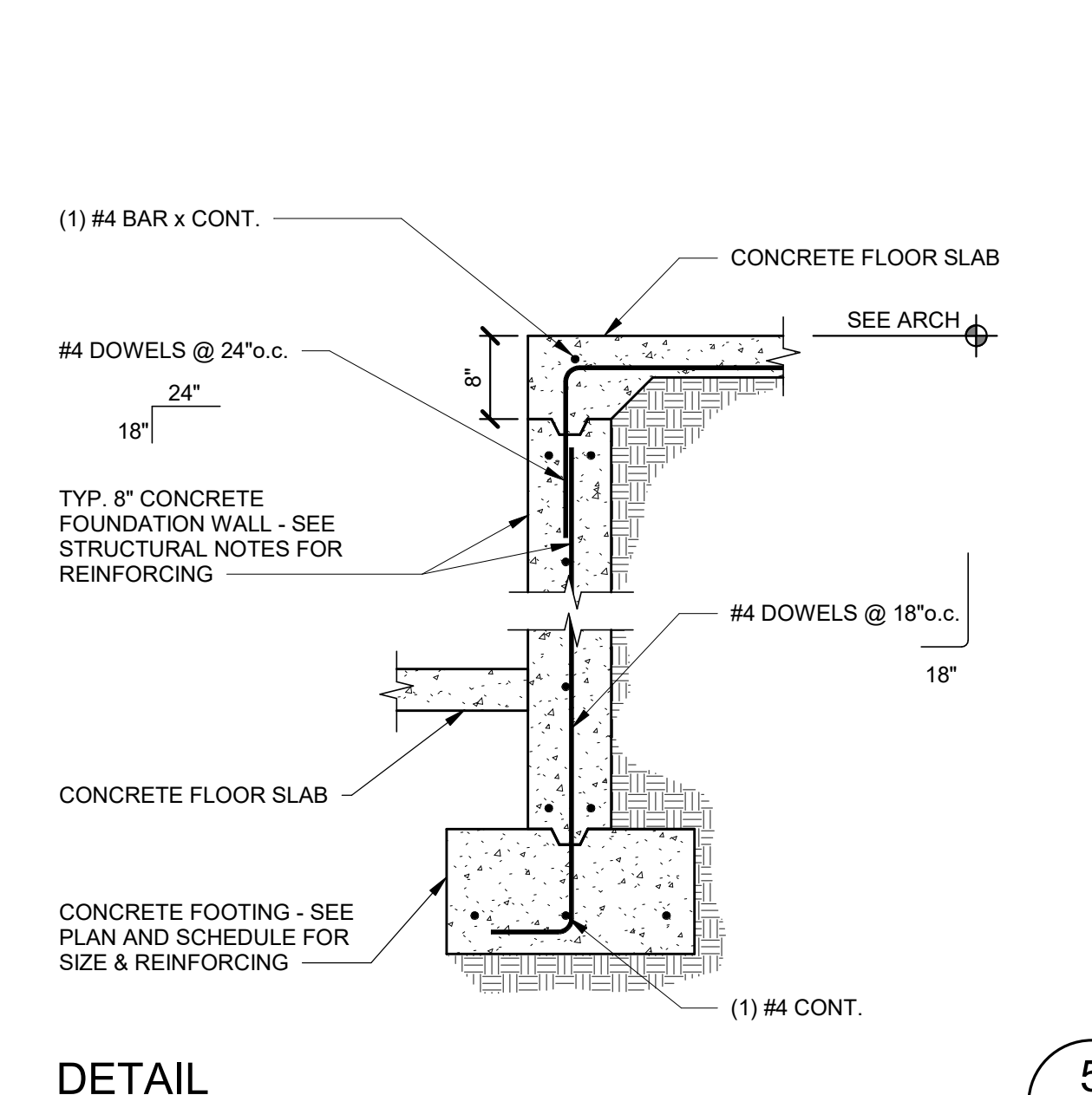
**2** MASONRY WALL ON FOOTING  
SCALE: NONE



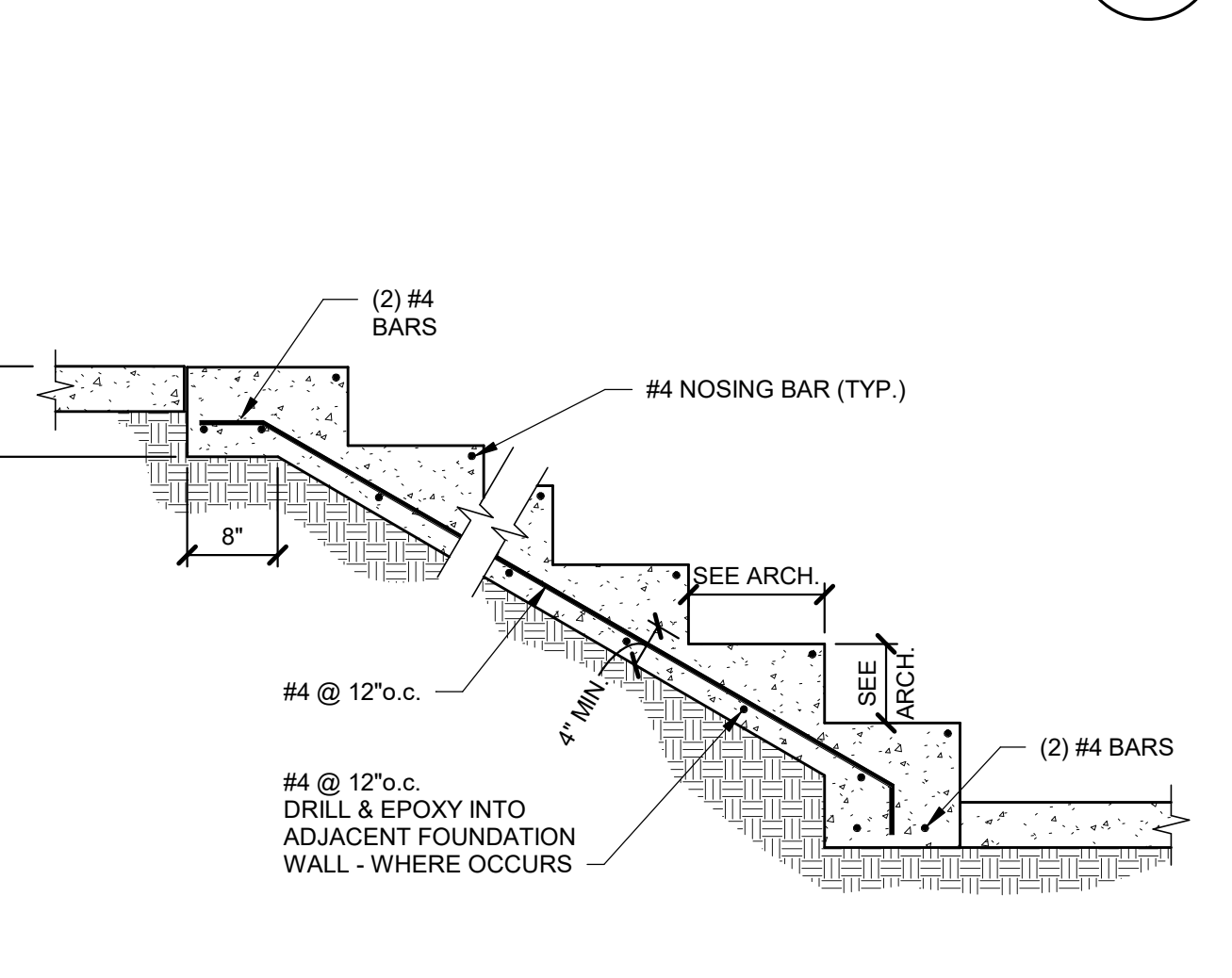
**3** SECTION @ ELEVATOR PIT  
SCALE: NONE



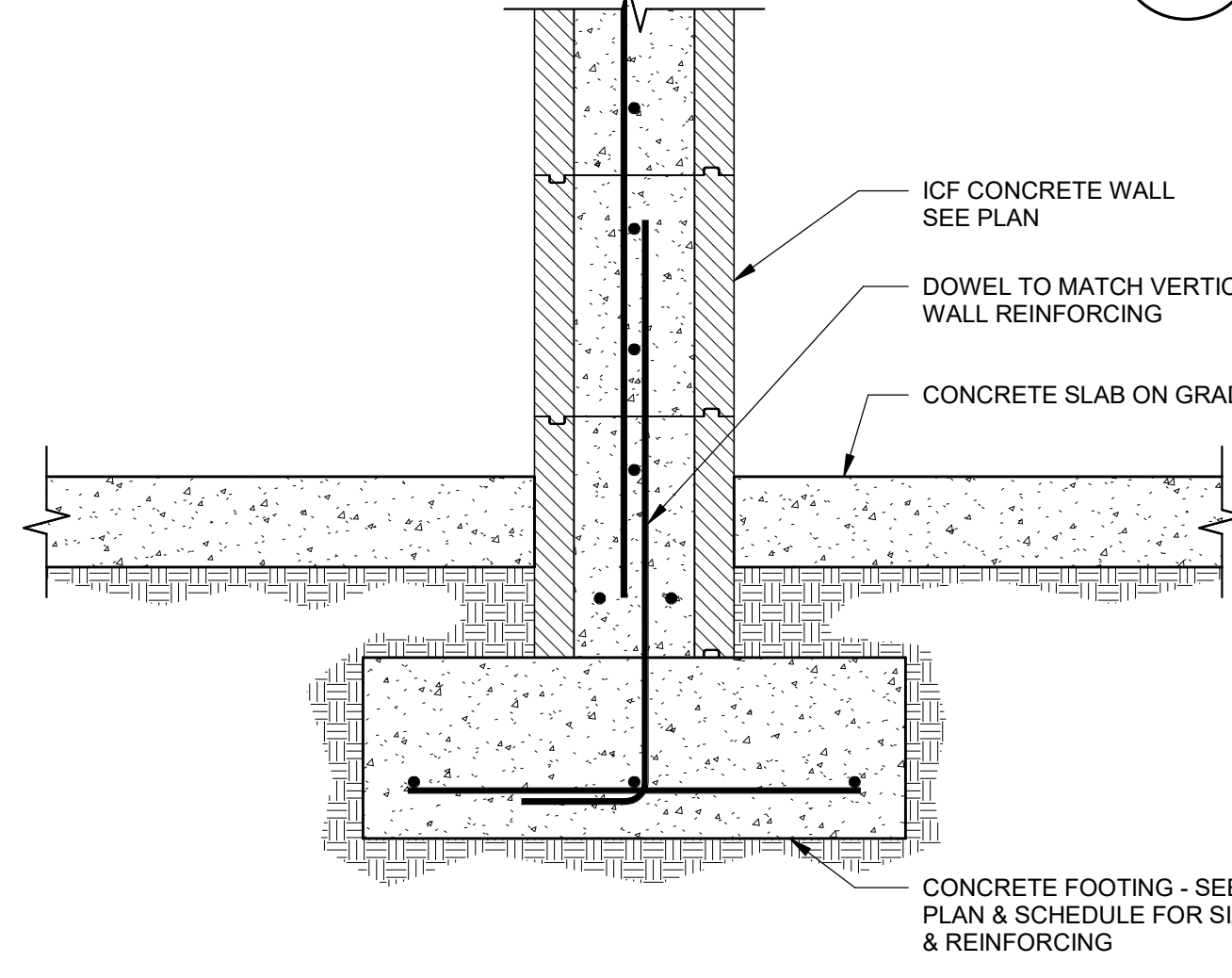
**4** THICKENED SLAB @ STAIR  
SCALE: NONE



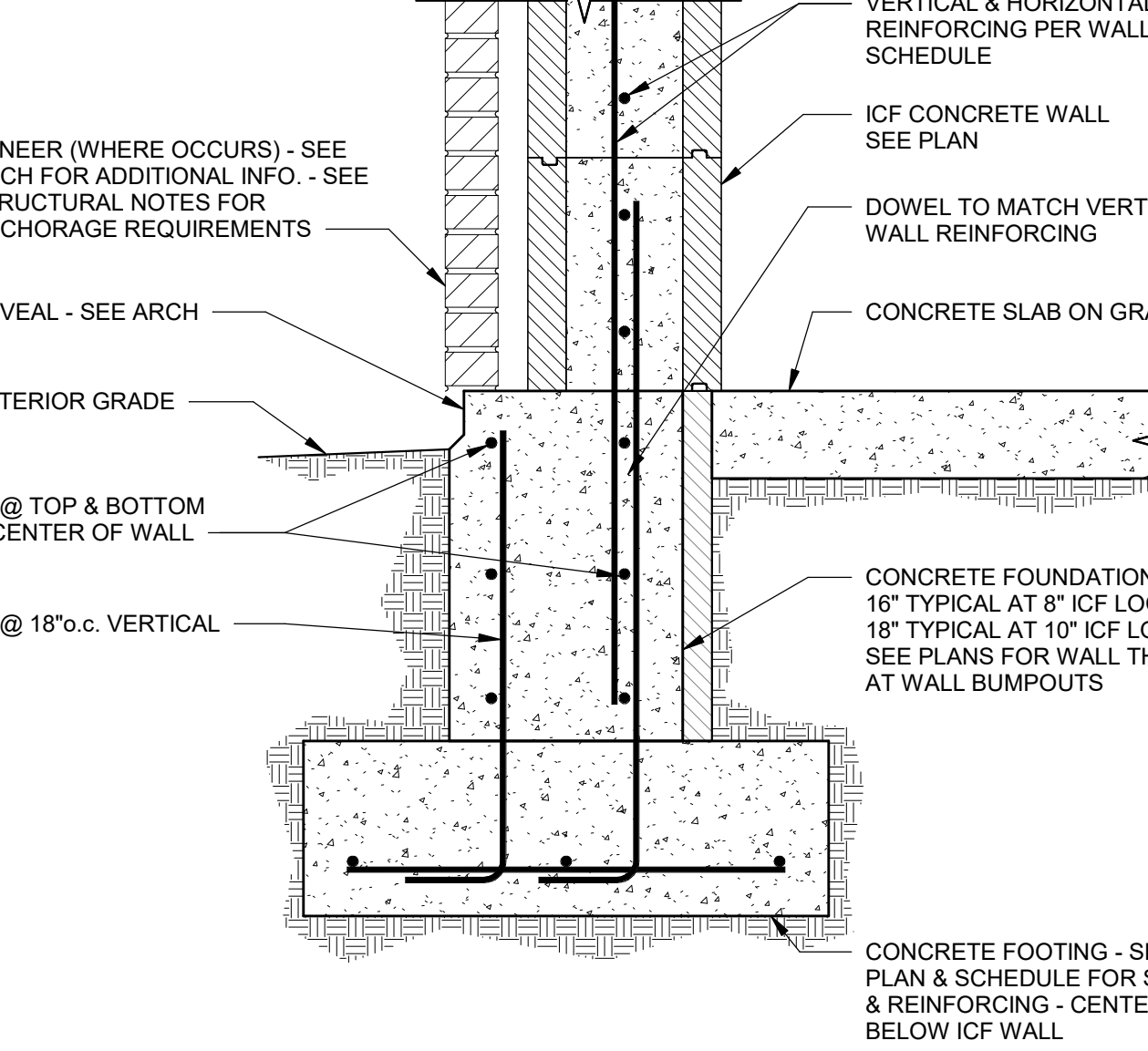
**5** DETAIL  
SCALE: NONE



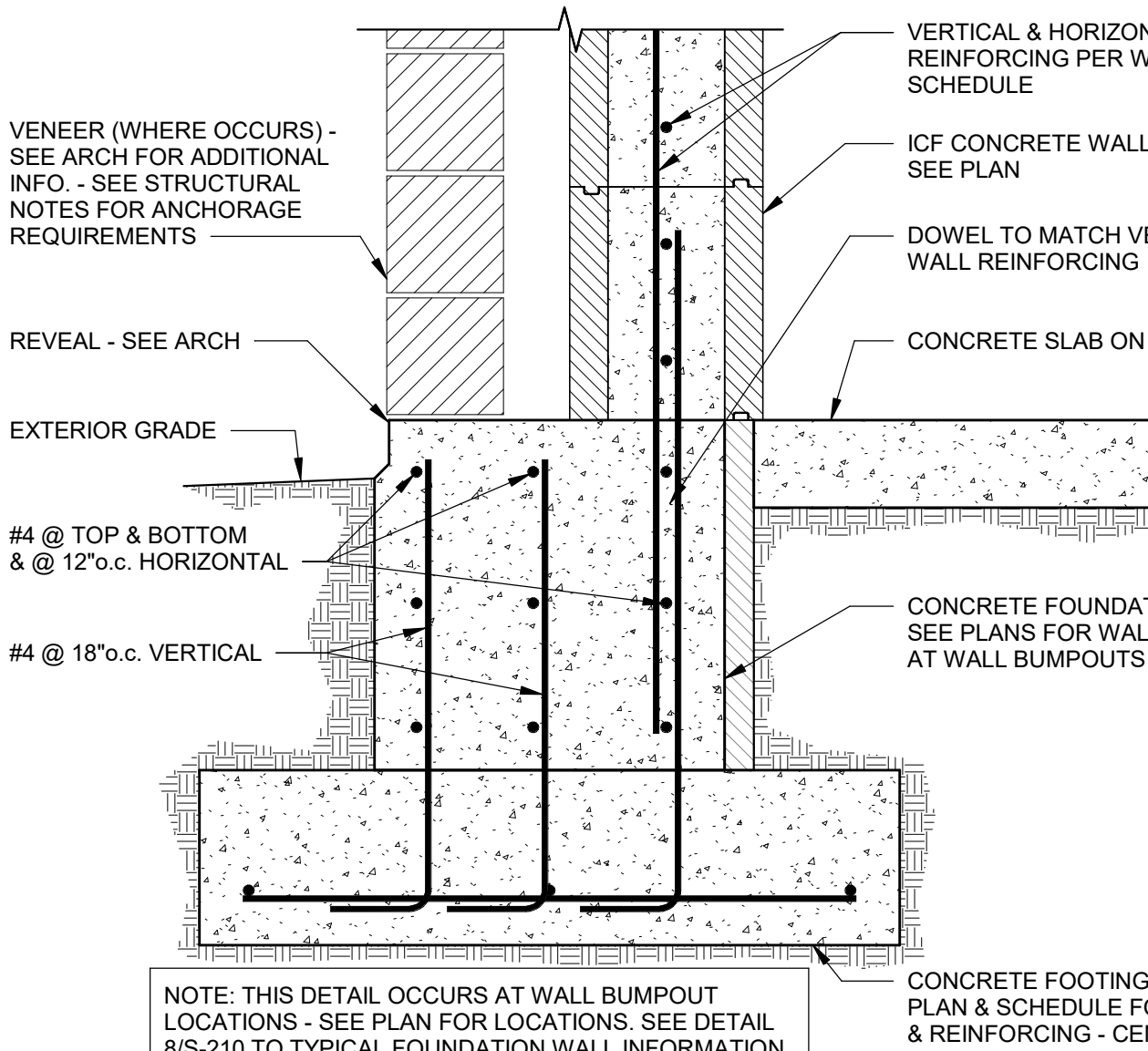
**6** TYPICAL STEPPED STAIR FOOTING  
SCALE: NONE



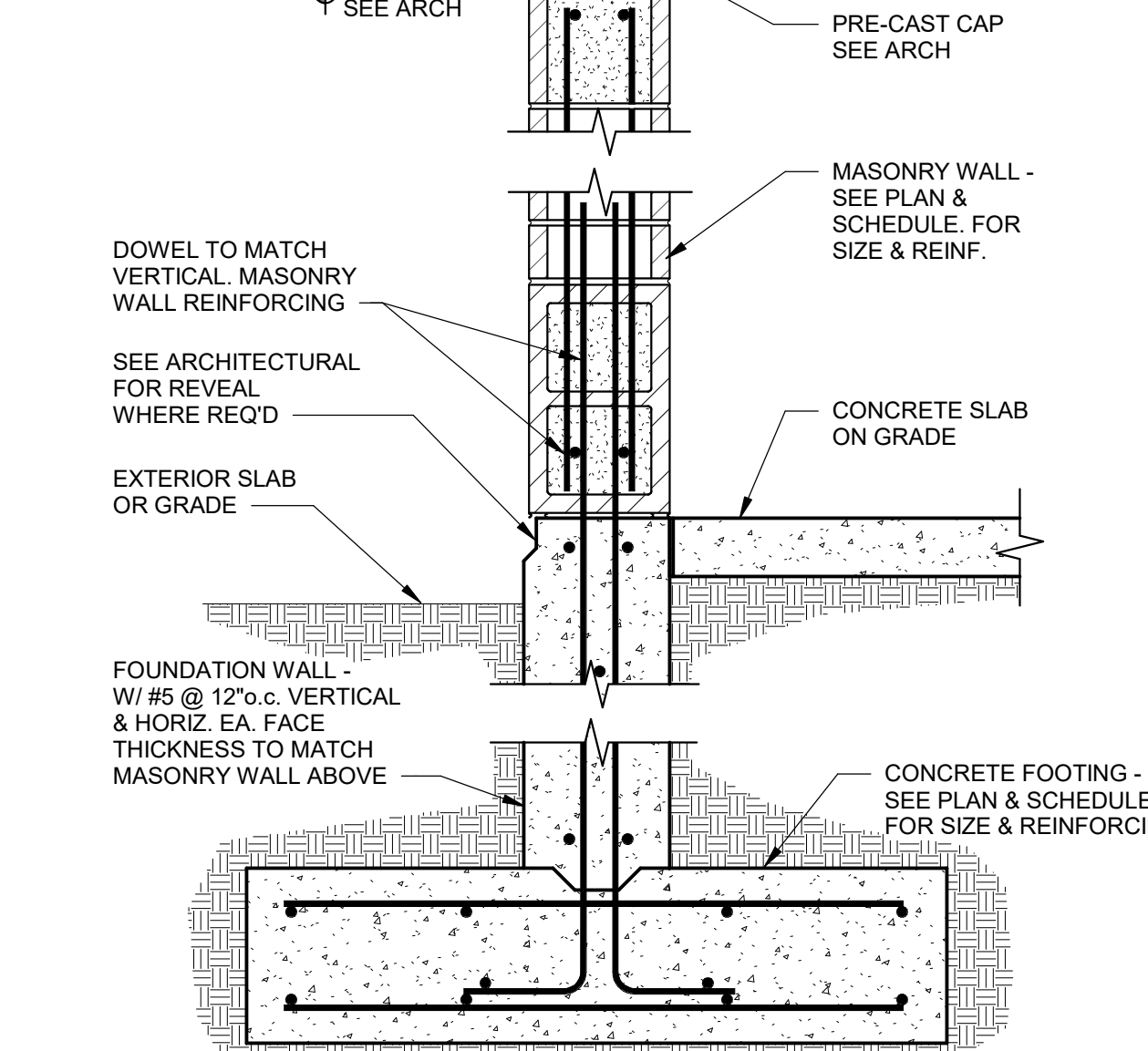
**7** DETAIL  
SCALE: NONE



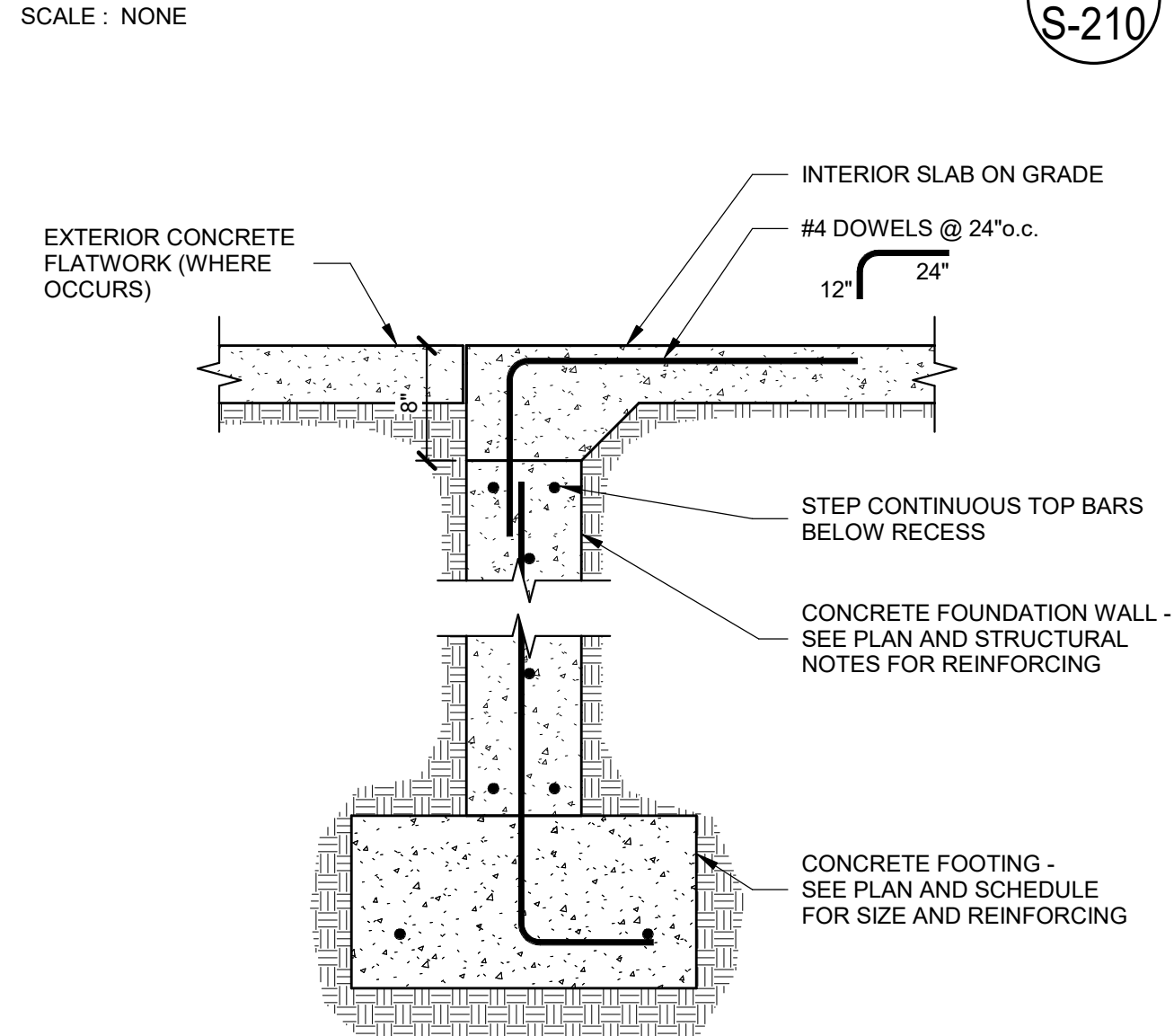
**8** DETAIL  
SCALE: NONE



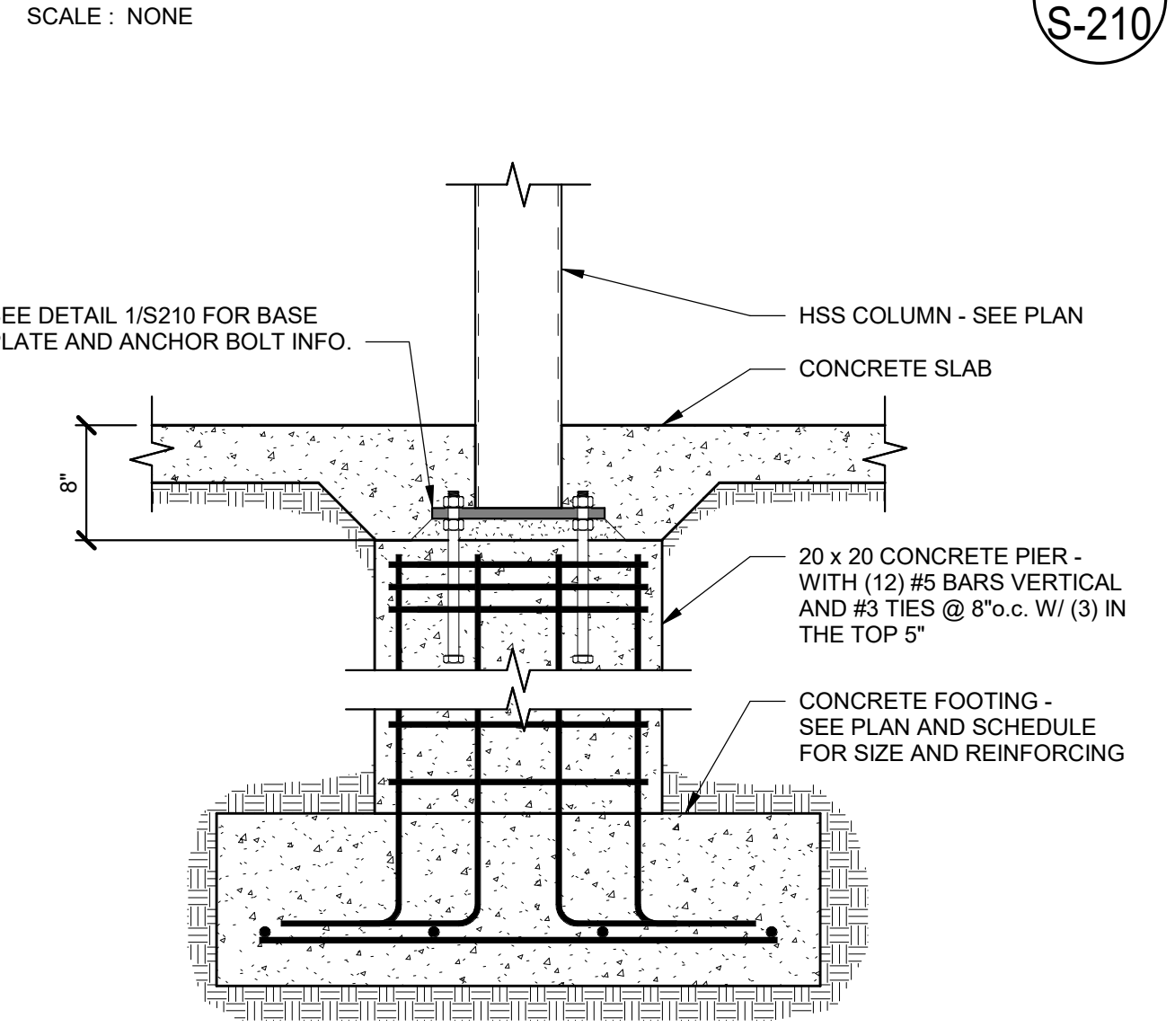
**9** DETAIL  
SCALE: NONE



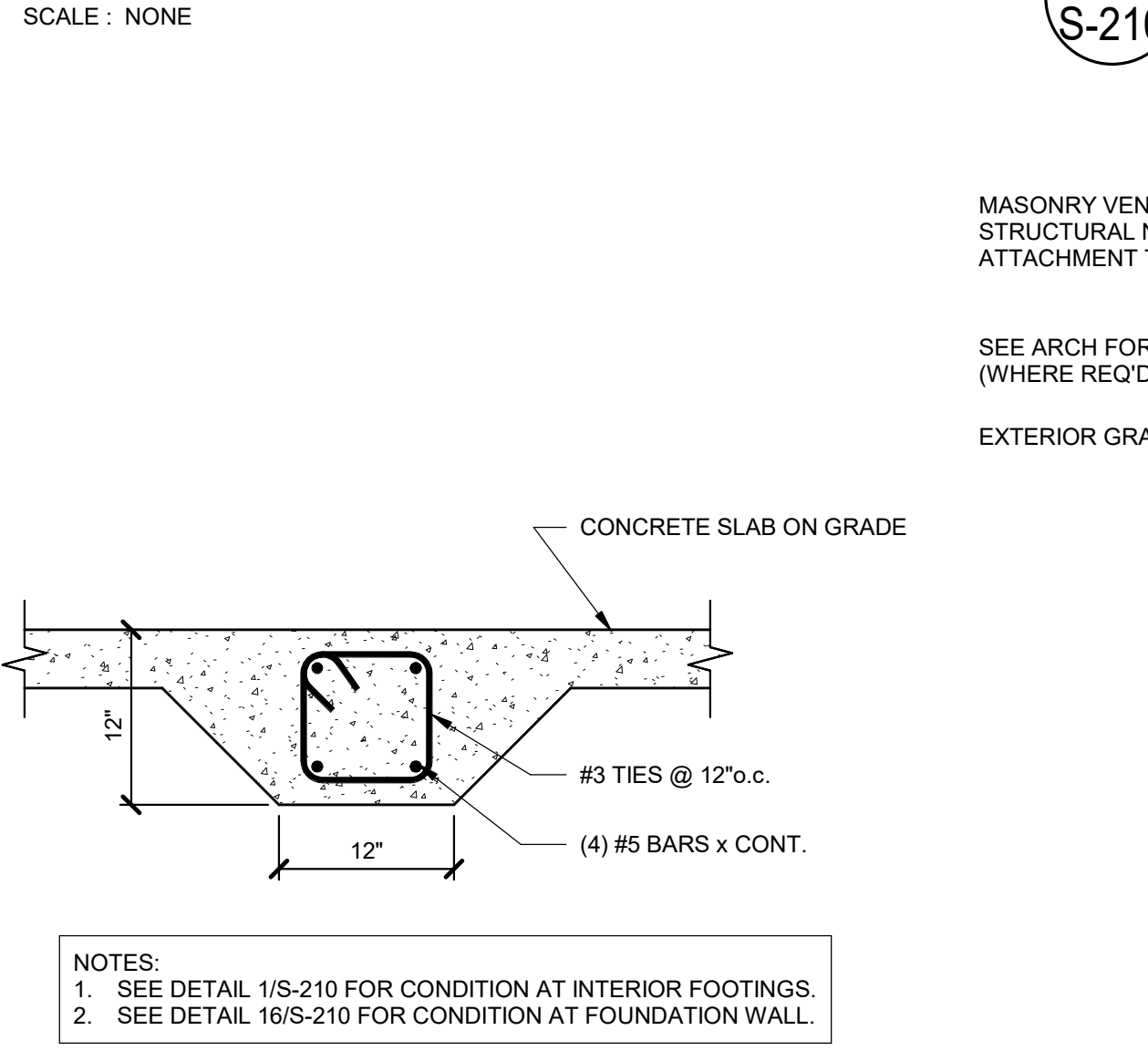
**10** DETAIL  
SCALE: NONE



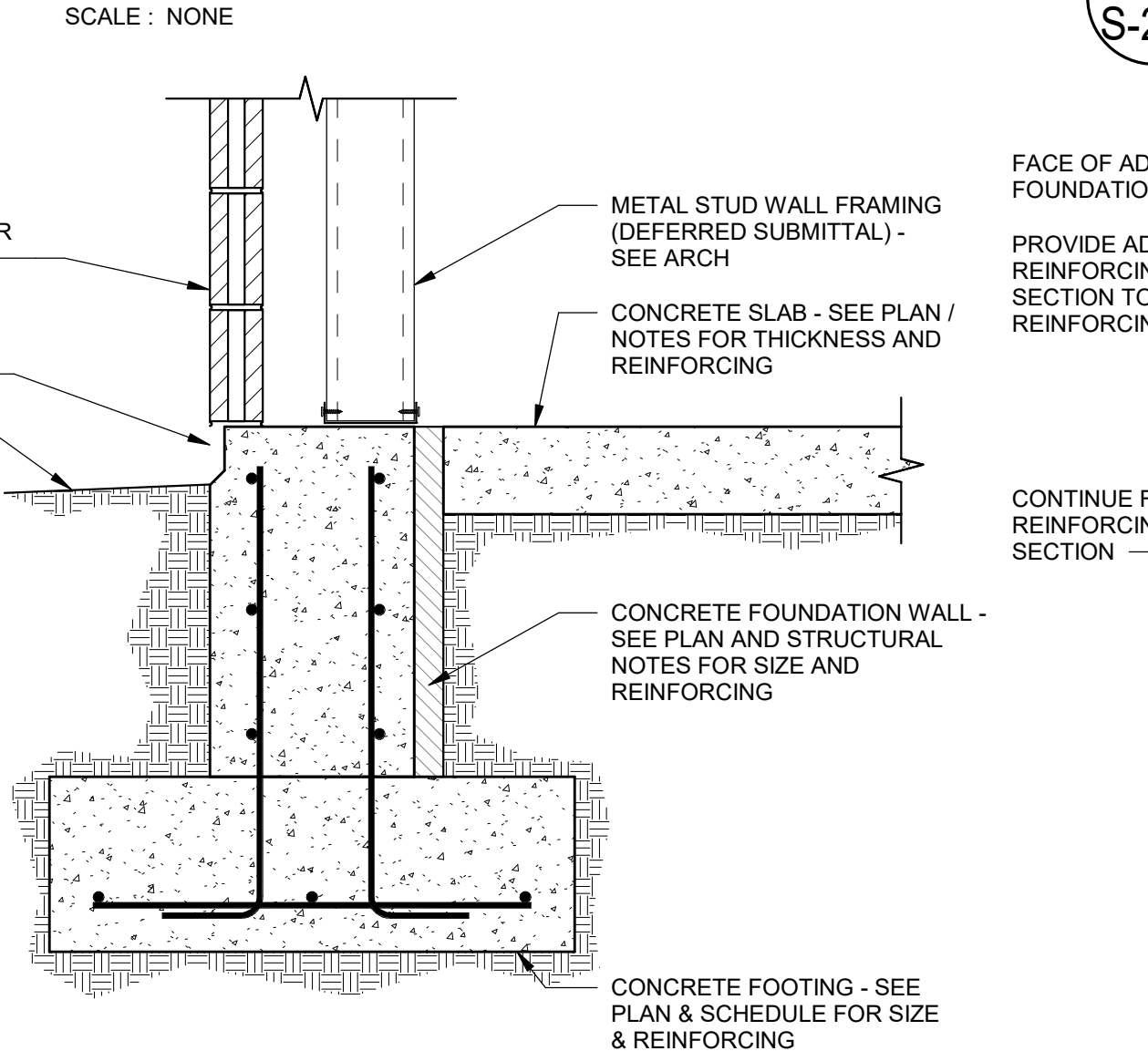
**11** CONCRETE FOUNDATION @ OPENING  
SCALE: NONE



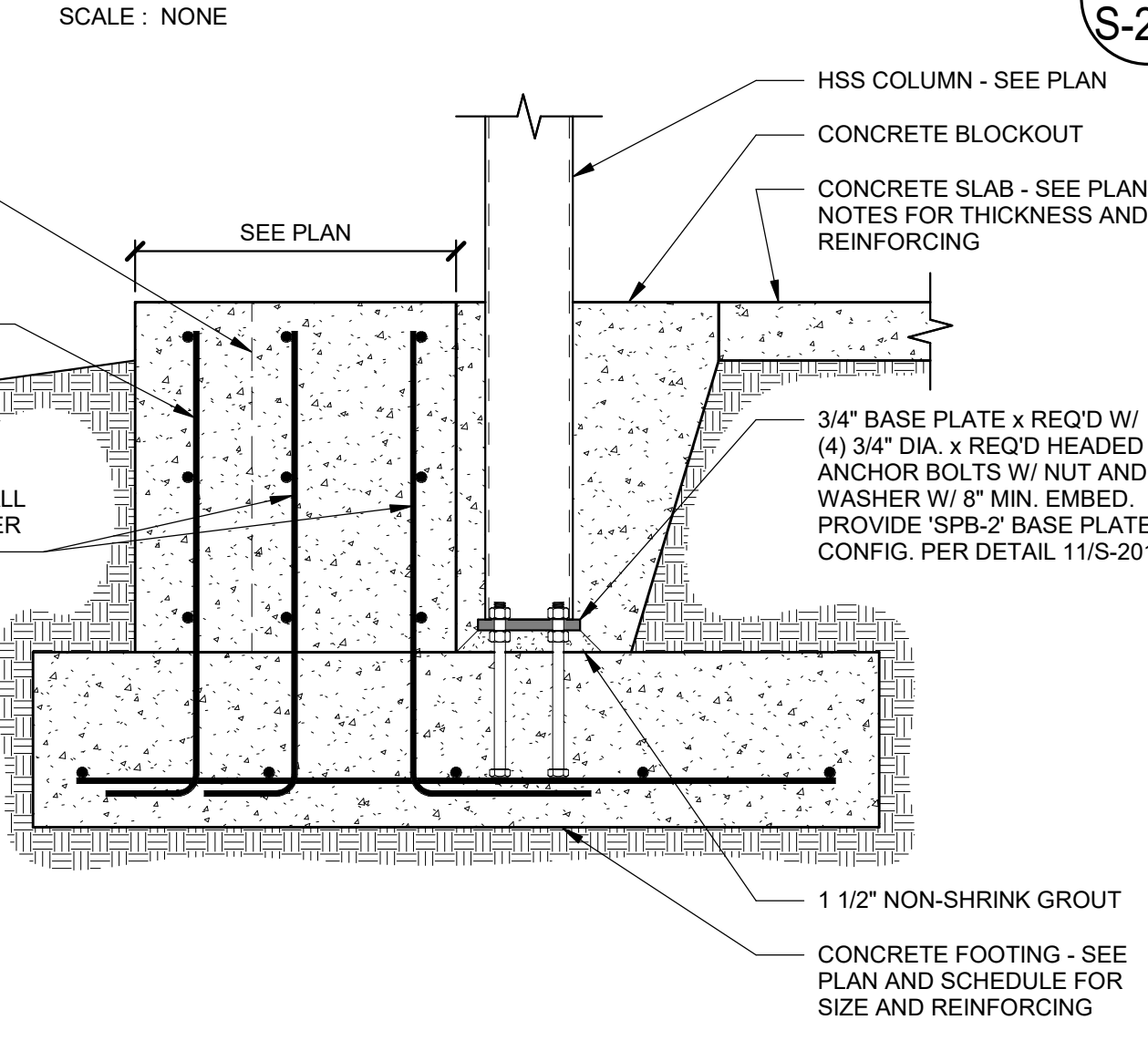
**12** CONCRETE PIER  
SCALE: NONE



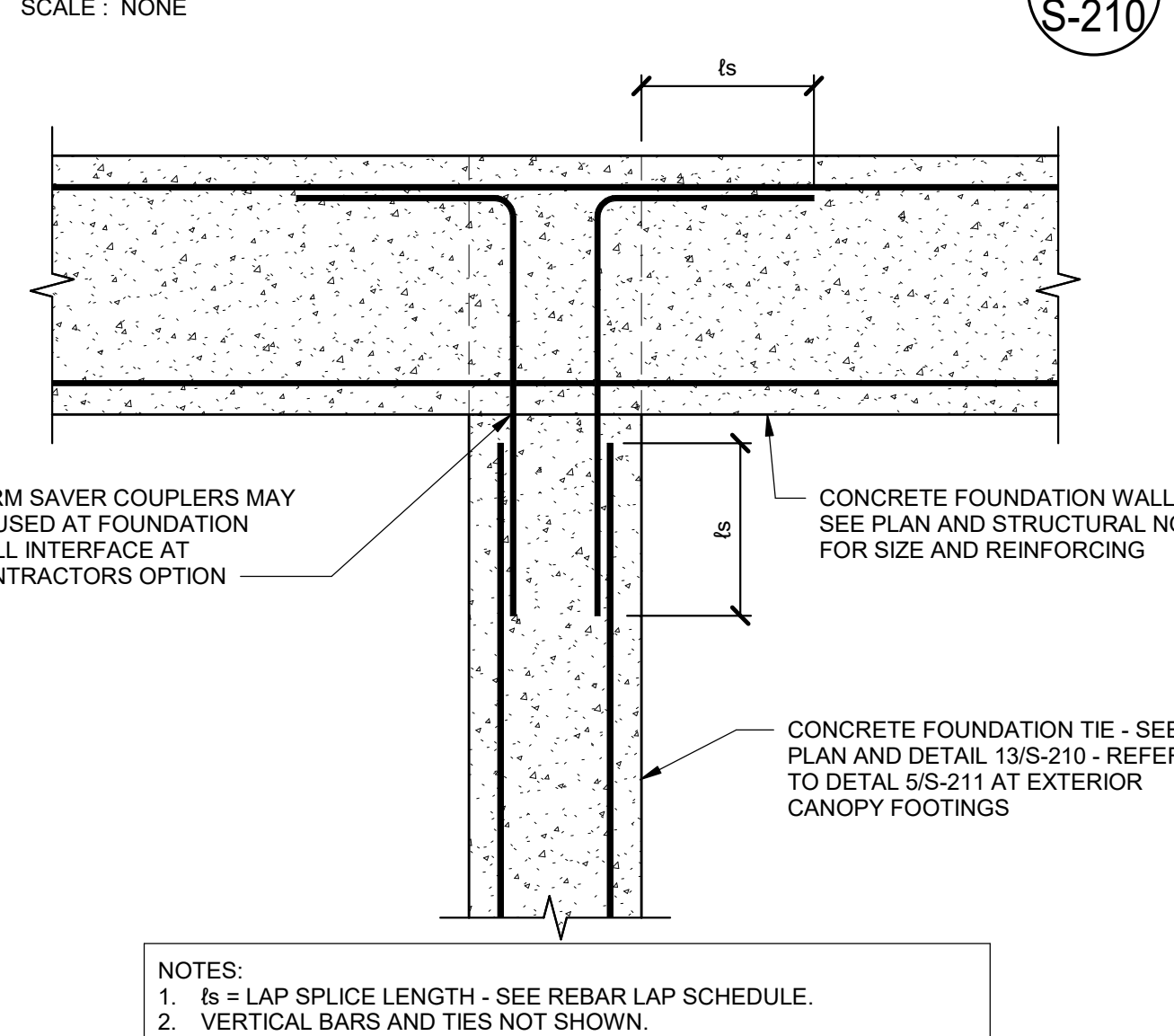
**13** DETAIL  
SCALE: NONE



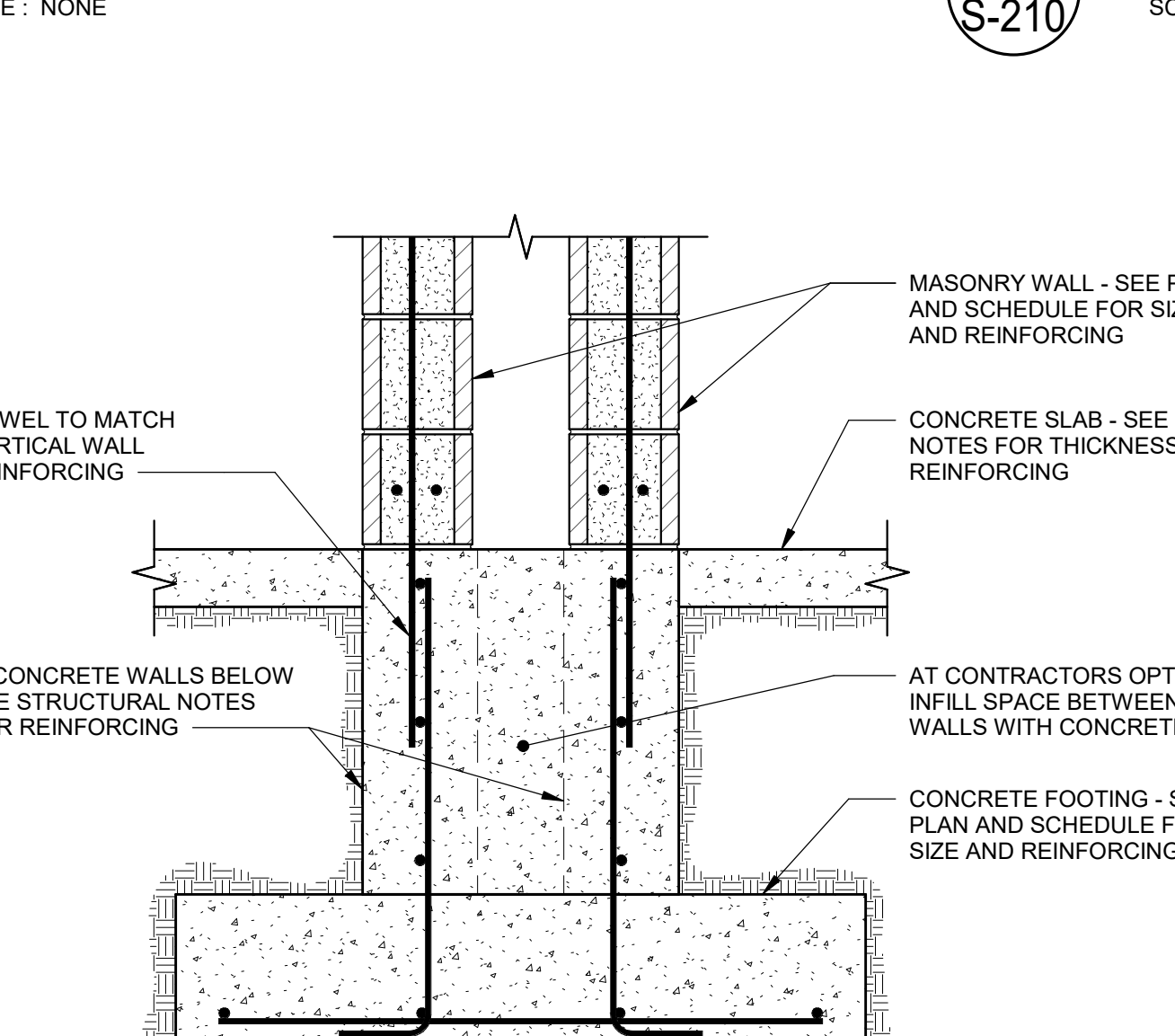
**14** DETAIL  
SCALE: NONE



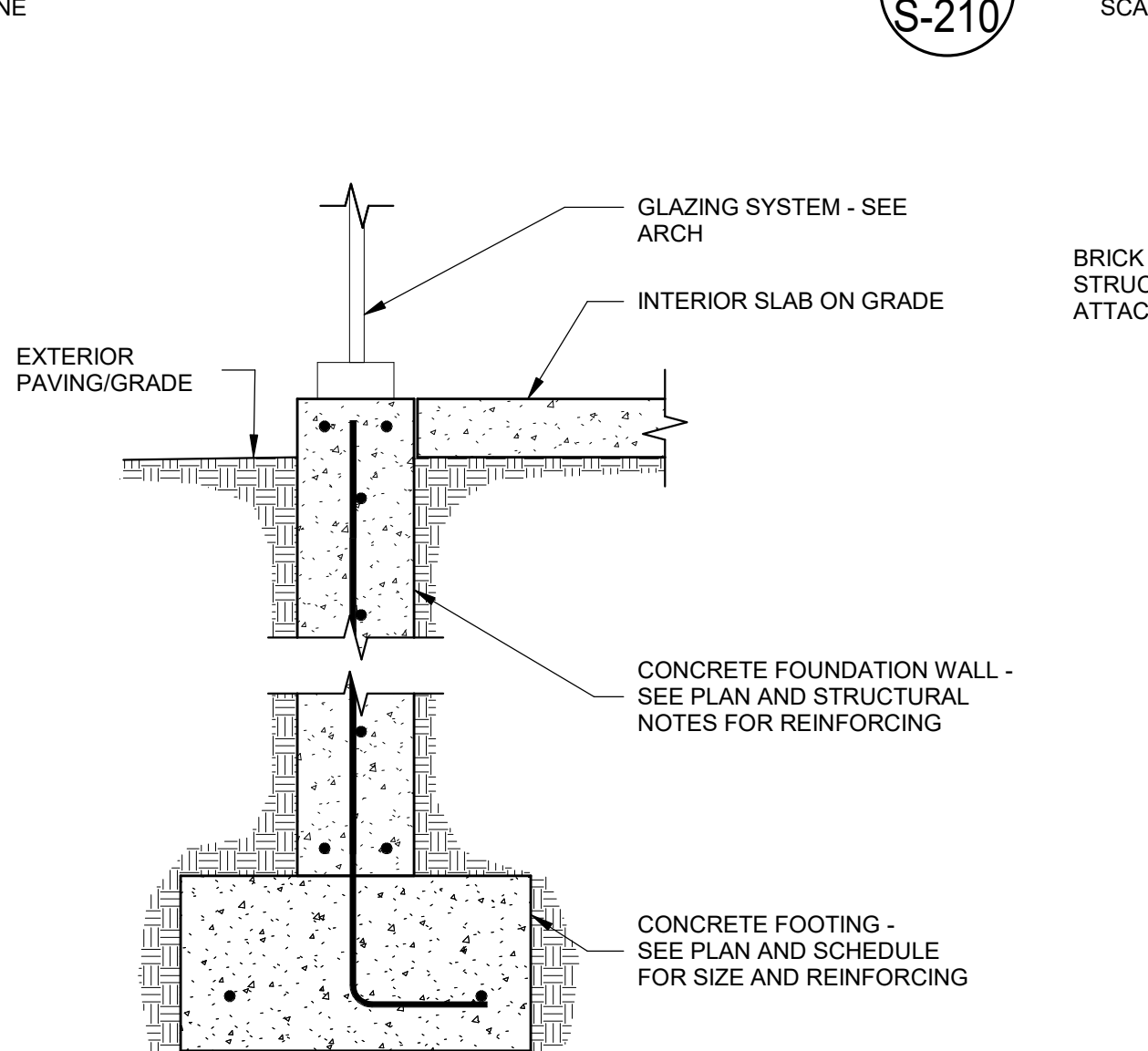
**15** DETAIL  
SCALE: NONE



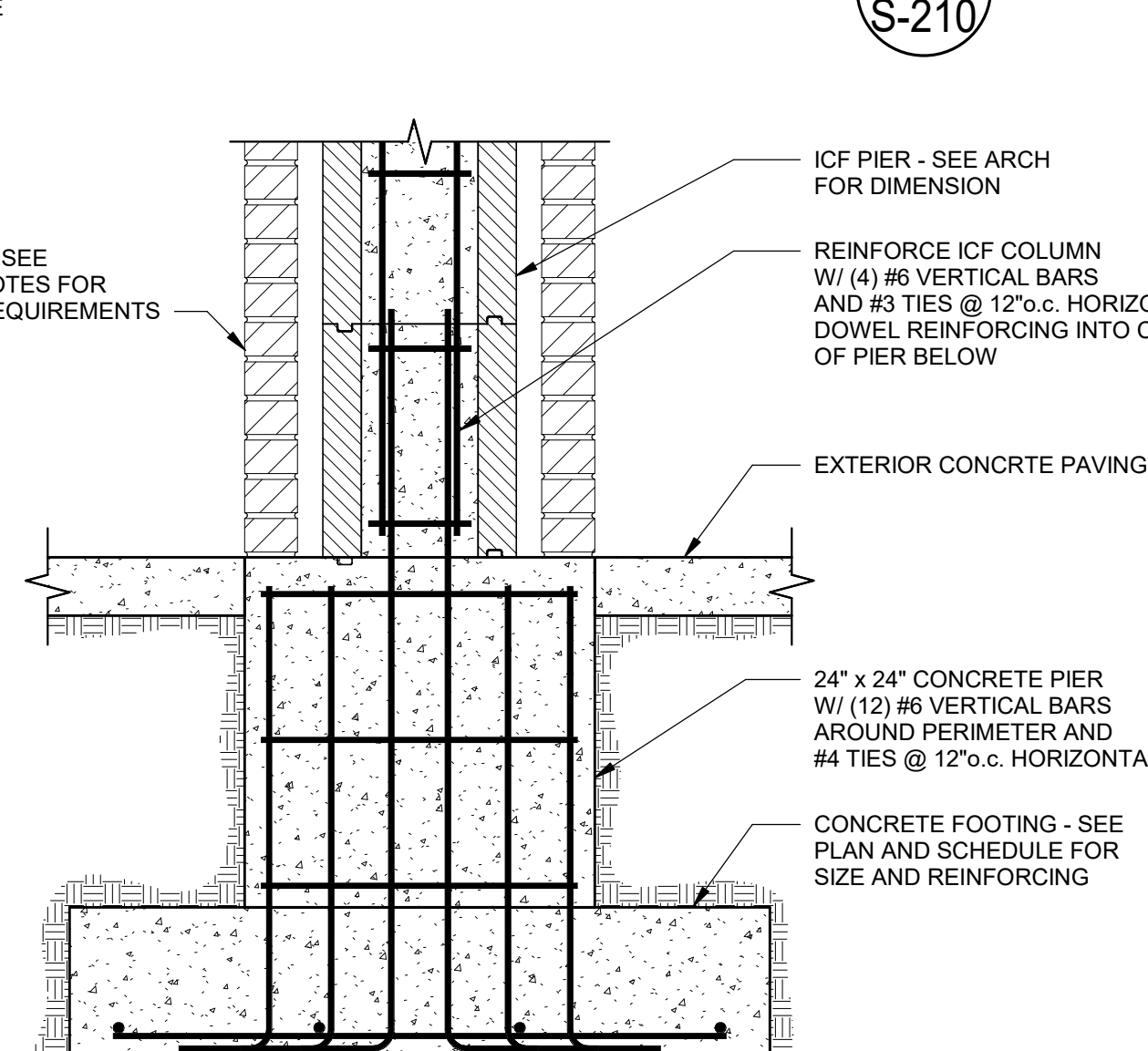
**16** DETAIL  
SCALE: NONE



**17** DETAIL  
SCALE: NONE



**18** DETAIL  
SCALE: NONE



**19** DETAIL  
SCALE: NONE



**20** DETAIL  
SCALE: NONE

MARK	DATE	DESCRIPTION

PROJECT #: 123005  
DRAWN BY: BLP  
CHECKED BY: J. Blizard  
ISSUED: 03.14.2024



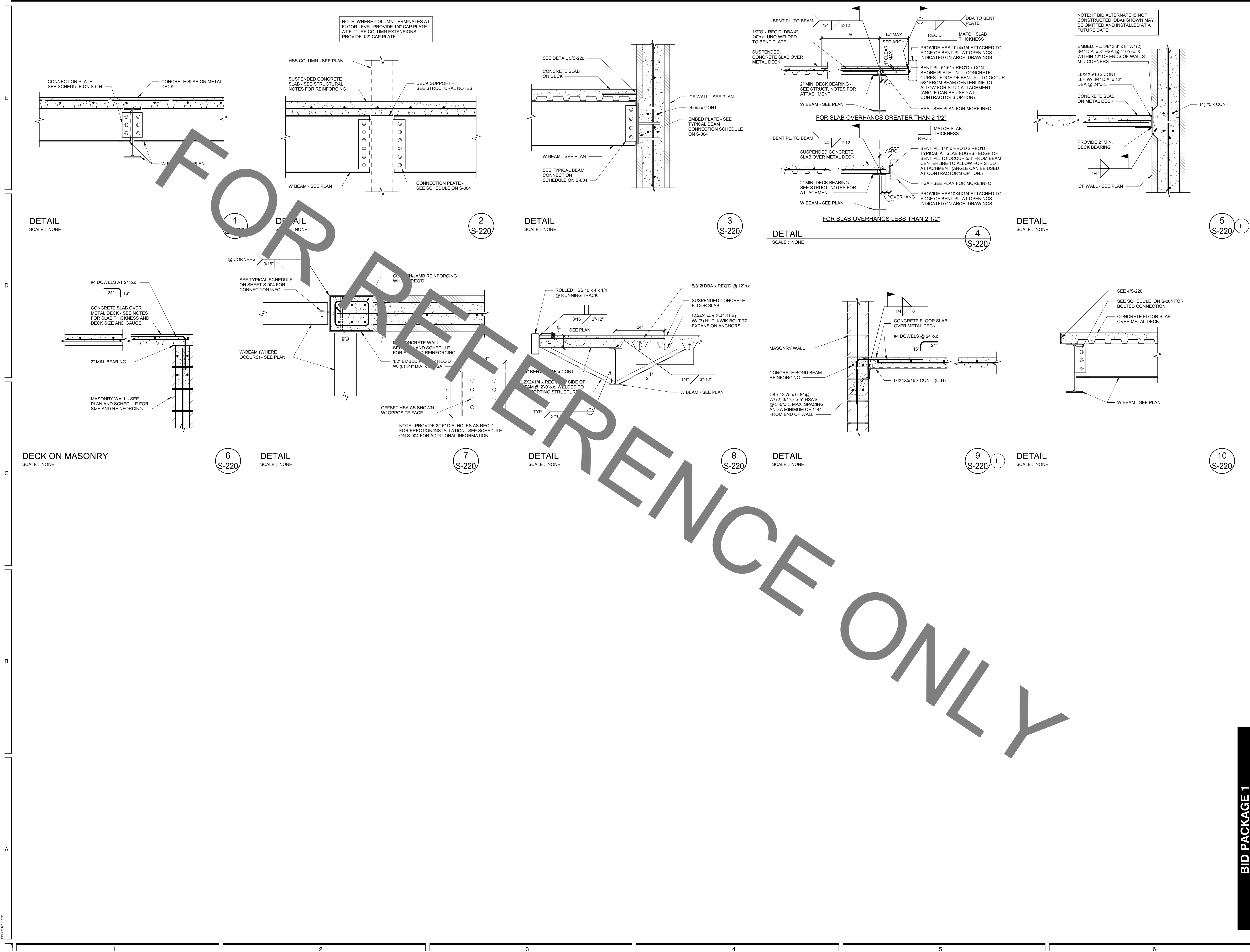




MARK	DATE	DESCRIPTION

PROJECT #: 123005  
 DRAWN BY: BLP  
 CHECKED BY: J. Blazzard  
 ISSUED: 03.14.2024

BID PACKAGE 1



FOR REFERENCE ONLY

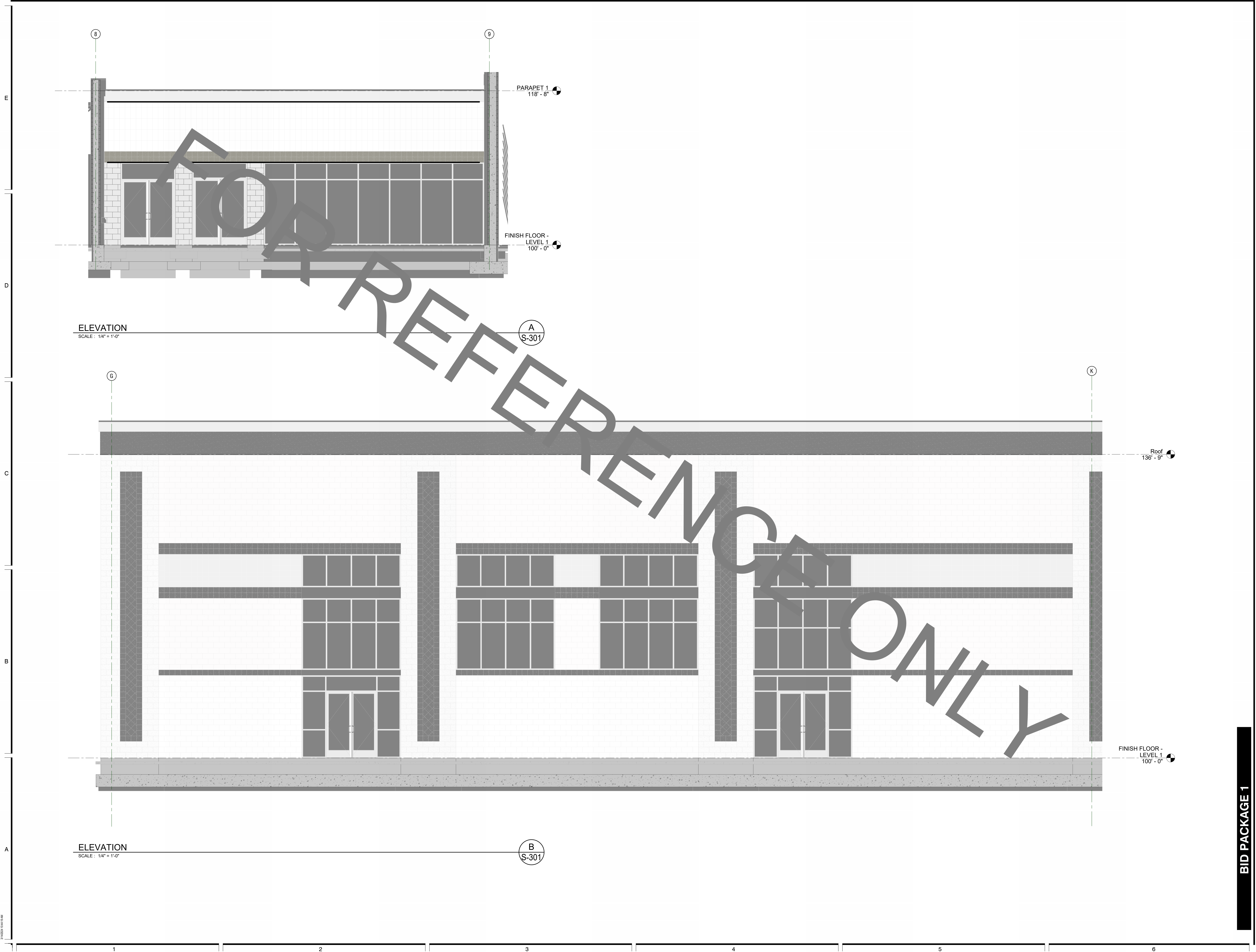










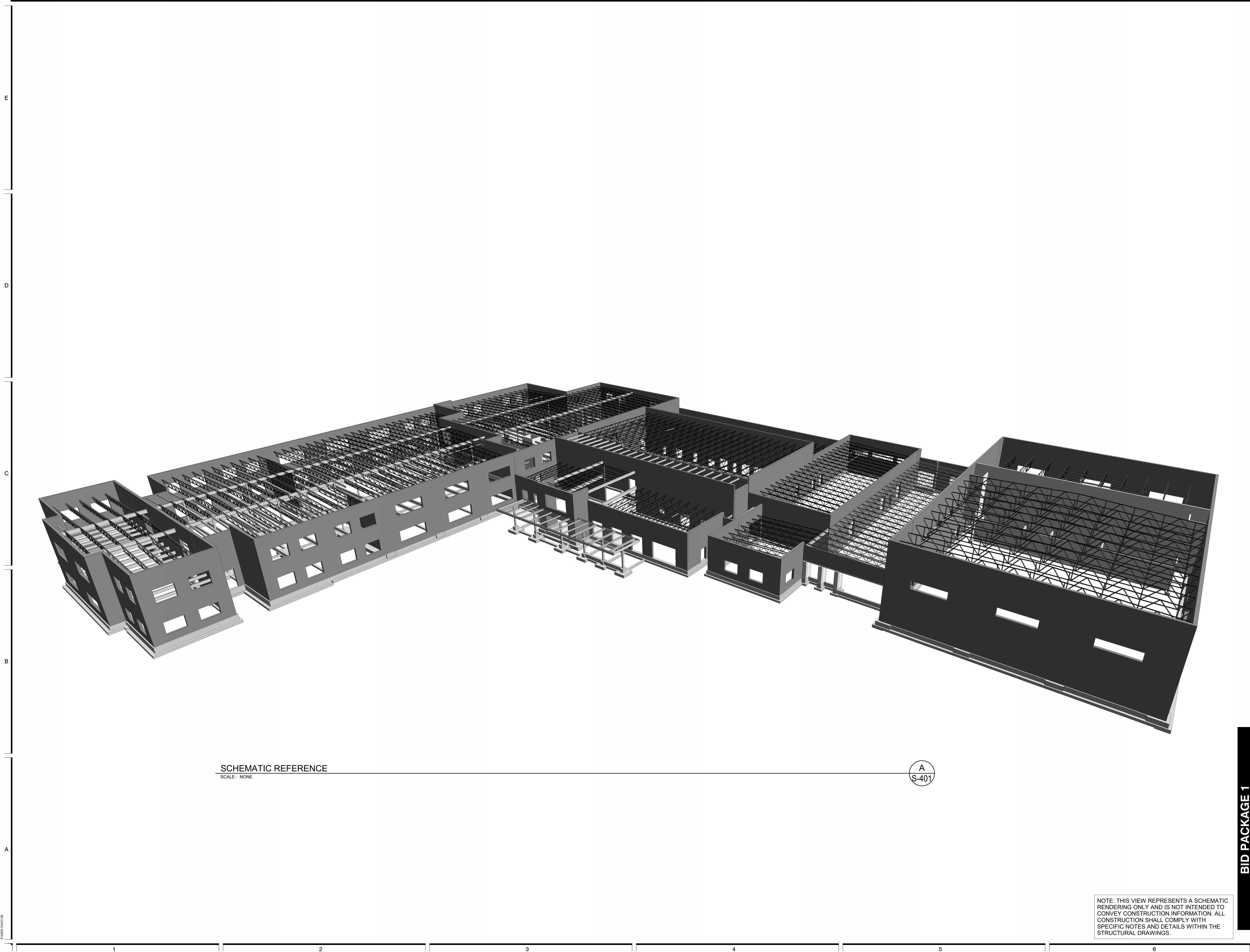


MARK	DATE	DESCRIPTION

PROJECT #:	123005
DRAWN BY:	BLP
CHECKED BY:	J. Blazzard
ISSUED:	03.14.2024

**BID PACKAGE 1**





SCHEMATIC REFERENCE  
SCALE: NONE

A  
S-401

NOTE: THIS VIEW REPRESENTS A SCHEMATIC RENDERING ONLY AND IS NOT INTENDED TO CONVEY CONSTRUCTION INFORMATION. ALL CONSTRUCTION SHALL COMPLY WITH SPECIFIC NOTES AND DETAILS WITHIN THE STRUCTURAL DRAWINGS.

BID PACKAGE 1

MARK	DATE	DESCRIPTION

PROJECT #: 123005  
 DRAWN BY: BLP  
 CHECKED BY: J. Blazzard  
 ISSUED: 03.14.2024





















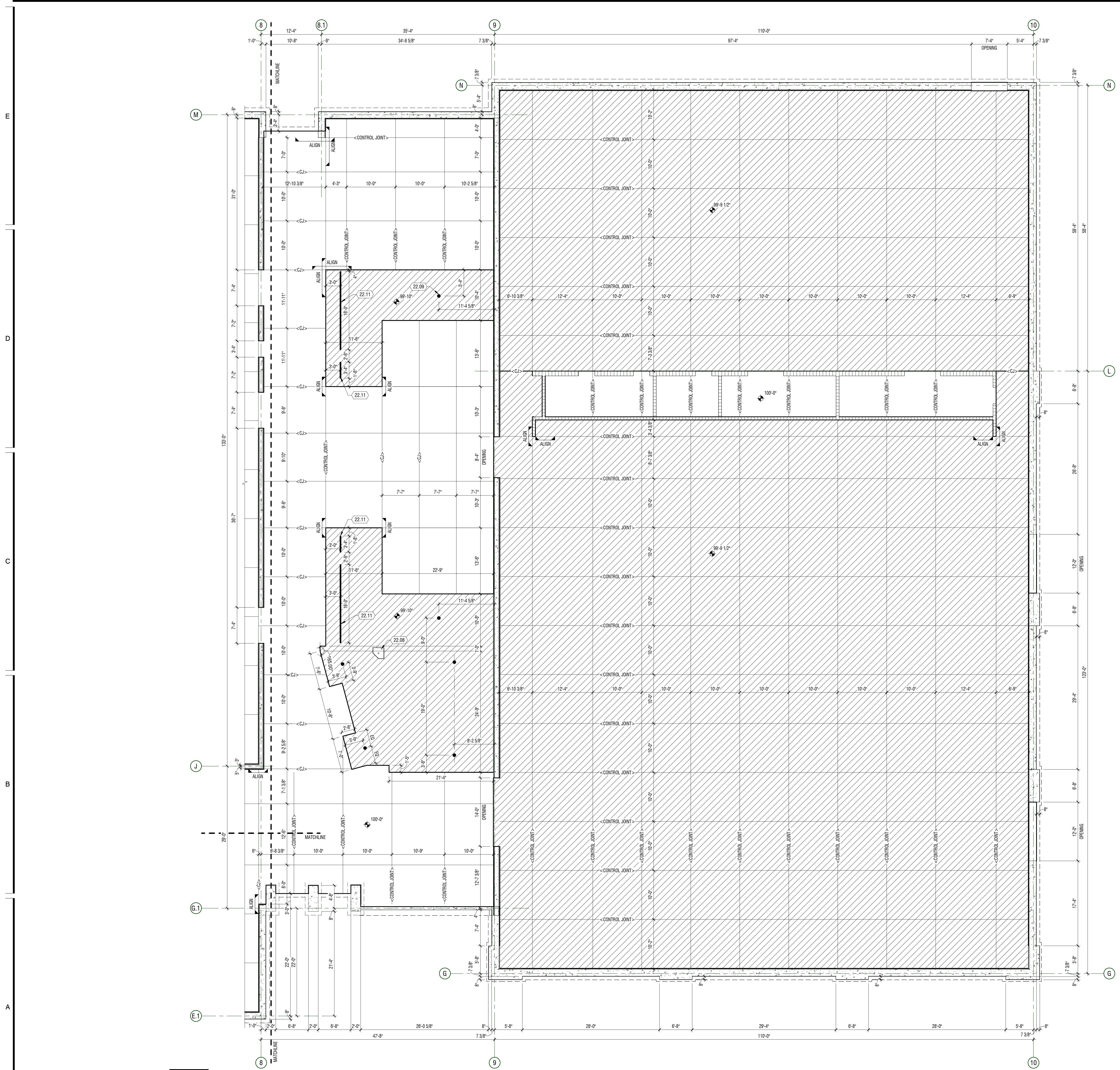












**A2** PLAN - SLAB  
 1/8" = 1'-0" AREA F - LEVEL 01

**GENERAL NOTES**

- CONTRACTOR SHALL BE FAMILIARIZED WITH THE LAY-OUT OF STRUCTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS. ANY QUESTIONS SHALL BE SUBMITTED VIA REQUEST FOR INFORMATION (RFI).
- DIMENSIONS ARE FROM GRID TO CENTERLINE OF COLUMN, DRAINS, ETC., U.N.O. AND EDGE OF ICF FOAM BLOCKS.
- COORDINATE WITH STRUCTURAL DRAWINGS FOR SIZE AND DEPTH OF FOOTING, METAL DECK, AND OTHER DETAILS.
- NOT ALL SLAB PENETRATIONS ARE SHOWN ON PLAN. COORDINATE WITH PLUMBING AND MECHANICAL DRAWINGS FOR ADDITIONAL DEVIATIONS.
- SLOPE ALL SETTING BEDS TO FLOOR DRAINS U.N.O.
- REVEAL TO OCCUR AT EXTERIOR GRADE. SEE BID PACKAGE 2 FOR PROFILE.

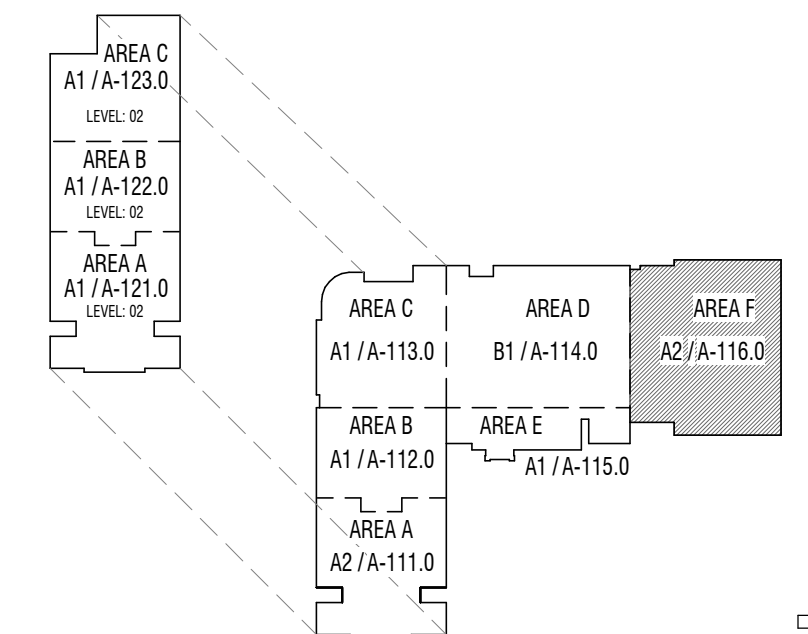
**KEYNOTES**

MARK	DESCRIPTION
22.08	MOP SINK
22.09	FLOOR DRAIN
22.11	LINEAR TRENCH DRAIN

**LEGEND**

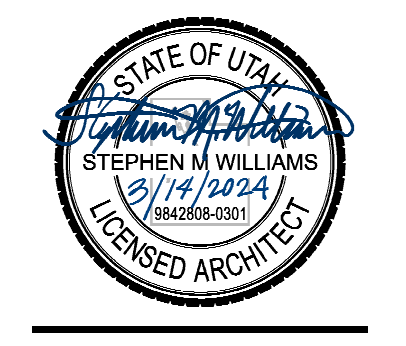
- CONCRETE SLAB
- TUBULAR CONCRETE FORM BLOCK-OUTS
- RECESSED SLAB - SEE PLAN FOR ELEVATION
- ELEVATOR PIT
- 2'-0" x 2'-0" x 2'-0" SUMP AT ELEVATOR PIT

**KEY PLAN**



MARK	DATE	DESCRIPTION

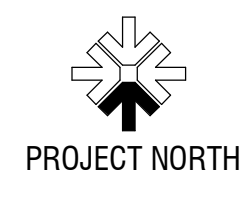
PROJECT #: 123005  
 DRAWN BY: LOYND  
 CHECKED BY: RIGBY  
 ISSUED: 03.14.2024



PLAN - LEVEL 01  
 -AREA F - SLAB & FOUNDATION

**A-116.0**

**BID PACKAGE 1**



PROJECT NORTH



