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# HYDE PARK MIDDLE SCHOOL

ADDITIONALLY, DRAWINGS MAY NOT BE RE-SCALED WHEN PRINTED, WRITTEN DIMENSIONS SHALL HAVE PRECEDENCE, AND LARGER SCALE DRAWINGS SHALL HAVE PRECEDENCE OVER SMALLER SCALE

ANY DEVIATION FROM OR CONFLICT WITHIN THE DRAWINGS AND/OR SPECIFICATIONS, MUST BE SUBMITTED VIA REQUEST FOR INFORMATION (RFI) AND RESPONDED TO BY THE ARCHITECT PRIOR TO BID OR BEFORE CONTINUING THAT PORTION OF WORK.

# BID ALTERNATES

(REFER TO SPECIFICATIONS 01 2300-ALTERNATES FOR FULL DESCRIPTION OF ALTERNATES)

BID ALTERNATE 1 REMOVAL OF CLASSROOMS

BID ALTERNATE 2 ROOFING MEMBRANE

# DEFFERED SUBMITTALS

 FIRE ALARMS 2. FIRE PROTECTION AND SEISMIC DESIGNS

3. OPEN WEB JOISTS AND RELATED COMPONENTS 4. COLD FORM FRAMING 5. SEISMIC BRACING OF ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL ITEMS

# BID PACKAGE NOTES

SHEET REVISIONS WILL BE ISSUED ACCORDING TO THEIR BID PACKAGE. (EX: ASI-1/BP-1)

STEEL STAIRS

BID PACKAGE 1 CIVIL / UNDERGROUND MEP / FOUNDATION (BC.....ISSUED FOR BIDDING AND CONSTRUCTION)

(**RO**.....ISSUED FOR REFERENCE ONLY)

BID PACKAGE 2

LANDSCAPE / STRUCTURAL / ARCHITECTURAL / MECH., PLUMBING, & FIRE / ELECTRICAL / KITCHEN (**BC**.....ISSUED FOR BIDDING AND CONSTRUCTION) (RR/WR.....RE-RELEASED w/ REVISIONS)

# SHEET INDEX

FOOTING AND FOUNDATION PLAN - AREA F

FLOOR FRAMING PLAN - OVERALL

FLOOR FRAMING PLAN - AREA A

FLOOR FRAMING PLAN - AREA B

FLOOR FRAMING PLAN - AREA C

FLOOR FRAMING PLAN - AREA F

ROOF FRAMING PLAN - OVERALL

ROOF FRAMING PLAN - AREA A

ROOF FRAMING PLAN - AREA B

ROOF FRAMING PLAN - AREA C

ROOF FRAMING PLAN - AREA D

ROOF FRAMING PLAN - AREA E

ROOF FRAMING PLAN - AREA F

FOOTING & FOUNDATION DETAILS

FOOTING & FOUNDATION DETAILS

FLOOR FRAMING DETAILS

ROOF FRAMING DETAILS

ROOF FRAMING DETAILS

SCHEMATIC REFERENCE

AS-100 OVERALL ARCHITECTURAL SITE PLAN

ARCHITECTURAL SITE PLAN AREA A

ARCHITECTURAL SITE PLAN AREA B

ARCHITECTURAL SITE PLAN AREA C

ARCHITECTURAL SITE PLAN AREA D

W/0

WELDED WIRE FABRIC

ELEVATIONS

SITE DETAILS

TYPICAL DETAILS

TYPICAL DETAILS

SPORTS STORAGE FOOTING, FDN, AND ROOF FRAMING PLAN

S-120

S-124

S-130

S-131

S-132

S-133

S-134

S-135

S-136

S-140

S-201

S-202

S-220

S-230

S-231

S-301

S-401

AS-101

AS-103

AS-104

AS-501

ARCHITECTURAL SITE:

	1GE 1		1GE 1		4GE 1			2 7 dgE 1
	ACK/		ACK/		ACK		CAGE	CAGE CAGE
VOLUME 1	ID P.		IID P.		PACE	VOLUME 2	PACE CONTRACTOR OF THE	HACE PACE
	4 - B		4 - B		8- 4- GB			BID 810
	.202		.202		.202	903	202	. 202 . 202
SHEET # SHEET CONTENTS	<b>)3.07</b>	SHEET # SHEET CONTENTS	<b>33.07</b>	SHEET # SHEET CONTENTS	<b>33.07</b>	SHEET # SHEET CONTENTS g	S 07: S Sheet # Sheet Contents	S 23 SHEET # SHEET CONTENTS SG 53 SG 55 SHEET # SHEET CONTENTS
						_ <del>-</del>		
GENERAL:		ARCHITECTURAL:		A-416 ENLARGED VIEWS	BC	M-402 MECHANICAL ENLARGED VIEWS	BC E-112 LEVEL 1 - AREA B - LIGHTING	F-101 FIRE PROTECTION PLAN - LEVEL 01 - OVERALL BC
G-001 COVER SHEET	ВС	A-101 PLAN - OVERALL LEVEL 1	BC RR	A-417 ENLARGED VIEWS	BC	M-403 MECHANICAL ENLARGED VIEWS	BC E-113 LEVEL 1 - AREA C - LIGHTING	F-102 FIRE PROTECTION PLAN - LEVEL 02 - OVERALL BC
G-002 PROJECT INFORMATION	BC	A-102 PLAN - OVERALL LEVEL 2	BC RR	A-418 ENLARGED VIEWS	BC	M-404 MECHANICAL ENLARGED VIEWS	BC E-114 LEVEL 1 - AREA D - LIGHTING	F-501 FIRE PROTECTION DETAILS BC
G-003 PLAN - SITE CODE	RO BC	A-111.0 PLAN - LEVEL 01 - AREA A - SLAB & FOUNDATION	BC RR	A-419 ENLARGED VIEWS	BC	M-501 MECHANICAL DETAILS	BC E-115 LEVEL 1 - AREA E - LIGHTING	- WTOUEN
G-004 CODE ANALYSIS	BC	A-111.1 PLAN - LEVEL 01 - AREA A - DIMENSION	BC	A-420 ENLARGED VIEWS	BC BC	M-502 MECHANICAL DETAILS	BC E-116 LEVEL 1 - AREA F - LIGHTING	KITCHEN  K 101 KITCHEN FOURMENT LAVOUT
G-005 PLAN - CODE G-006 PLAN - CODE	BC BC	A-111.2 PLAN - LEVEL 01 - AREA A - ANNOTATION  A-111.3 PLAN - LEVEL 01 - AREA A - FINISH	BC BC	A-421 ENLARGED VIEWS A-422 ENLARGED VIEWS	BC BC	M-503 MECHANICAL DETAILS M-504 MECHANICAL DETAILS	BC E-121 LEVEL 2 - AREA A - LIGHTING  BC E-122 LEVEL 2 - AREA B - LIGHTING	K-101 KITCHEN EQUIPMENT LAYOUT BC  K-102 KITCHEN EQUIPMENT SCHEDULES BC
G-007 PLAN - CODE	BC BC	A-111.4 PLAN - LEVEL 01 - AREA A - FINISH  A-111.4 PLAN - LEVEL 01 - AREA A - REFLECTED CEILING	BC BC	A-422 ENLARGED VIEWS A-423 ENLARGED VIEWS	BC BC	M-505 MECHANICAL DETAILS  M-505 MECHANICAL DETAILS	BC E-123 LEVEL 2 - AREA D - LIGHTING  E-123 LEVEL 2 - AREA C - LIGHTING	K-201 KITCHEN ELECTRICAL ROUGH-INS BC
G-007 FLAN - CODE		A-112.0 PLAN - LEVEL 01 - AREA B - SLAB & FOUNDATION	BC BB	A-424 ENLARGED VIEWS	BC:	M-506 MECHANICAL DETAILS  M-506 MECHANICAL DETAILS	BC E-131 LEVEL 1 - ENLARGED LIGHTING PLANS	K-301 KITCHEN PLUMBING ROUGH-INS BC
CIVIL:		A-112.1 PLAN - LEVEL 01 - AREA B - DIMENSION	BC RC	A-425 INTERIOR ELEVATIONS	BC	M-601 MECHANICAL SCHEDULES	BC E-141 THEATRICAL LIGHTING RISER	K-401 KITCHEN ELEVATIONS BC
C-001 CIVIL GENERAL NOTES	BC	A-112.2 PLAN - LEVEL 01 - AREA B - ANNOTATION	BC	A-426 INTERIOR ELEVATIONS	BC	M-602 MECHANICAL SCHEDULES	BC E-151 LIGHTING DETAILS	K-402 KITCHEN ELEVATIONS BC
C-101 CIVIL - DEMOLITION PLAN	BC	A-112.3 PLAN - LEVEL 01 - AREA B - FINISH	BC	A-427 INTERIOR ELEVATIONS	BC	M-603 MECHANICAL SCHEDULES	BC E-152 LIGHTING NOTES	K-403 SECTIONS AND DETAILS BC
C-201 CIVIL - OVERALL SITE PLAN	RO	A-112.4 PLAN - LEVEL 01 - AREA B - REFLECTED CEILING	BC	A-428 INTERIOR ELEVATIONS	BC	M-604 MECHANICAL SCHEDULES	BC E-161 LIGHT FIXTURE SCHEDULE	K-501 KITCHEN EXHAUST HOOD DETAILS BC
C-202 CIVIL - SITE PLAN SOUTHWEST	RO	A-113.0 PLAN - LEVEL 01 - AREA C - SLAB & FOUNDATION	BC RR	A-431 ENLARGED VIEWS - STAIR/ELEVATOR/ LADDER	BC	M-701 MECHANICAL SCHEMATICS	BC E-211 LEVEL 1 - AREA A - POWER	K-502 KITCHEN EXHAUST HOOD DETAILS BC
C-203 CIVIL - SITE PLAN NORTHWEST	RO	A-113.1 PLAN - LEVEL 01 - AREA C - DIMENSION	BC	A-432 ENLARGED VIEWS - STAIR/ELEVATOR/ LADDER	BC	M-702 MECHANICAL SCHEMATICS	BC E-212 LEVEL 1 - AREA B - POWER	K-503 KITCHEN EXHAUST HOOD DETAILS BC
C-204 CIVIL - SITE PLAN NORTHEAST	RO	A-113.2 PLAN - LEVEL 01 - AREA C - ANNOTATION	BC	A-433 ENLARGED VIEWS - STAIR/ELEVATOR/ LADDER	BC	M-703 MECHANICAL SCHEMATICS	BC E-213 LEVEL 1 - AREA C - POWER	K-504 KITCHEN EXHAUST HOOD DETAILS BC
C-205 CIVIL - SITE PLAN SOUTHEAST	RO	A-113.3 PLAN - LEVEL 01 - AREA C - FINISH	BC	A-434 ENLARGED VIEWS - ELEVATOR	BC	M-704 MECHANICAL SCHEMATICS	BC E-214 LEVEL 1 - AREA D - POWER	K-505 KITCHEN UDS WALL DETAILS BC
C-301 CIVIL UTILITY PLAN	BC	A-113.4 PLAN - LEVEL 01 - AREA C - REFLECTED CEILING	BC	A-511 DETAILS - METAL STUD TYPICALS	BC	M-705 MECHANICAL SCHEMATICS	BC E-215 LEVEL 1 - AREA E - POWER	K-601 BUILDING CONDITIONS - SLAB DEPRESSION PLAN BC
C-302 CIVIL STORMWATER PLAN	R0	A-114.0 PLAN - LEVEL 01 - AREA D - SLAB & FOUNDATION	BC RR	A-512 DETAILS - ADA STANDARDS	BC	M-706 MECHANICAL SCHEMATICS	BC E-216 LEVEL 1 - AREA F - POWER	
C-401 CIVIL - OVERALL GRADING PLAN	BC	A-114.1 PLAN - LEVEL 01 - AREA D - DIMENSION	BC	A-531 DETAILS - SLAB & FOUNDATION	BC	M-707 MECHANICAL SCHEMATICS	BC E-221 LEVEL 2 - AREA A - POWER	
C-402 CIVIL - GRADING PLAN SOUTHWEST	BC	A-114.2 PLAN - LEVEL 01 - AREA D - ANNOTATION	BC	A-532 DETAILS - BUILDING	BC	MP-101 MECHANICAL PIPING PLAN - LEVEL 01 - OVERALL	BC E-222 LEVEL 2 - AREA B - POWER	
C-403 CIVIL - GRADING PLAN NORTHWEST	BC	A-114.3 PLAN - LEVEL 01 - AREA D - FINISH	BC	A-533 DETAILS - BUILDING	BC	MP-102 MECHANICAL PIPING PLAN - LEVEL 02 - OVERALL	BC E-223 LEVEL 2 - AREA C - POWER	
C-404 CIVIL - GRADING PLAN NORTHEAST	BC	A-114.4 PLAN - LEVEL 01 - AREA D - REFLECTED CEILING	BC	A-534 DETAILS - BUILDING	BC	MP-103 MECHANICAL PIPING PLAN - ROOF - OVERALL	BC E-224 LEVEL 2 - ROOF PLAN - OVERALL - POWER	
C-405 CIVIL - GRADING PLAN SOUTHEAST	BC	A-115.0 PLAN - LEVEL 01 - AREA E - SLAB & FOUNDATION	BC RR	A-541 DETAILS - ROOF	BC BC	MP-111.A MECHANICAL PIPING PLAN - LEVEL 01 - AREA A	BC E-231 LEVEL 1 - ENLARGED POWER PLANS	
C-501 CIVIL UTILITY DETAILS	BC	A-115.1 PLAN - LEVEL 01 - AREA E - DIMENSION	BC	A-542 DETAILS - ROOF	BC	MP-111.B MECHANICAL PIPING PLAN - LEVEL 01 - AREA B	BC E-232 ENLARGED KITCHEN PLAN	
C-502 CIVIL UTILITY DETAILS	BU	A-115.2 PLAN - LEVEL 01 - AREA E - ANNOTATION	BC BC	A-543 DETAILS - CANOPY	BC BC	MP-111.C MECHANICAL PIPING PLAN - LEVEL 01 - AREA C	BC E-241 POWER DETAILS	
C-503 CIVIL UTILITY DETAILS C-601 CIVIL - ROADWAY PLAN & PROFILE 200 SOUTH	BC BC	A-115.3 PLAN - LEVEL 01 - AREA E - FINISH  A-115.4 PLAN - LEVEL 01 - AREA E - REFLECTED CEILING	BC BC	A-544 DETAILS - CANOPY  A-545 DETAILS - CANOPY	BC BC	MP-111.D MECHANICAL PIPING PLAN - LEVEL 01 - AREA D  MP-111.E MECHANICAL PIPING PLAN - LEVEL 01 - AREA E	BC E-242 DATA RISERS  BC E-251 ONE-LINE DIAGRAM	
	R0	A-115.4 PLAN - LEVEL 01 - AREA E - REFLECTED CEILING  A-116.0 PLAN - LEVEL 01 -AREA F - SLAB & FOUNDATION	BC RR	A-545 DETAILS - CANOPY A-551 DETAILS - STAIRS	BC	MP-111.F MECHANICAL PIPING PLAN - LEVEL 01 - AREA F	BC E-251 ONE-LINE DIAGRAM - POWER	
C-602 CIVIL - ROADWAY PLAN & PROFILE 200 WEST	nu	A-116.1 PLAN - LEVEL 01 - AREA F - DIMENSION	DU NN	A-561 SCHEDULE - DOOR & TYPE NOTES	BC BC	MP-112.A MECHANICAL PIPING PLAN - LEVEL 01 - AREA A	BC E-261 PANEL SCHEDULES	
LANDSCAPE:		A-116.2 PLAN - LEVEL 01 - AREA F - ANNOTATION	BC:	A-571 SCHEDULE - FRAME TYPES	BC BC	MP-112.B MECHANICAL PIPING PLAN - LEVEL 02 - AREA B	BC E-262 PANEL SCHEDULES	
L-100 OVERALL PLANTING PLAN	BC	A-116.3 PLAN - LEVEL 01 - AREA F - FINISH	BC BC	A-572 SCHEDULE - FRAME TYPES	BC BC	MP-112.C MECHANICAL PIPING PLAN - LEVEL 02 - AREA C	BC E-263 PANEL SCHEDULES	
L-101 PLANTING PLAN AREA A	BC	A-116.4 PLAN - LEVEL 01 - AREA F - REFLECTED CEILING	BC BC	A-573 DETAILS - WINDOWS	BC BC	MP-112.E MECHANICAL PIPING PLAN - ROOF - AREA E	BC E-264 PANEL SCHEDULES	
L-102 PLANTING PLAN AREA B	BC	A-121.0 PLAN - LEVEL 02 - AREA A - SLAB	BC RR	A-574 DETAILS - WINDOWS	BC	MP-112.F MECHANICAL PIPING PLAN - LEVEL 02 - AREA F	BC E-265 PANEL SCHEDULES	
L-103 PLANTING PLAN AREA C	BC	A-121.1 PLAN - LEVEL 02 - AREA A - DIMENSION	BC	A-576 DETAILS - DOORS	BC	MP-113.A MECHANICAL PIPING PLAN - ROOF - AREA A	BC E-266 PANEL SCHEDULES	
L-104 PLANTING PLAN AREA D	BC	A-121.2 PLAN - LEVEL 02 - AREA A - ANNOTATION	BC	A-577 DETAILS - DOORS	BC	MP-113.B MECHANICAL PIPING PLAN - ROOF - AREA B	BC E-267 PANEL SCHEDULES	
L-501 LANDSCAPE DETAILS	BC	A-121.3 PLAN - LEVEL 02 - AREA A - FINISH	BC	A-581 DETAILS - CEILING	BC	MP-113.C MECHANICAL PIPING PLAN - ROOF - AREA C	BC E-311 LEVEL 1 - AREA A - SYSTEMS	
		A-121.4 PLAN - LEVEL 02 - AREA A - REFLECTED CEILING	BC	A-591 DETAILS - MILLWORK	BC	MP-113.D MECHANICAL PIPING PLAN - ROOF - AREA D	BC E-312 LEVEL 1 - AREA B - SYSTEMS	
STRUCTURAL		A-122.0 PLAN - LEVEL 02 - AREA B - SLAB	BC RR	A-592 DETAILS - MILLWORK	BC	MP-113.F MECHANICAL PIPING PLAN - ROOF - AREA F	BC E-313 LEVEL 1 - AREA C - SYSTEMS	
S-001 STRUCTURAL NOTES	BC BC	A-122.1 PLAN - LEVEL 02 - AREA B - DIMENSION	BC	A-593 DETAILS - MILLWORK	BC	MP-401 MECHANICAL PIPING ENLARGED VIEWS	BC E-314 LEVEL 1 - AREA D - SYSTEMS	
S-002 STRUCTURAL NOTES	BC BC	A-122.2 PLAN - LEVEL 02 - AREA B - ANNOTATION	BC	A-594 DETAILS - INTERIOR	BC	MP-402 MECHANICAL PIPING ENLARGED VIEWS	BC E-315 LEVEL 1 - AREA E - SYSTEMS	
S-010 SCHEDULES	BC BC	A-122.3 PLAN - LEVEL 02 - AREA B - FINISH	BC	A-611 ASSEMBLIES - EXTERIOR WALLS, FLOORS, ROOFS	BC	MP-403 MECHANICAL PIPING ENLARGED VIEWS	BC E-316 LEVEL 1 - AREA F - SYSTEMS	
S-011 SCHEDULES	BC BC	A-122.4 PLAN - LEVEL 02 - AREA B - REFLECTED CEILING	BC	A-612 ASSEMBLIES - INTERIOR WALLS	BC	MP-404 MECHANICAL PIPING ENLARGED VIEWS	BC E-321 LEVEL 2 - AREA A - SYSTEMS	
S-012 SCHEDULES	BC BC	A-123.0 PLAN - LEVEL 02 - AREA C - SLAB	BC RR	A-613 ASSEMBLIES - INTERIOR WALLS	BC	MP-701 MECHANICAL PIPING SCHEMATICS	BC E-322 LEVEL 2 - AREA B - SYSTEMS	
S-013 SCHEDULES	BC BC	A-123.1 PLAN - LEVEL 02 - AREA C - DIMENSION	BC	A-681 SCHEDULE - SIGNAGE	BC	MP-702 MECHANICAL PIPING SCHEMATICS	BC E-323 LEVEL 2 - AREA C - SYSTEMS	
S-110 FOOTING AND FOUNDATION PLAN - OVERALL	BC BC	A-123.2 PLAN - LEVEL 02 - AREA C - ANNOTATION	BC	A-691 - H SCHEDULE - FINISH	BC		E-341 CABLE TRAY DETAILS	
S-111 FOOTING AND FOUNDATION PLAN - AREA A	BC BC	A-123.3 PLAN - LEVEL 02 - AREA C - FINISH	BC	A-691 - N SCHEDULE - FINISH	BC	PLUMBING:	E-342 SECURITY DETAIL & RISERS	
S-112 FOOTING AND FOUNDATION PLAN - AREA B	BC BC	A-123.4 PLAN - LEVEL 02 - AREA C - REFLECTED CEILING	BC	A-692 - H SCHEDULE - FINISH	BC	P-000 PLUMBING TITLE SHEET	BC E-343 ACCESS CONTROL DOOR ROUGH-IN DETAILS	
S-113 FOOTING AND FOUNDATION PLAN - AREA C	BC BC	A-124.1 PLAN - LEVEL 02 - AREA F - DIM/ANNO	RC	A-692 - N SCHEDULE - FINISH	BC	P-101 PLUMBING PLAN - LEVEL 01 - OVERALL	BC E-351 FIRE ALARM RISER DIAGRAM	
S-114 FOOTING AND FOUNDATION PLAN - AREA D	RC RC	A-151 PLAN - ROOF - OVERALL	RC BD	A-811.0 BID ALT 1 - PLAN - LEVEL 01 - AREA A - SLAB	BC	P-102 PLUMBING PLAN - LEVEL 02 - OVERALL	BC E-352 FIRE RISER DIAGRAM HORN ALARM SYSTEM	
S-115 FOOTING AND FOUNDATION PLAN - AREA E	BC BC	A-161 STORAGE SHED	RC KK	A-811.1 BID ALT 1 - PLAN - LEVEL 01 - AREA A - DIMENSION	BC BC	P-103 PLUMBING PLAN - ROOF - OVERALL	BC E-411 LEVEL 1 - AREA A - AUDIO VIUSAL	

**ABBREVIATIONS** 

EQUIP

EQUIPMENT

ELECTRIC WATER COOLER

ABR. **DESCRIPTION DESCRIPTION DESCRIPTION** PART BD ANCHOR BOLT EXIST PARTICLE BOARD EXISTING Part'n P-lam ACRYLONITRILE-BUTADIENE EXPANSION PARTITION PLASTIC LAMINATE PLATE EXTERIOR FLOOR DRAIN PLYWD ACOUSTIC, ACOUSTICAL PLYW00D ACCESSIBLE STATION FOUNDATION PREFABRICATED FIRE EXTINGUISHER CABINET PROJECTION ADDENDUM ADJUSTABLE FINISH PRESERVATIVE TREATED ABOVE FINISH FLOOR FL00R POLYVINYL CHLORIDE QUARRY TILE ALTERNATE FOOTING ALUMINUM GAUGE ROUND ARCHITECT SUPPLEMENTAL GALV GALVANIZED RADIUS INSTRUCTION GALVANIZED IRON **ROOF DRAIN** GYP BD ASPHALT GYPSUM BOARD REFRIGERATOR BASKETBALL HDWD HARDW00D REINFORCE BOARD HOLLOW METAL REVISION REQUEST FOR INFORMATION BUILDING HORIZONTAL BLOCKING HFIGHT ROUGH OPENING INSIDE DIAMETER BENCH MARK SCHED SCHEDULE BOTTOM OF INSULATION INTERIOR SIMILAR BEARING BASEMENT SPECIFICATION KNOCK DOWN BUILT UP ROOF SQUARE KNOCK OUT STAINLESS STEEL CHANNEL STANDARD CHALKBOARD ANGLE LONG LEG VERTICAL STEEL STORAGE CENTER LINE CEILING MAXIMUM STRUCT STRUCTURAL CONCRETE MASONRY UNIT MARKER BOARD CLEAN OUT MECHANICAL SUSPENDED, SUSPENSION MANUFACTURER COLUMN SYSTEM CONCRETE MANHOLE TOP AND BOTTOM MINIMUM CONNECTION TACKBOARD CONTINUOUS MISCELLANEOUS TEMPORARY CONTR TELEPHONE CONTRACTOR MASONRY OPENING THRESHOLD CONTROL JOINT MOUNT CERAMIC TILE METAL TUBE STEEL PENNY NEW TOP OF DIMENSION NOT IN CONTRACT TELEVISION TYPICAL VERTICAL NOT TO SCALE DRAWING ON CENTER EXISTING VERT OUTSIDE DIAMETER EACH OVERHEAD U.N.O. UNLESS NOTED OTHERWISE EXTERIOR INSULATION OWNER FURNISHED / WIDE FLANGE CONTRACTOR INSTALLED FINISH SYSTEM WITH WATER CLOSET ELECTRICAL OWNER FURNISHED / WOOD ELEV ELEVATION OWNER INSTALLED EQUAL OPNG WATER METER WITHOUT

OPPOSITE

OPEN TO STRUCTURE

0.T.S.

ELEVATIONS - EXTERIOR

SECTIONS - BUILDING

SECTIONS - WALL

WALL PROFILES

WALL PROFILES

WALL PROFILES

WALL PROFILES

WALL PROFILES

ENLARGED VIEWS

ENLARGED VIEWS

ENLARGED VIEWS

ENLARGED VIEWS

ENLARGED VIEWS

A-202

A-203

A-204

A-205

A-206

A-331

A-332

A-333

A-334

A-335

A-341

A-342

A-343

A-344

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A-351

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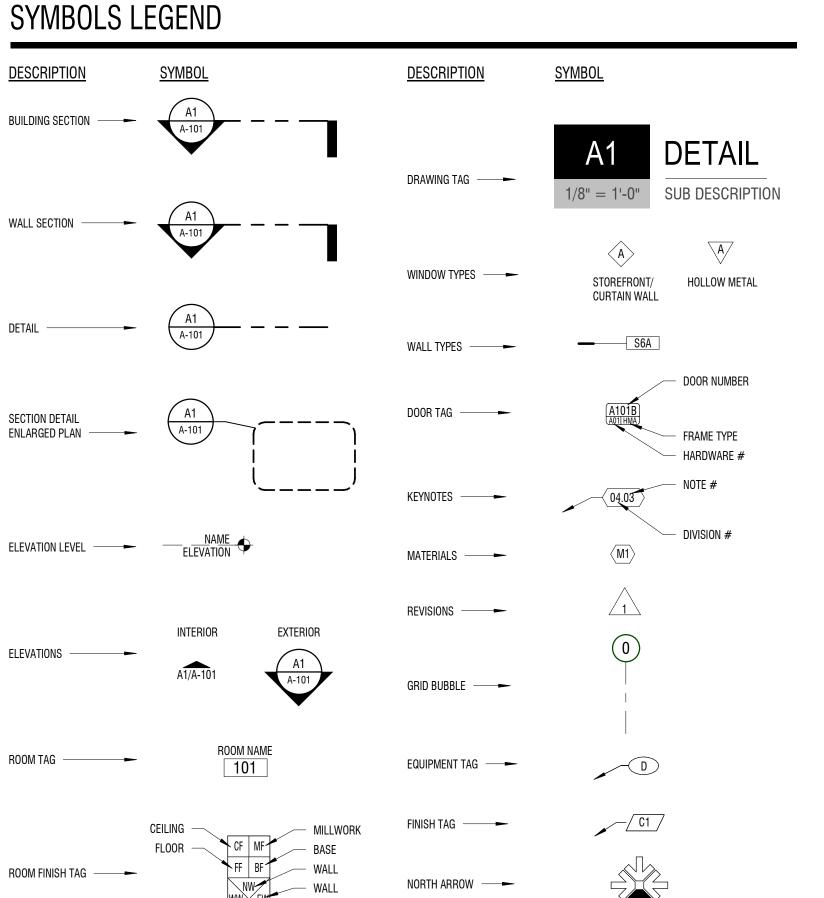
A-356

A-411

A-412

A-413

A-414



MATERIALS LEGEND

A-821.0 BID ALT 1 - PLAN - LEVEL 02 - AREA A - SLAB

MECHANICAL TITLE SHEET

MECHANICAL GENERAL NOTES

M-111.B MECHANICAL HVAC PLAN - LEVEL 01 - AREA B

MECHANICAL HVAC PLAN - LEVEL 01 - OVERALL

MECHANICAL HVAC PLAN - LEVEL 02 - OVERALL

MECHANICAL HVAC PLAN - ROOF - OVERALL

MECHANICAL HVAC PLAN - LEVEL 01 - AREA A

MECHANICAL HVAC PLAN - LEVEL 01 - AREA C

MECHANICAL HVAC PLAN - LEVEL 01 - AREA D

MECHANICAL HVAC PLAN - LEVEL 01 - AREA E

MECHANICAL HVAC PLAN - LEVEL 01 - AREA F

MECHANICAL HVAC PLAN - LEVEL 02 - AREA A

MECHANICAL HVAC PLAN - LEVEL 02 - AREA B

MECHANICAL HVAC PLAN - LEVEL 02 - AREA C

MECHANICAL HVAC PLAN - LEVEL 02 - AREA F

MECHANICAL HVAC PLAN - ROOF - AREA E

MECHANICAL HVAC PLAN - ROOF - AREA A

MECHANICAL HVAC PLAN - ROOF - AREA D

MECHANICAL HVAC PLAN - ROOF - AREA F

NORTH DIRECTION IS INDICATED BY THE FILLED ARROW

M-113.B MECHANICAL HVAC PLAN - ROOF - AREA B

M-113.C MECHANICAL HVAC PLAN - ROOF - AREA C

MECHANICAL ISOMETRIC VIEWS

MECHANICAL ISOMETRIC VIEWS

MECHANICAL ISOMETRIC VIEWS

MECHANICAL ISOMETRIC VIEWS

MECHANICAL ENLARGED VIEWS

MECHANICAL:

M-000

M-001

M-101

M-102

M-103

M-111.A

M-111.C

M-111.D

M-112.E

M-112.F

M-301

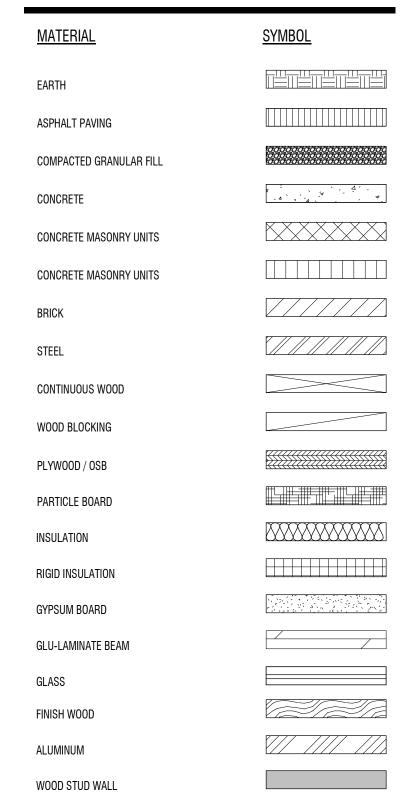
M-302

M-303

M-304

M-401

A-821.1 BID ALT 1 - PLAN - LEVEL 02 - AREA A - DIMENSION



VICINITY MAP

P-111.A PLUMBING PLAN - LEVEL 01 - AREA A

P-111.B PLUMBING PLAN - LEVEL 01 - AREA B

P-111.E

P-111.F

P-112.A

P-112.B

P-402

P-403

P-404

P-405

P-810

P-811

P-813

E-001

E-002

E-103

ELECTRICAL

PLUMBING PLAN - LEVEL 01 - AREA C

PLUMBING PLAN - LEVEL 01 - AREA D

PLUMBING PLAN - LEVEL 01 - AREA E

PLUMBING PLAN - LEVEL 01 - AREA F

PLUMBING PLAN - LEVEL 02 - AREA A

PLUMBING PLAN - LEVEL 02 - AREA B

PLUMBING PLAN - LEVEL 02 - AREA C

PLUMBING ENLARGED VIEWS

PLUMBING UNDERFLOOR PLAN - LEVEL 01 - BID ALTERNATE

PLUMBING PLAN - LEVEL 01 - BID ALTERNATE

PLUMBING PLAN - LEVEL 02 - BID ALTERNATE

PLUMBING PLAN - ROOF - BID ALTERNATE

GENERAL NOTES AND SYMBOLS LISTS

SITE PLAN - HYDE PARK - ELECTRICAL

SITE PLAN - NIBLEY - ELECTRICAL

LEVEL 1 - AREA A - LIGHTING

PLUMBING DETAILS

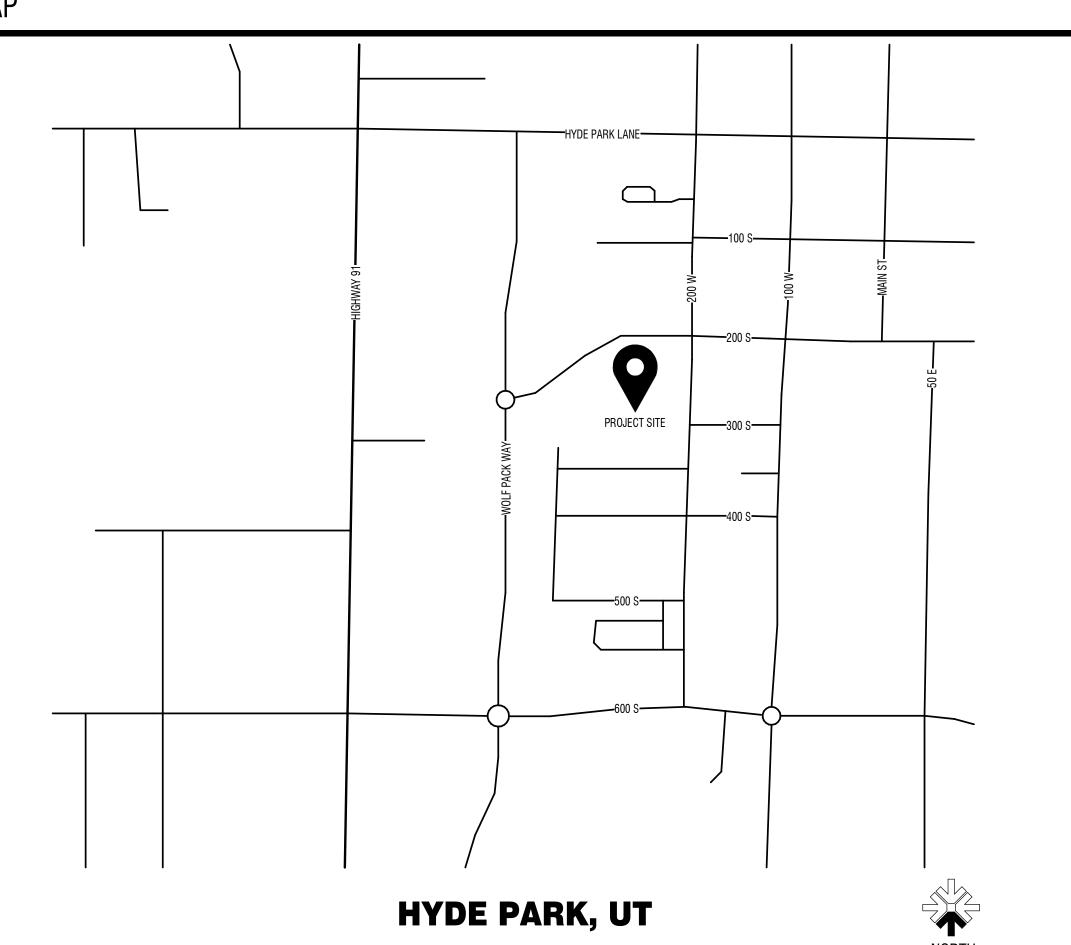
PLUMBING DETAILS

PS-101 PLUMBING SITE PLAN

SYMBOL LISTS

SITE DETAILS

PLUMBING SCHEDULES



BID PACKAG



LEVEL 1 - AREA B - AUDIO VIUSAL

LEVEL 1 - AREA C - AUDIO VIUSAL

LEVEL 1 - AREA D - AUDIO VIUSAL

LEVEL 1 - AREA E - AUDIO VIUSAL

LEVEL 1 - AREA F - AUDIO VIUSAL

LEVEL 2 - AREA A - AUDIO VIUSAL

LEVEL 2 - AREA B - AUDIO VIUSAL

LEVEL 2 - AREA C - AUDIO VIUSAL

AUDIO VISUAL SECTIONS AND ELEVATIONS

AUDIO VISUAL SECTIONS AND ELEVATIONS

AUDIO VISUAL ENLARGED PLANS Copy 1

AUDIO VISUAL RISER AND EQUIPMENT LIST

AUDIO VISUAL DETAILS

AV RISER & EQUIPMENT LIST

LEVEL 1 - AREA A - INTERCOM

LEVEL 1 - AREA B - INTERCOM

LEVEL 1 - AREA C - INTERCOM

LEVEL 1 - AREA D - INTERCOM

LEVEL 1 - AREA E - INTERCOM

LEVEL 1 - AREA F - INTERCOM

LEVEL 2 - AREA A - INTERCOM

LEVEL 2 - AREA B - INTERCOM

LEVEL 2 - AREA C - INTERCOM

AUDIO VISUAL ENLARGED PLANS - TYPICAL CLASSROOMS

E-413

INTERCOM DETAILS INTERCOM - RISER & EQUIP LIST. FIRE PROTECTION: F-001 FIRE PROTECTION TITLE SHEET

HYD

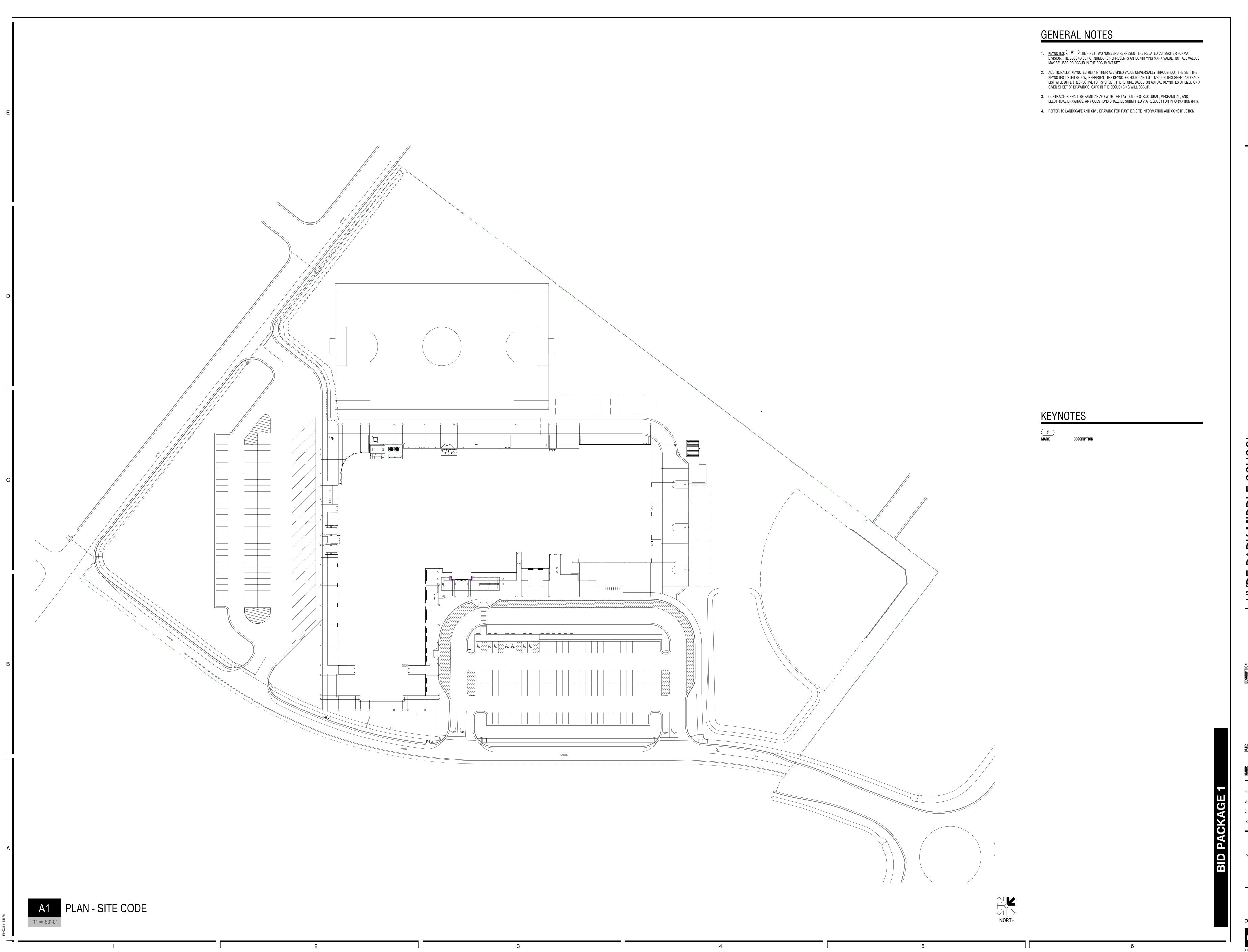
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architects

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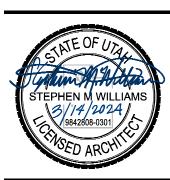


architects
LOGAN UT 84321
ALT LAKE CITY UT 84103

4

design 255 SOUTH 300 WEST 795 NORTH 400 WEST

ARK MIDDLE



PLAN - SITE CODE

GH-003

CIVIL GENERAL NOTES

BID PACKAGE NOTES:

### BID PACKAGE #1 TO INCLUDE:

- 1. ROUGH GRADING, ON-SITE.
- 2. ROUGH GRADING, 200 SOUTH STREET.
- 3. INSTALLATION OF WATER AND SEWER UTILITIES.
- 4. STOCKPILING OF TOP SOIL.

# BID PACKAGE #2 TO INCLUDE:

- 1. INSTALLATION OF STORMDRAIN, GAS, POWER, AND COMMUNICATION UTILITIES.
- 2. FINISH GRADING, ROADWAY AND ON-SITE, INCLUDING:
- -ASPHALT PAVING. -CONCRETE CURBING AND FLATWORK. -TOP SOIL IN LANDSCAPE AREAS.
- 3. WIDEN EXISTING ROADWAY (200 WEST STREET).

# GENERAL SITE NOTES

- 1. NO WORK IS TO BEGIN UNTIL NECESSARY PERMITS HAVE BEEN OBTAINED.
- 2. REQUIREMENTS SHOWN ON SITE PLAN SHALL GOVERN. GENERAL CONTRACTOR TO POINT OUT ANY DISCREPANCIES PRIOR TO CONSTRUCTION.
- 3. ENTIRE INSTALLATION SHALL MEET ALL APPLICABLE CODES.
- 4. VERIFY ALL CONDITIONS AND DIMENSIONS ON SITE.
- 5. GENERAL CONTRACTOR RESPONSIBLE TO OBTAIN AND PAY FOR ALL NECESSARY PERMITS.
- 6. GENERAL CONTRACTOR TO PROVIDE ALL EQUIPMENT, PERSONNEL, AND CONSTRUCTION STAKING REQUIRED FOR FINAL CHECKOUT OF ALL FACILITIES BY OWNER'S REPRESENTATIVE.
- 7. GENERAL CONTRACTOR TO PERFORM GENERAL YARD AND BUILDING CLEAN-UP AT COMPLETION OF
- 8. ALL PUBLIC IMPROVEMENTS SHALL BE IN ACCORDANCE WITH THE HYDE PARK CITY STANDARDS AND SPECIFICATIONS FOR THE DESIGN AND CONSTRUCTION OF PUBLIC IMPROVEMENTS, LATEST REVISION THEREOF. IT IS RECOMMENDED THAT THE CONTRACTOR OBTAIN A COPY OF THIS MANUAL FROM THE CITY OFFICES.
- 9. ALL ASPHALT CUTS FOR UTILITIES AND PAVEMENT WITHIN PUBLIC RIGHTS OF WAY SHALL BE IN ACCORDANCE WITH THE HYDE PARK CITY STANDARDS AND SPECIFICATIONS. "WORK IN R/W" PERMITS ARE REQUIRED.
- 10. GENERAL CONTRACTOR SHALL PROVIDE TRAFFIC CONTROL PLAN WHICH SHALL BE SUBMITTED TO AND APPROVED BY THE CITY OF NORTH LOGAN PRIOR TO ANY WORK IN THE PUBLIC R/W. CONTRACTOR IS RESPONSIBLE FOR SAFETY TO THE PUBLIC BY MINIMIZING THE INTERRUPTION OF THE USE OF ROADS AND PROVIDING SIGNS, FLARES, BARRICADES, ETC. AS NECESSARY. TRAFFIC CONTROL TO BE COMPLIANT WITH CURRENT MUTCD. WORK SHALL COMPLY WITH "WORK IN R/W"
- 11. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO VERIFY THE ACTUAL LOCATION AND ELEVATION OF EXISTING UTILITIES WHICH MAY BE IN CONFLICT WITH THE PROPOSED CONSTRUCTION. IF A CONFLICT DOES EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER PRIOR TO CONSTRUCTION SO THAT ADJUSTMENTS CAN BE MADE.
- 12. IT WILL BE THE RESPONSIBILITY OF THE CONTRACTOR TO NOTIFY ALL UTILITIES WHEN CONSTRUCTION WORK BEGINS IN THE VICINITY OF ANY UTILITY LINES AND TO ARRANGE FOR A REPRESENTATIVE OF THE UTILITY TO BE PRESENT IF THE CONTRACTOR'S OPERATIONS ARE IN CLOSE PROXIMITY TO ANY LINES IN THEIR EXISTING OR RELOCATED POSITION WHICH COULD CREATE A HAZARDOUS CONDITION.
- 13. WHERE THERE IS A CONFLICT BETWEEN THESE PLANS AND THE SPECIFICATIONS, OR ANY APPLICABLE STANDARDS, THE HIGHER QUALITY STANDARD SHALL APPLY.
- 14. THE CONTRACTOR SHALL BE RESPONSIBLE FOR KEEPING ROADWAYS FREE AND CLEAR OF ALL CONSTRUCTION DEBRIS AND DIRT TRACKED FROM THE SITE.
- 15. DIMENSIONS FOR LAYOUT AND CONSTRUCTION ARE NOT TO BE SCALED FROM ANY DRAWING. IF PERTINENT DIMENSIONS ARE NOT SHOWN, CONTACT THE CONSULTING ENGINEER FOR CLARIFICATION, AND ANNOTATE THE DIMENSION ON THE AS-BUILT RECORD DRAWINGS.
- 16. OWNER/CONTRACTOR IS RESPONSIBLE TO OBTAIN A UPDES STORMWATER DISCHARGE PERMIT AND IS RESPONSIBLE FOR DEVELOPING AND IMPLEMENTING A STORMWATER POLLUTION PREVENTION PLAN (SWPPP) AS PER THE REQUIREMENTS OF THE UPDES STORMWATER CONSTRUCTION PERMIT (NOI PERMIT # \_\_\_\_\_).

# UTILITY NOTES

- 1. ALL UTILITY LOCATIONS SHOWN ARE BASED ON MAPS PROVIDED BY THE APPROPRIATE UTILITY COMPANY AND FIELD SURFACE EVIDENCE AT THE TIME OF SURVEY AND IS TO BE CONSIDERED AN APPROXIMATE LOCATION ONLY. IT IS THE CONTRACTOR'S RESPONSIBILITY TO FIELD VERIFY THE ACTUAL LOCATION OF ALL UTILITIES. PUBLIC OR PRIVATE. WHETHER SHOWN ON THE PLANS OR NOT, PRIOR TO CONSTRUCTION. REPORT ANY DISCREPANCIES TO THE ENGINEER PRIOR TO CONSTRUCTION.
- 2. WHERE A PROPOSED UTILITY CROSSES AN EXISTING UTILITY, IT IS THE CONTRACTOR'S RESPONSIBILITY TO FIELD VERIFY THE HORIZONTAL AND VERTICAL LOCATION OF SUCH EXISTING UTILITY, EITHER THROUGH POTHOLING OR ALTERNATIVE METHOD. REPORT
- INFORMATION TO THE ENGINEER PRIOR TO CONSTRUCTION. 3. VERIFY ALL CONDITIONS AND DIMENSIONS ON SITE.
- 4. GENERAL CONTRACTOR TO COORDINATE ALL UTILITY WORK WITH THE APPROPRIATE UTILITY PROVIDER. GENERAL CONTRACTOR TO VERIFY AND FOLLOW ALL UTILITY PROVIDER REQUIREMENTS, PROCEDURES, STANDARDS AND SPECIFICATIONS.
- 5. GENERAL CONTRACTOR TO PROVIDE ALL EQUIPMENT, PERSONNEL, AND CONSTRUCTION STAKING REQUIRED FOR FINAL CHECKOUT OF ALL FACILITIES BY OWNER'S REPRESENTATIVE.
- 6. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO VERIFY THE ACTUAL LOCATION AND ELEVATION OF EXISTING UTILITIES WHICH MAY BE IN CONFLICT WITH THE PROPOSED CONSTRUCTION. IF A CONFLICT DOES EXIST, THE CONTRACTOR SHALL NOTIFY THE ENGINEER PRIOR TO CONSTRUCTION SO THAT ADJUSTMENTS CAN BE MADE.
- 7. ALL WATER LINES TO HAVE A MINIMUM COVER OF 5-FEET.
- 8. ALL WATER MAINS SHALL BE C900 PVC AND AWWA STANDARD EQUIVALENT.
- 9. SEPARATE WATER AND SEWER MAIN LINES AND SERVICES HORIZONTALLY BY 10-FEET MINIMUM FROM OUTSIDE OF PIPE TO OUTSIDE OF PIPE.

# <u>GRADING NOTES</u>

- 1. FINAL GRADES ARE SUBJECT TO MINOR CHANGES AS APPROVED BY OWNER. NO GRADE CHANGES IN EXCESS OF 0.1 FEET WITHOUT ENGINEERS APPROVAL.
- 2. ALL ROUGH GRADES TO BE ESTABLISHED AS FOLLOWS:
- A. BUILDING PADS, ROADWAYS, PARKING LOTS, FIRE AND TRUCK ACCESS WITHIN 0.10 FT ON TOP OF PIT RUN
- ROUGH GRADE FOR: BUILDING PADS: 8" BELOW FG
- ROADWAYS: 21" BELOW FG
- PARKING LOTS: 7" BELOW FG FIRE AND TRUCK ACCESS (HEAVY-DUTY CONCRETE): 16" BELOW FG
- BUS PARKING: 8" BELOW FG
- B. ATHLETIC PERFORMANCE AREAS- SOFTBALL AND SOCCER PLAY FIELDS WITHIN 0.1 FT OF SUBGRADE
- ROUGH GRADE FOR:
- SOCCER PLAY FIELD: 8" BELOW FG SOFTBALL PLAY FIELD: 8" BELOW FG
- C. SITE CONCRETE (FLATWORK) AND SIDEWALKS WITHIN 0.10 FT OF SUBGRADE
- ROUGH GRADE FOR: SITE CONCRETE: 8" BELOW FG
- D. LANDSCAPE AREAS, PLANTER AREAS, SLOPED AREAS WITHIN 0.2 FT OF SUBGRADE
- ROUGH GRADE FOR:
- LANDSCAPE AREAS: 8" BELOW FG PLANTER AREAS: 18" BELOW FG
- 3. ALL FILL MATERIAL REQUIRED, SHALL BE CLEAN FILL SOIL APPROVED BY ENGINEER.
- 4. GENERAL CONTRACTOR SHALL BE RESPONSIBLE FOR MINIMIZING DEPOSITION OF ONSITE SEDIMENTS ONTO SURROUNDING PUBLIC STREETS DURING CONSTRUCTION. SEE EROSION CONTROL PLAN.
- 5. GENERAL CONTRACTOR TO PROVIDE BARRICADE PROTECTION WITH FLASHING LIGHTS AROUND ALL EXCAVATIONS
- 6. UNLESS OTHERWISE SPECIFIED, ALL CONSTRUCTION LAYOUT AND STAKING SHALL BE PERFORMED UNDER THE RESPONSIBLE CHARGE OF A LAND SURVEYOR LICENSED IN THE STATE OF UTAH AND BY PARTY CHIEF OR ENGINEERING TECHNICIAN EXPERIENCED IN CONSTRUCTION LAYOUT AND STAKING TECHNIQUES AS REQUIRED BY THE SPECIFIC TYPE OF WORK BEING PERFORMED.
- 7. THE CROSS SLOPE ON NEW SIDEWALKS SHALL NOT EXCEED 2.0%.
- 8. ALL AREAS NOT LOCATED WITHIN A RIGHT—OF—WAY OR UTILITY EASEMENT ARE TO REMAIN UNDISTURBED TO THE MAXIMUM EXTENT POSSIBLE. THESE AREAS SHALL BE AVOIDED AND MAINTAINED BY THE CONTRACTOR IN THEIR PRE-CONSTRUCTION STATE.
- 9. ACCESS AND HAULAGE ROADS SHALL BE MAINTAINED IN A DUST-FREE CONDITION BY SURFACING OR OTHER TREATMENT AS APPROVED BY THE NORTH LOGAN CITY ENGINEER AND FUGITIVE DUST SHALL BE CONTROLLED IN ALL OTHER OPERATIONAL AREAS OF THE EXCAVATION SITE. USE WATER TRUCKS OR OTHER MEANS TO CONTROL DUST AS

# SEWER LAYOUT GENERAL NOTES

- 1. SEE PLAN AND PROFILE SHEETS FOR MORE DETAILED SEWER INFORMATION INCLUDING RIM &
- 2. ALL SEWER MANHOLES SHALL BE INSTALLED PER HYDE PARK CITY STANDARDS UNLESS OTHERWISE INDICATED ON DRAWINGS.
- 2. ALL EXISTING UTILITY LOCATIONS SHOWN ARE BASED UPON MAPS PROVIDED BY THE APPROPRIATE UTILITY COMPANY AND FIELD SURFACE EVIDENCE AT THE TIME OF SURVEY. LOCATIONS ARE TO BE CONSIDERED APPROXIMATE AND ARE PROVIDED FOR REFERENCE AND COORDINATION ONLY. IT IS THE CONTRACTOR'S RESPONSIBILITY TO FIELD VERIFY THE ACTUAL LOCATION OF ALL UTILITIES, PUBLIC OR PRIVATE, WHETHER SHOWN ON THESE PLANS OR NOT, PRIOR TO CONSTRUCTION. REPORT ANY DISCREPANCIES TO THE CIVIL ENGINEER IMMEDIATELY AND PRIOR TO ANY CONSTRUCTION ACTIVITY.
- 3. WHERE A PROPOSED UTILITY CROSSES AN EXISTING UTILITY, IT IS THE CONTRACTOR'S RESPONSIBILITY TO FIELD VERIFY THE HORIZONTAL AND VERTICAL LOCATION OF THE EXISTING UTILITY, EITHER THROUGH POT—HOLING OR ANOTHER ALTERNATIVE METHOD. CONTRACTOR TO REPORT FINDINGS TO THE CIVIL ENGINEER PRIOR TO CONSTRUCTION.
- 4. THE GENERAL CONTRACTOR IS TO ENSURE THE ENTIRE PROJECT INSTALLATION MEETS ALL APPLICABLE FEDERAL, STATE AND LOCAL CODES.
- 5. CONTRACTOR TO FIELD VERIFY ALL CONDITIONS AND DIMENSIONS ON-SITE.
- 6. THE GENERAL CONTRACTOR SHALL COORDINATE ALL UTILITY WORK WITH THE APPROPRIATE UTILITY PROVIDER. THE GENERAL CONTRACTOR SHALL VERIFY AND FOLLOW ALL UTILITY PROVIDER REQUIREMENTS, PROCEDURES, STANDARDS AND SPECIFICATIONS.
- 7. SEPARATE WATER AND SEWER MAIN LINES AND SERVICES HORIZONTALLY BY A MINIMUM OF 10-FEET AS MEASURED FROM OUTSIDE OF PIPE TO OUTSIDE OF PIPE.

# GENERAL WATER NOTES

- 1. ALL UTILITY LOCATIONS SHOWN ARE BASED ON MAPS PROVIDED BY THE APPROPRIATE UTILITY COMPANY AND FIELD SURFACE EVIDENCE AT THE TIME OF SURVEY AND IS TO BE CONSIDERED AN APPROXIMATE LOCATION ONLY. IT IS THE CONTRACTOR'S RESPONSIBILITY TO FIELD VERIFY THE ACTUAL LOCATION OF ALL UTILITIES, PUBLIC OR PRIVATE, WHETHER SHOWN ON THE PLANS OR NOT, PRIOR TO CONSTRUCTION. REPORT ANY DISCREPANCIES TO THE ENGINEER PRIOR TO CONSTRUCTION.
- 2. WHERE A PROPOSED UTILITY CROSSES AN EXISTING UTILITY, IT IS THE CONTRACTOR'S RESPONSIBILITY TO FIELD VERIFY THE HORIZONTAL AND VERTICAL LOCATION OF SUCH EXISTING UTILITY, EITHER THROUGH POTHOLING OR ALTERNATIVE METHOD. REPORT INFORMATION TO THE ENGINEER PRIOR TO CONSTRUCTION.
- 3. VERIFY ALL CONDITIONS AND DIMENSIONS ON SITE.
- 4. GENERAL CONTRACTOR TO COORDINATE ALL UTILITY WORK WITH THE APPROPRIATE UTILITY PROVIDER. GENERAL CONTRACTOR TO VERIFY AND FOLLOW ALL UTILITY PROVIDER REQUIREMENTS, PROCEDURES, STANDARDS AND SPECIFICATIONS.
- 5. GENERAL CONTRACTOR TO PROVIDE ALL EQUIPMENT, PERSONNEL, AND CONSTRUCTION STAKING REQUIRED FOR FINAL CHECKOUT OF ALL FACILITIES BY OWNER'S REPRESENTATIVE.
- 6. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO VERIFY THE ACTUAL LOCATION AND ELEVATION OF EXISTING UTILITIES WHICH MAY BE IN CONFLICT WITH THE PROPOSED CONSTRUCTION. IF A CONFLICT DOES EXIST. THE CONTRACTOR SHALL NOTIFY THE ENGINEER PRIOR TO CONSTRUCTION SO THAT ADJUSTMENTS CAN BE MADE.
- 7. ALL WATER LINES TO HAVE A MINIMUM COVER OF 5-FEET.
- 8. ALL WATER MAINS SHALL BE C900 PVC AND AWWA STANDARD EQUIVALENT.
- 9. SEPARATE WATER AND SEWER MAIN LINES AND SERVICES HORIZONTALLY BY 10-FEET MINIMUM FROM OUTSIDE OF PIPE TO OUTSIDE OF PIPE.
- 10. ALL CONSTRUCTION TO MEET HYDE PARK CITY STANDARDS.

## TREE PROTECTION NOTES:

- 1. ALL EXISTING TREES TO REMAIN SHALL BE PROPERLY PROTECTED DURING CONSTRUCTION. PLACE FOUR-FOOT TALL CONSTRUCTION FENCE AROUND THE CRITICAL ROOT ZONE OF THE TREE. REMOVE FENCE ONLY AS NECESSARY FOR DAILY CONSTRUCTION, FENCE SHALL REMAIN IN PLACE DURING CONSTRUCTION TO PREVENT UNINTENDED IMPACTS.
- 2. THE TREE'S CRITICAL ROOT ZONE SHALL BE AT TREE'S CANOPY DRIP LINE OR A RADIUS OF TWELVE—TIMES THE DIAMETER OF THE TRUNK AT 4.5—FOOT DBH (DIAMETER AT BREAST HEIGHT) WHICHEVER IS LARGER.
- 3. IN THE CRITICAL ROOT ZONE:
- A. DO NOT ALTER OR DISTURB EXISTING GRADE. B. DO NOT STORE ANY CONSTRUCTION MATERIALS, EQUIPMENT, SOIL OR
- C. DO NOT DISPOSE OF ANY LIQUIDS E.G. CONCRETE, GAS, OIL, PAINT
- D. DO NOT PERMIT VEHICLES, EQUIPMENT, OR FOOT TRAFFIC . AVOID TRENCHING.
- F. AVOID CONSTRUCTION ACTIVITY THAT WILL COMPACT THE SOIL. 4. IF CONSTRUCTION WORK DOES ENCROACH INTO THE CRITICAL ROOT ZONE THEN LIMIT ENCROACHMENT TO LESS THAN TWENTY-FIVE PERCENT OF THE TOTAL AREA, AND NO CLOSER TO THE TRUNK THAT ONE-HALF THE RADIUS
- PROTECTIVE MAT OVER THE IMPACTED ROOT AREA. 5. IF TRENCHING IS REQUIRED IN THE ROOT AREA, THEN BORE UNDER THE ROOTING AREA AT A MINIMUM DEPTH OF THIRTY-INCHES. IF A TRENCH FOR AN IRRIGATION HEAD IS NEEDED IN THE ROOT ZONE AREA, TRENCH IN A

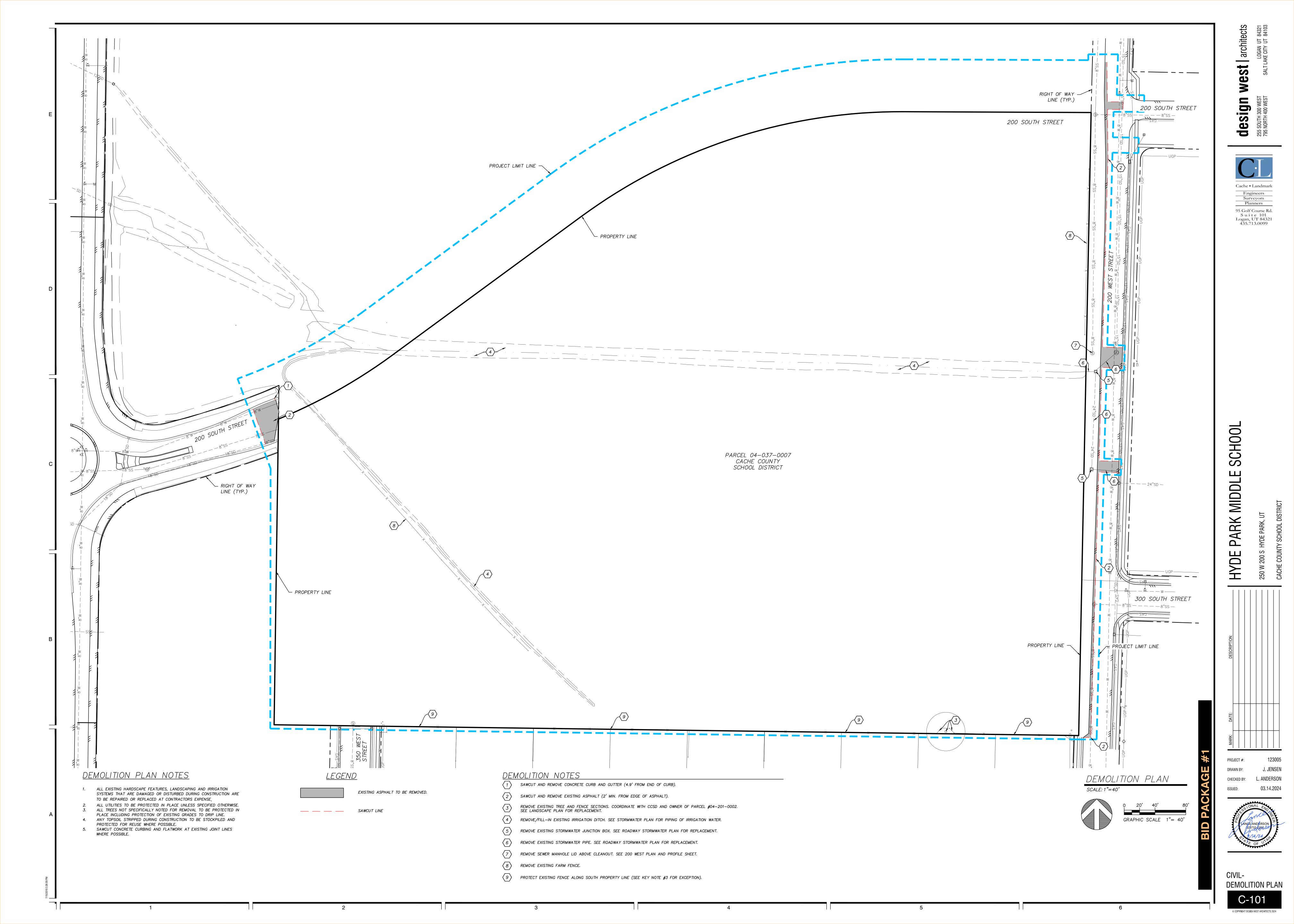
OF THE CRITICAL ROOT ZONE. PROVIDE FIVE INCHES OF MULCH AND A

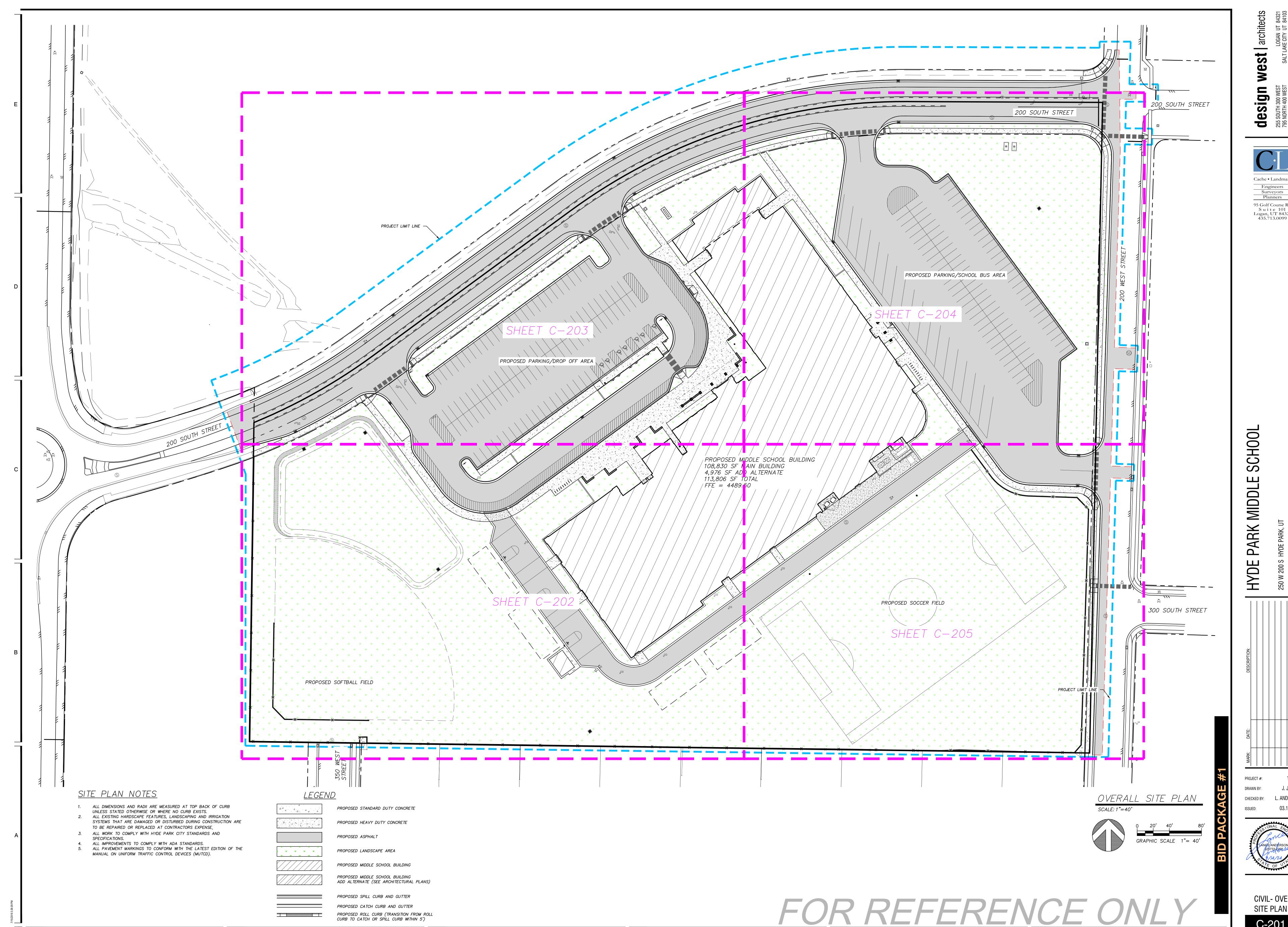
6. PROVIDE WATER TO THE TREE(S) DURING CONSTRUCTION TO MAINTAIN TREE

DIRECT LINE TOWARDS THE TRUNK TO MINIMIZE ROOT DAMAGE.

- 7. REPAIR OR REPLACE TREES AND VEGETATION INDICATED TO REMAIN THAT ARE DAMAGED BY CONSTRUCTION OPERATIONS, IN A MANNER APPROVED BY LANDSCAPE ARCHITECT. A. SUBMIT DETAILS OF PROPOSED REPAIRS TO DAMAGED TREES AND
- B. REPLACE TREES THAT CANNOT BE REPAIRED AND RESTORED TO FULL-GROWTH STATUS, AS DETERMINED BY A QUALIFIED ARBORIST.
- SEE ALSO ARCHITECTURAL PLANS SHEET AS-501/DETAIL 5.

Know what's **below.** Call 811 before you dig. BLUE STAKES OF UTAH UTILITY NOTIFICATION CENTER, INC. www.bluestakes.org 1-800-662-4111

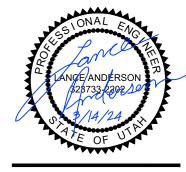




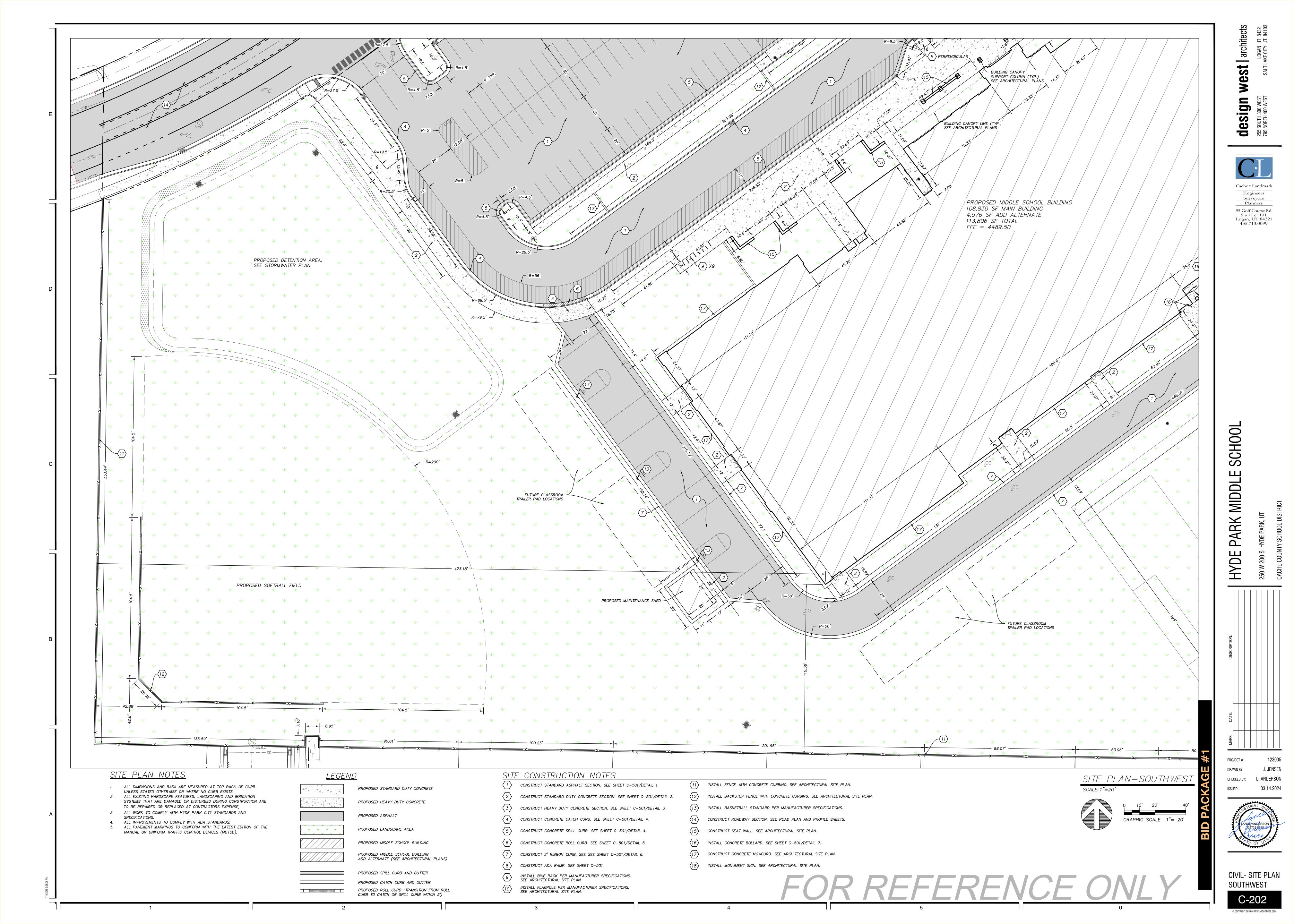
Engineers Surveyors Planners

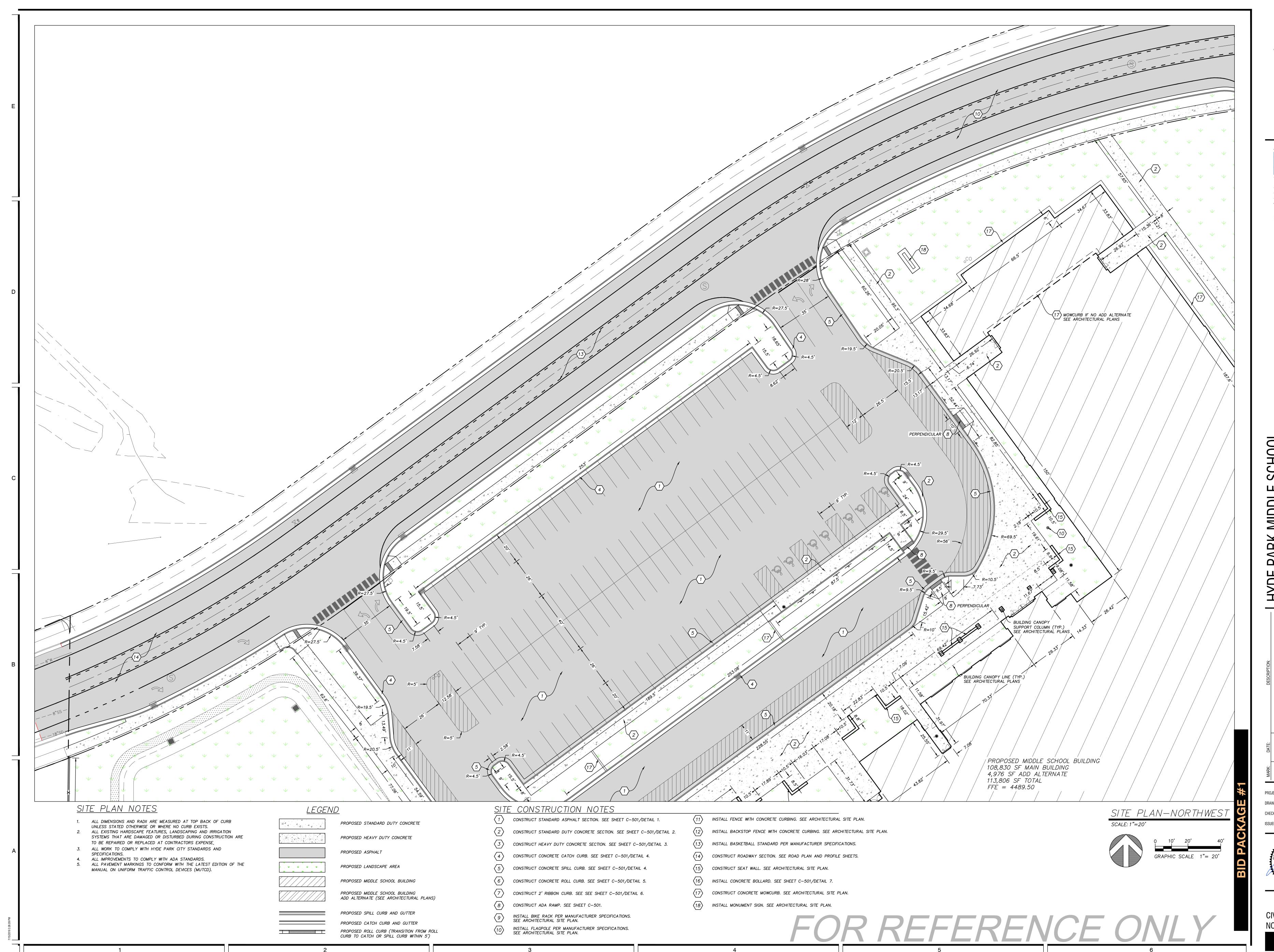
95 Golf Course Rd. S u i t e 101 Logan, UT 84321 435.713.0099

CHECKED BY: L. ANDERSON



CIVIL- OVERALL SITE PLAN





architects

design 255 SOUTH 300 WEST 795 NORTH 400 WEST



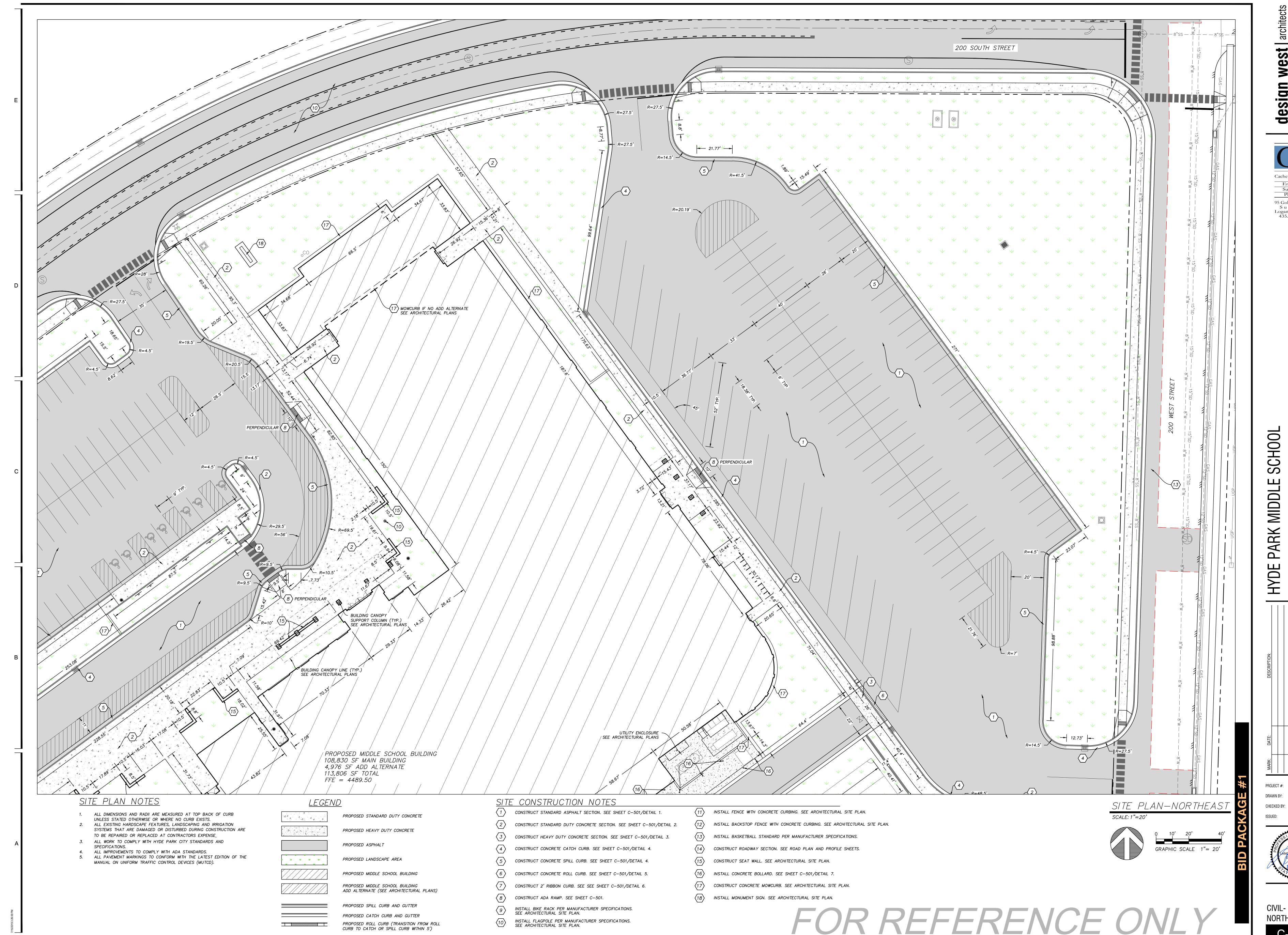


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03.14.2024

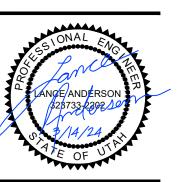


CIVIL- SITE PLAN NORTHWEST



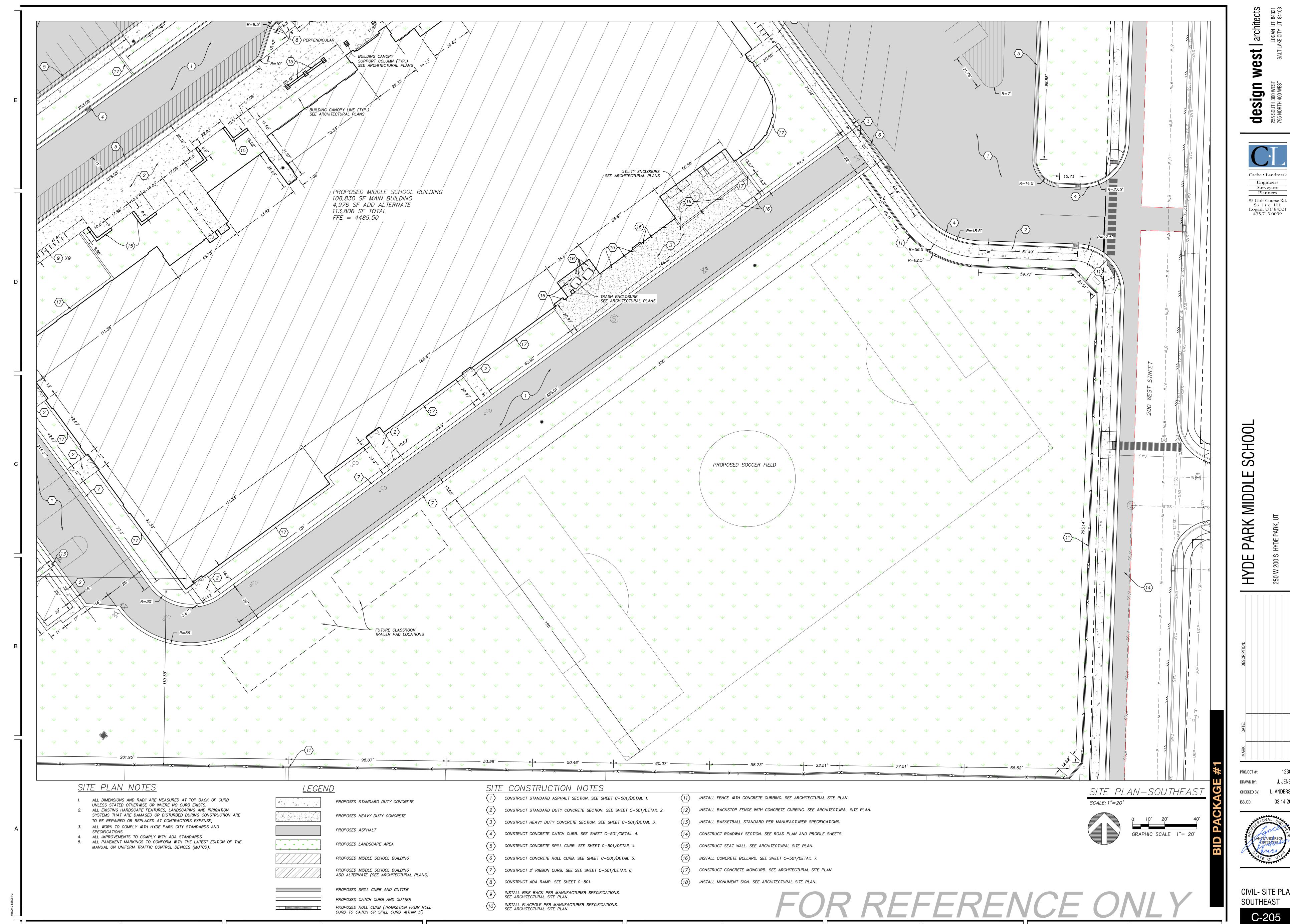


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CIVIL- SITE PLAN NORTHEAST



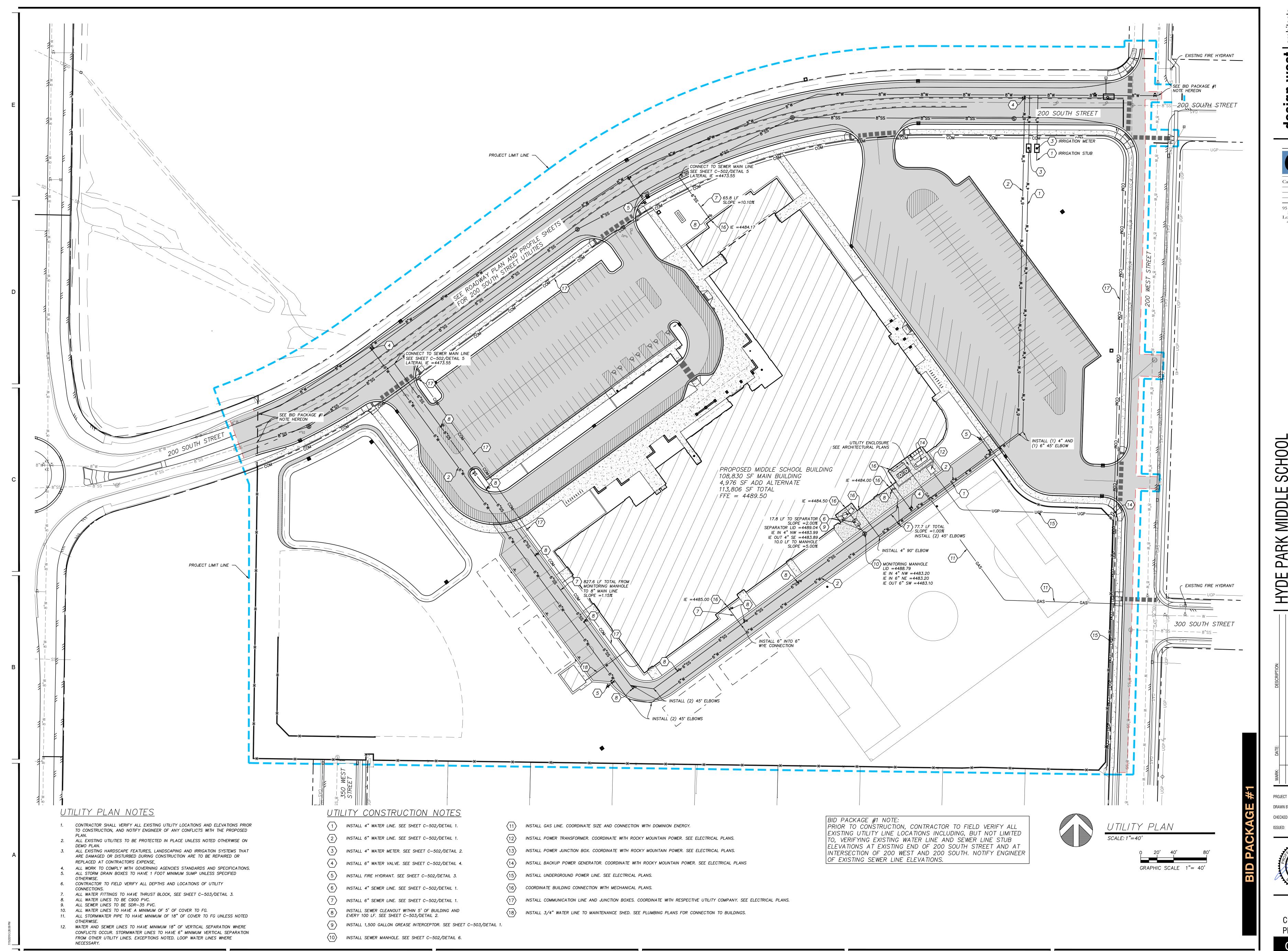




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CIVIL- SITE PLAN SOUTHEAST



LOGAN UT 84321
SALTLAKE CITY UT 84103

SIGN WEST | arc

design W
255 SOUTH 300 WEST



PROJECT #: 123005
DRAWN BY: J. JENSEN
CHECKED BY: L. ANDERSON

SUED: 03.14.2024

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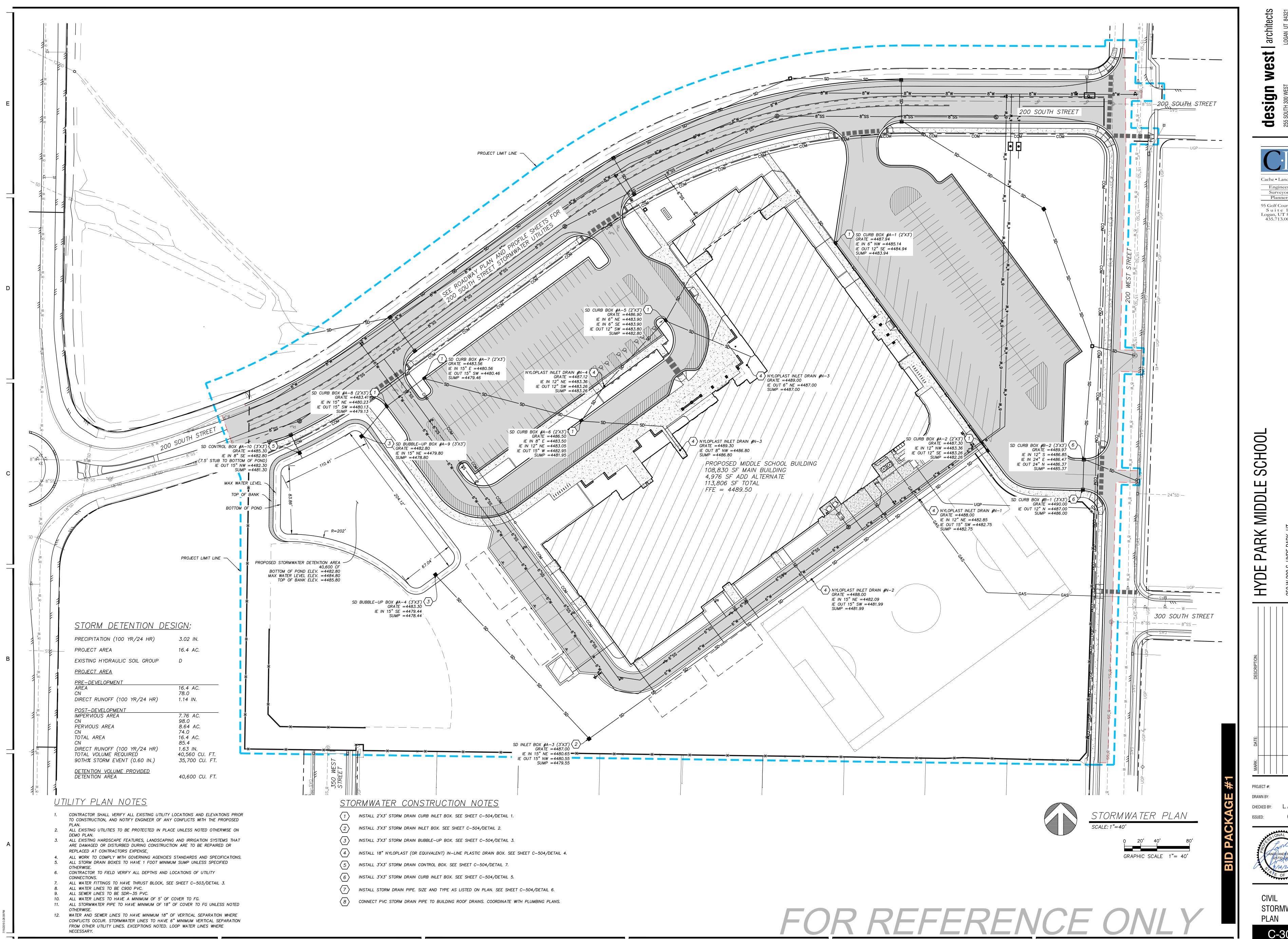
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CIVIL UTILITY PLAN

PLAN C-301



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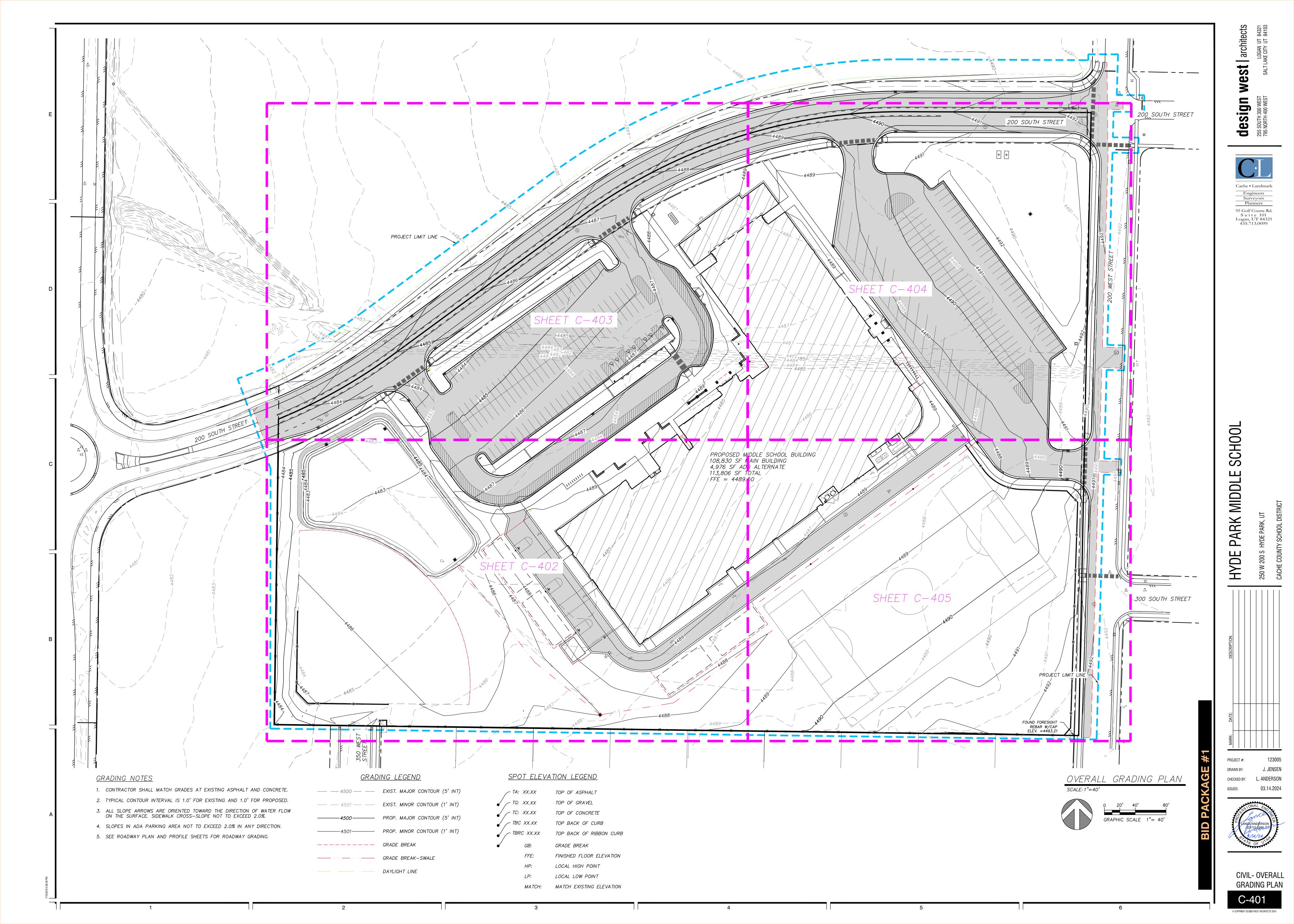
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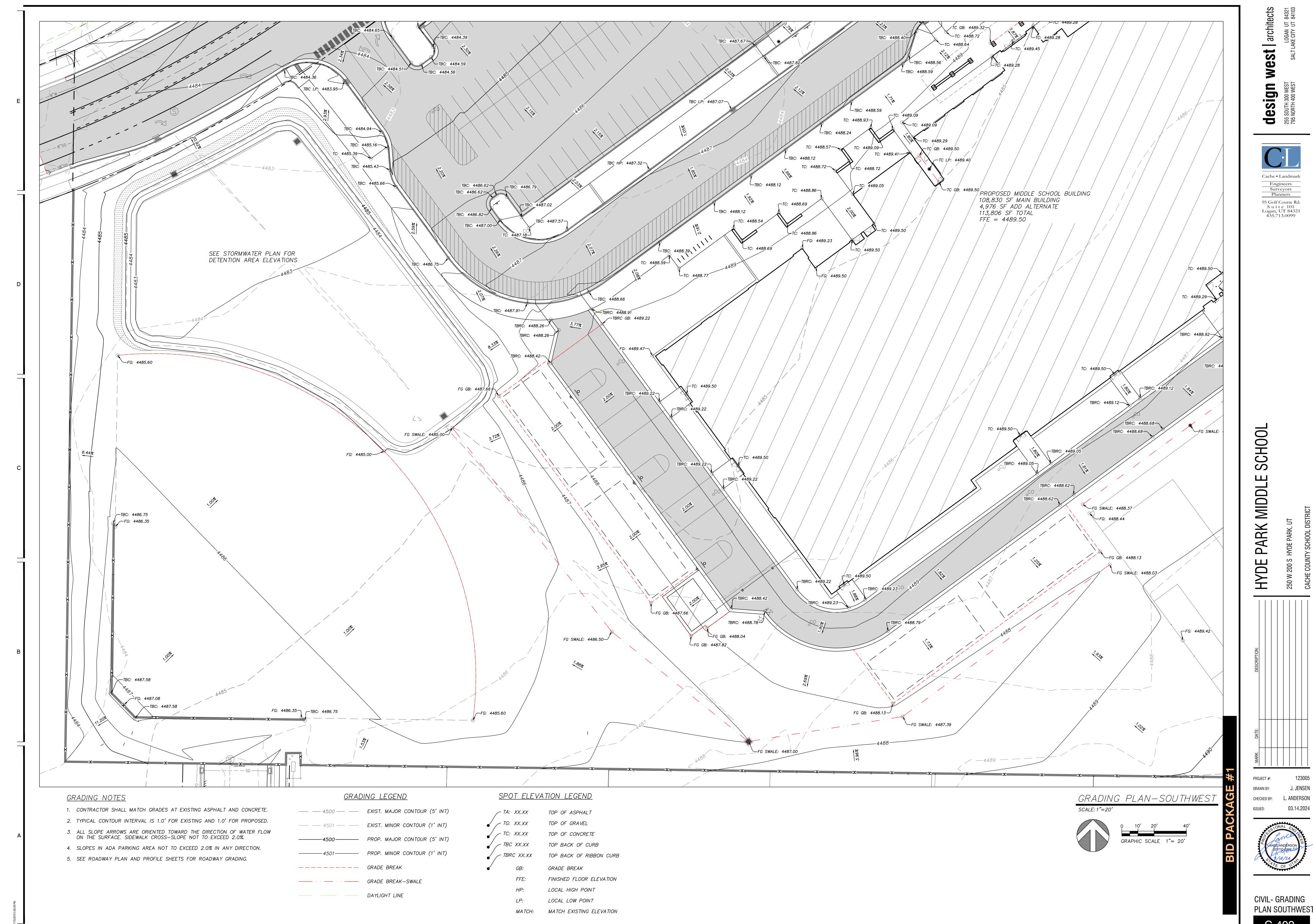
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STORMWATER

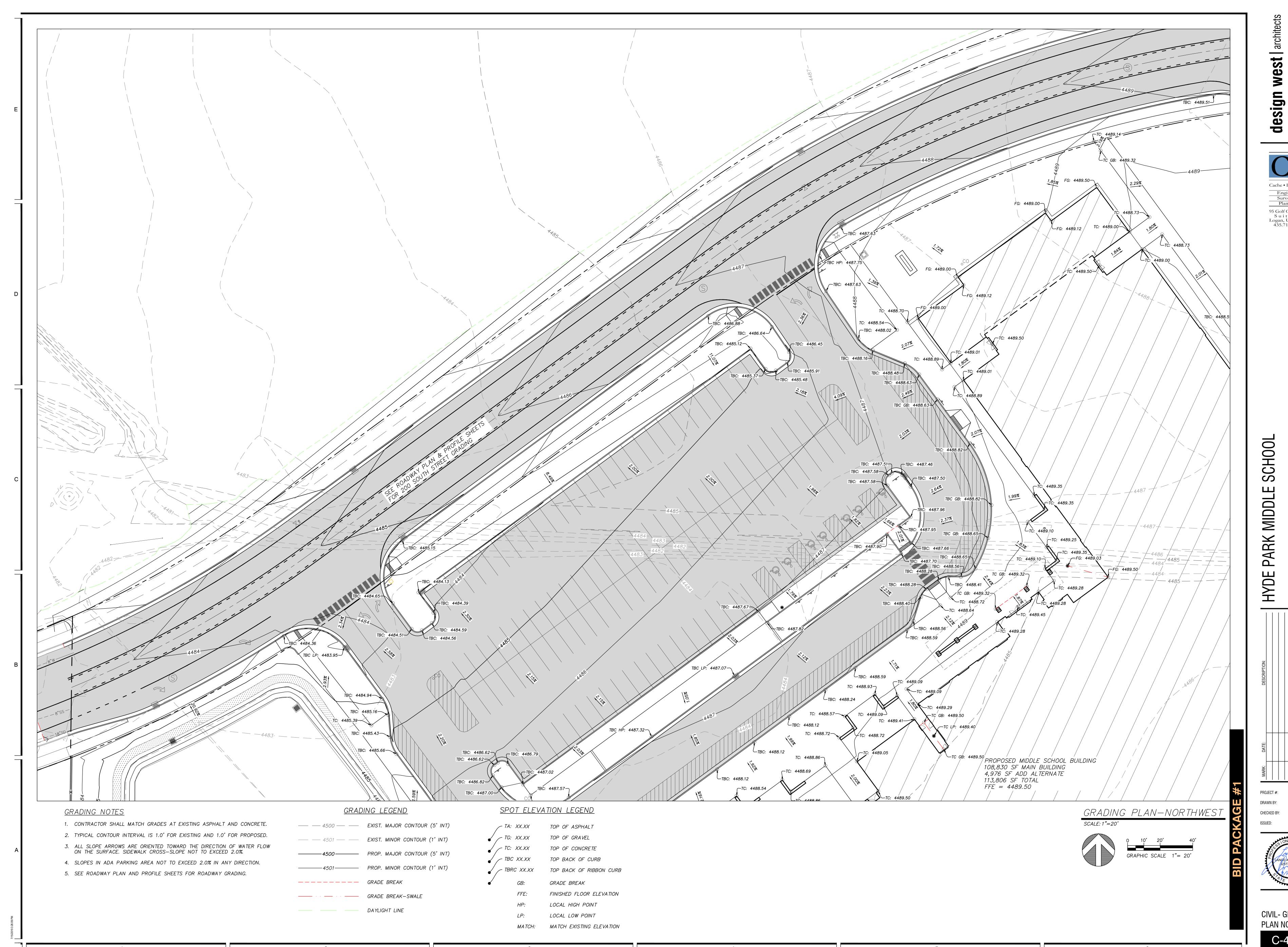
C-302







PLAN SOUTHWEST



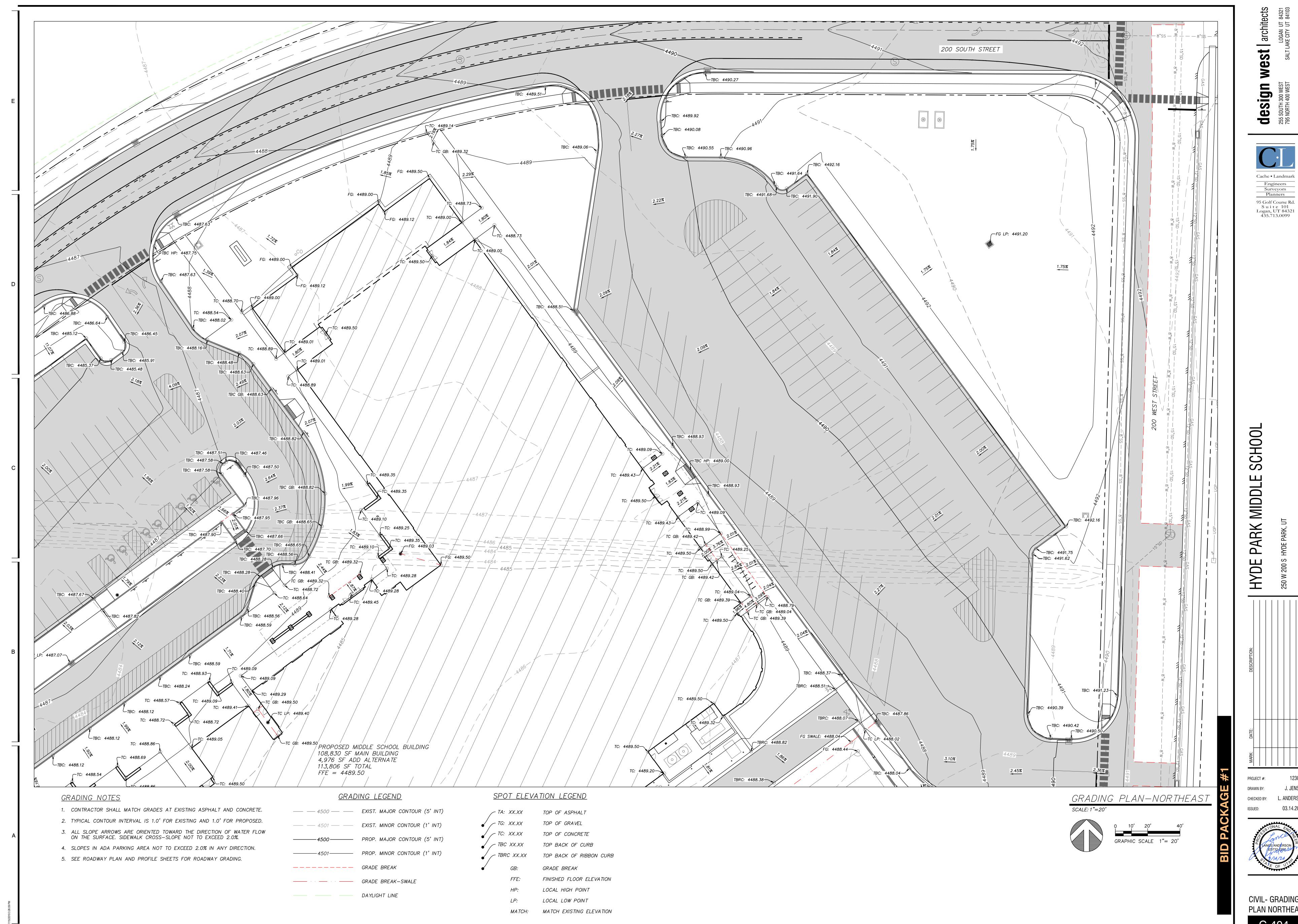
design 255 SOUTH 300 WEST 795 NORTH 400 WEST



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CIVIL- GRADING PLAN NORTHWEST

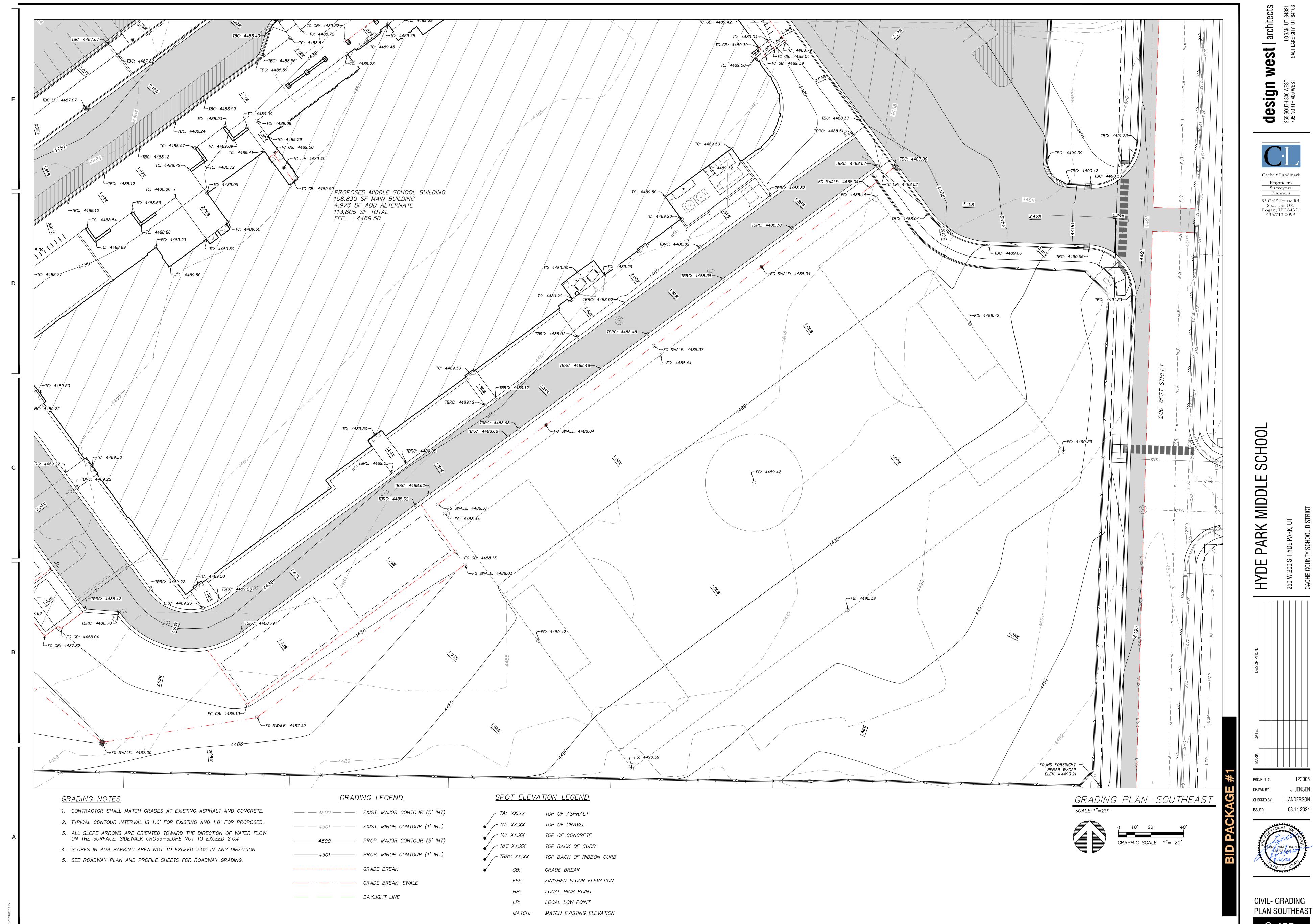


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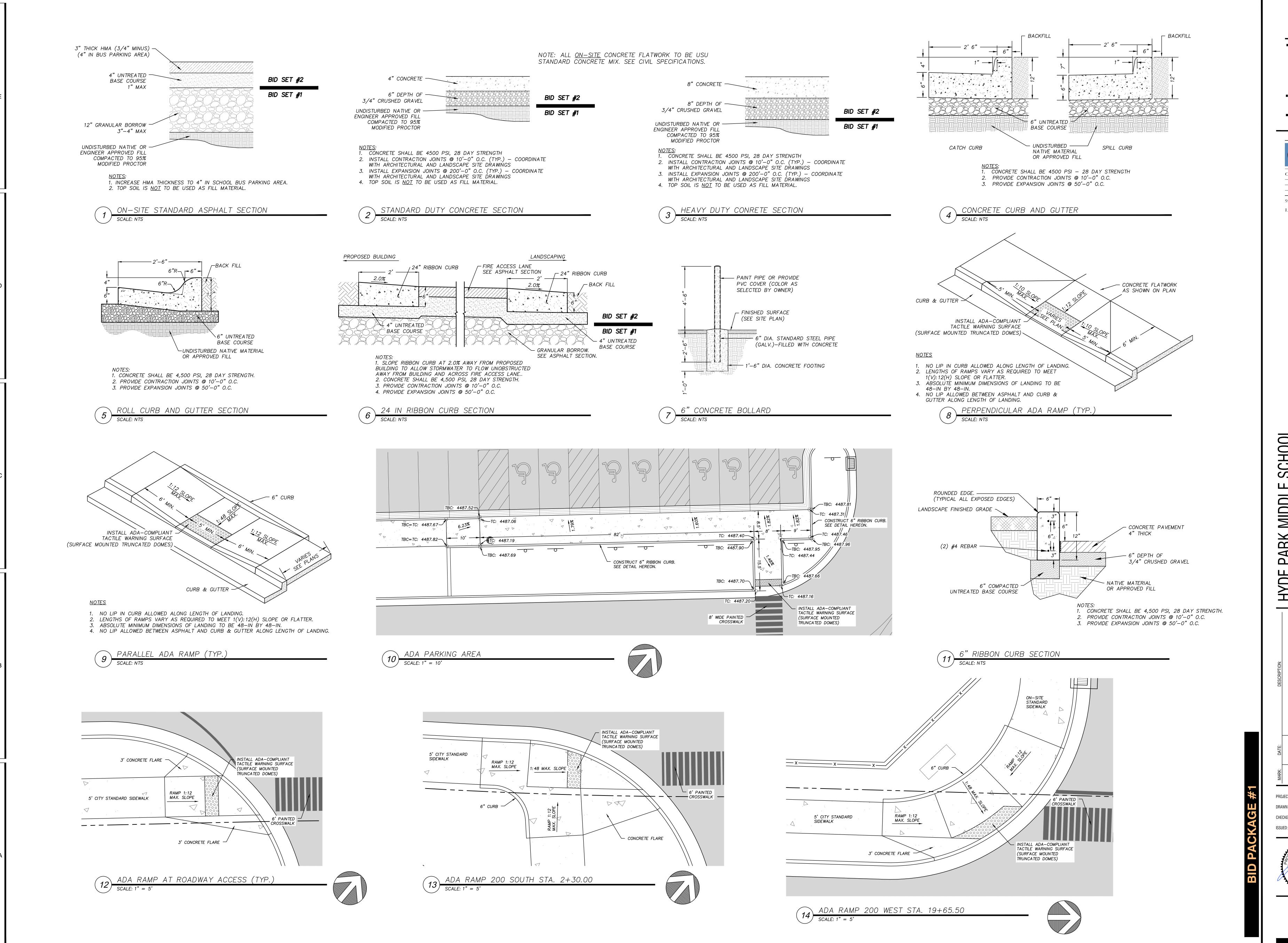
CIVIL- GRADING PLAN NORTHEAST



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CIVIL- GRADING PLAN SOUTHEAST



architects ゖゖ

west

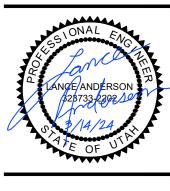
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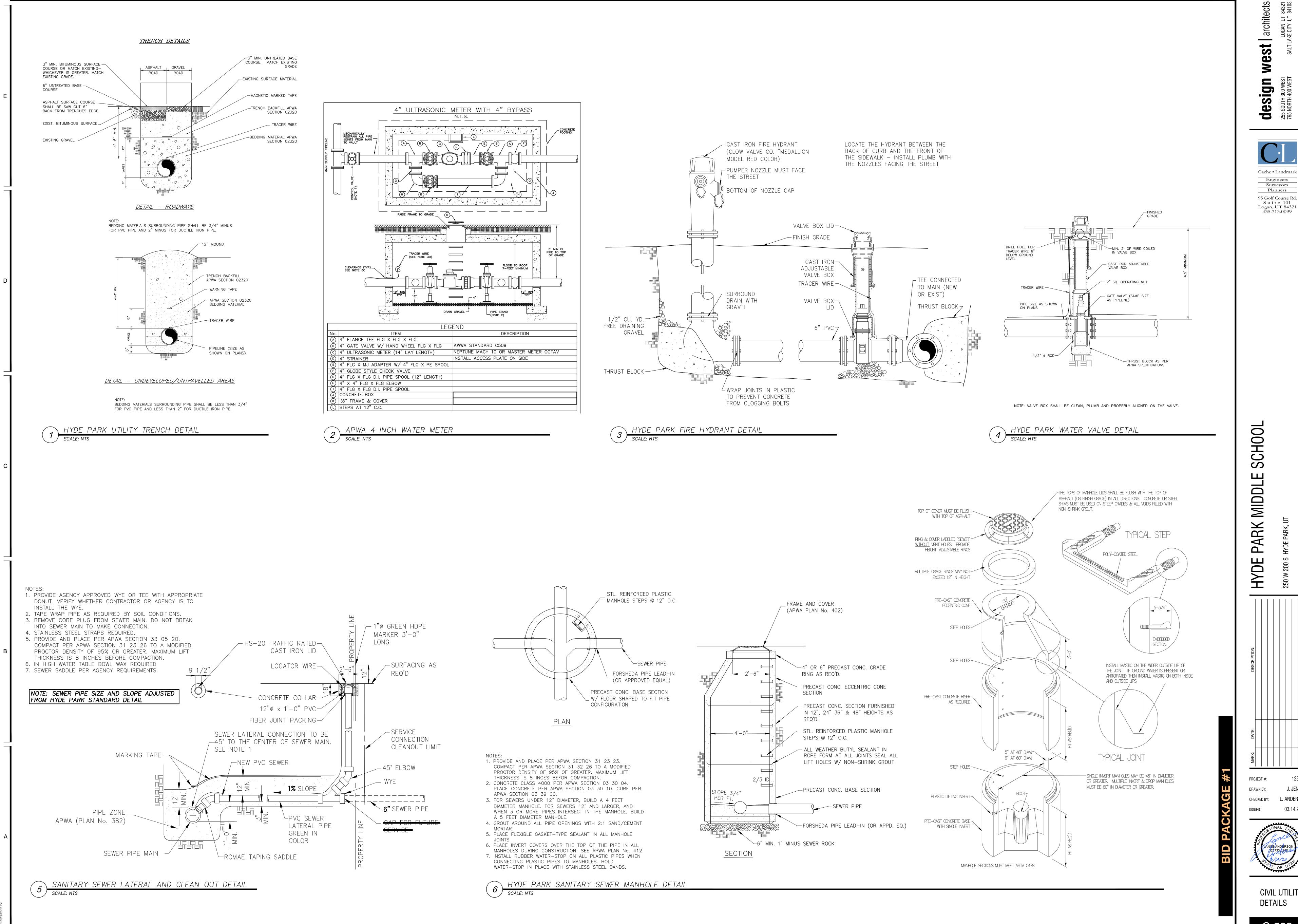
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CIVIL UTILITY **DETAILS** 

C-501



architects 55

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L. ANDERSON

CIVIL UTILITY **DETAILS** 

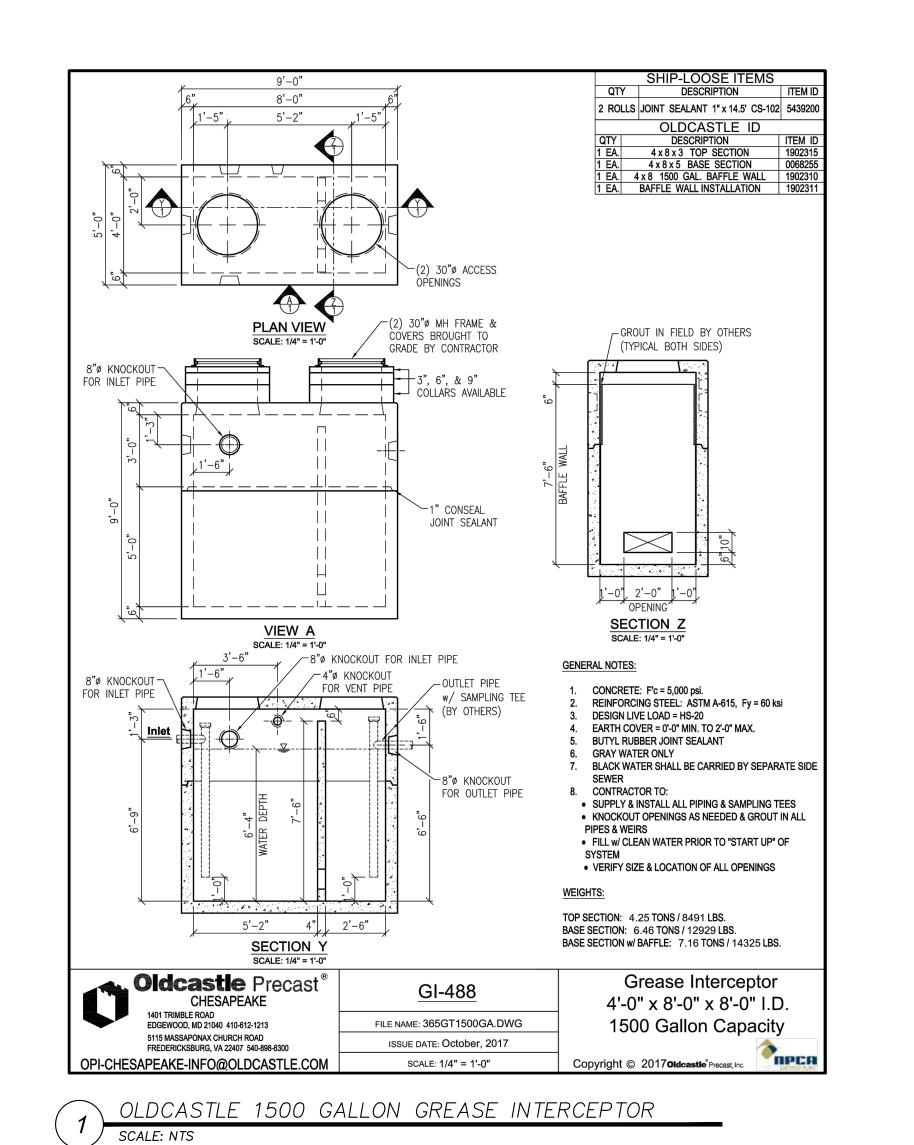
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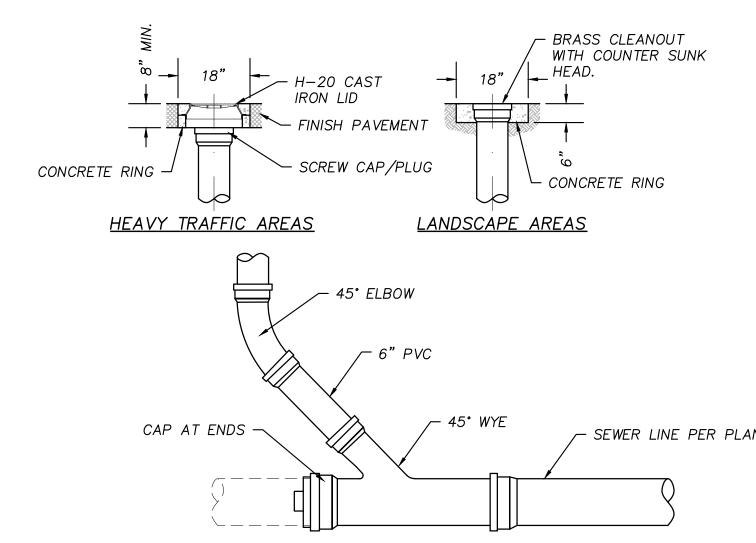
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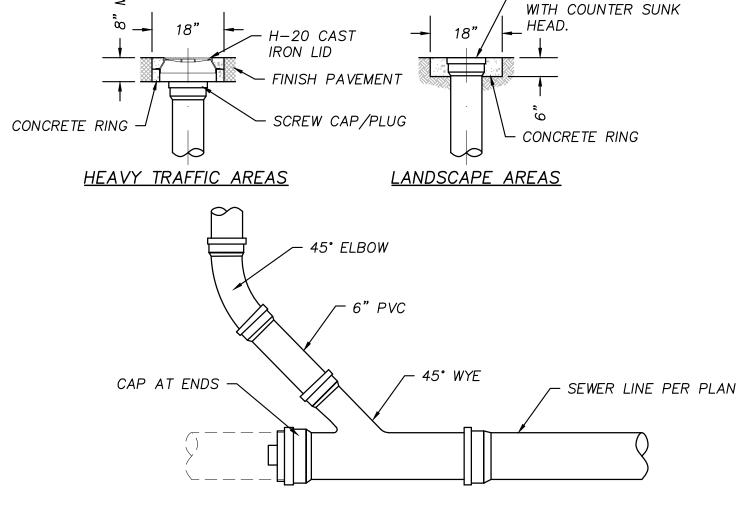
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CIVIL UTILITY **DETAILS** 











REQUIRED TH	IRUST BLOCK A	AREA		
	THR	RUST BLOCK DE	ESIGN INFO	
STATIC PRE	SSURE (PSi)	SOIL BEARING S	STRENGTH (PSF)	SAFETY FACTOR
15	50	20	000	1.50
	THRUST E	BLOCK AREA RI	EQUIRED (SQ F	T)
PIPE SIZE	DEAD END OR TEE	90° ELBOW	45° ELBOW	22.5° ELBOW
4	1.4	2.0	1.1	0.6
6	3.2	4.5	2.4	1.2
8	5.7	8.0	4.3	2.2
10	8.8	12.5	6.8	3.4
12	12.7	18.0	9.7	5.0
14	17.3	24.5	13.2	6.8
16	22.6	32.0	17.3	8.8
18	28.6	40.5	21.9	11.2
20	35.3	50.0	27.0	13.8
24	50.9	71.9	38.9	19.8

THRUST BLOCK DETAIL

SCALE: NTS

USE ONLY REDWOOD  $^{ackslash}$  OR CEDAR TIMBER

AND PIPE.

FOR BLOCKING

BEARING AREA, TYP

1. NO CONCRETE SHALL BE PLACED WITHIN 1

1/2" OF JOINT OR BOLTS. COVER ALL

METAL CONTACT W/ POLY-WRAP PRIOR
TO PLACING CONCRETE

2. THRUST BLOCKS SHALL BE REQUIRED AT

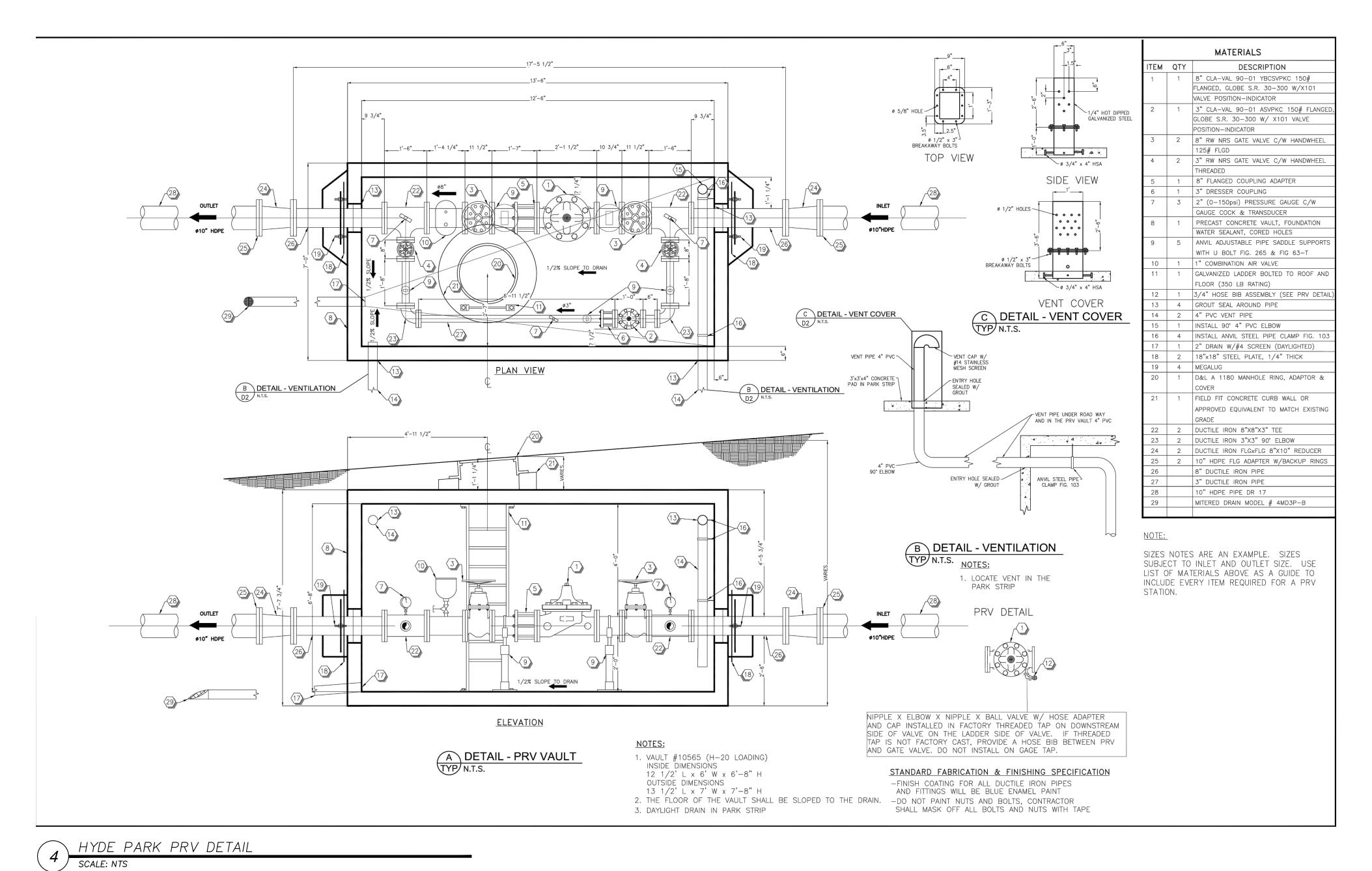
3. PLACE CONCRETE AGAINST UNDISTURBED GROUND IN TRENCH BOTTOM AND SIDES.
4. USE 6 MIL VISQUEEN BETWEEN CONCRETE

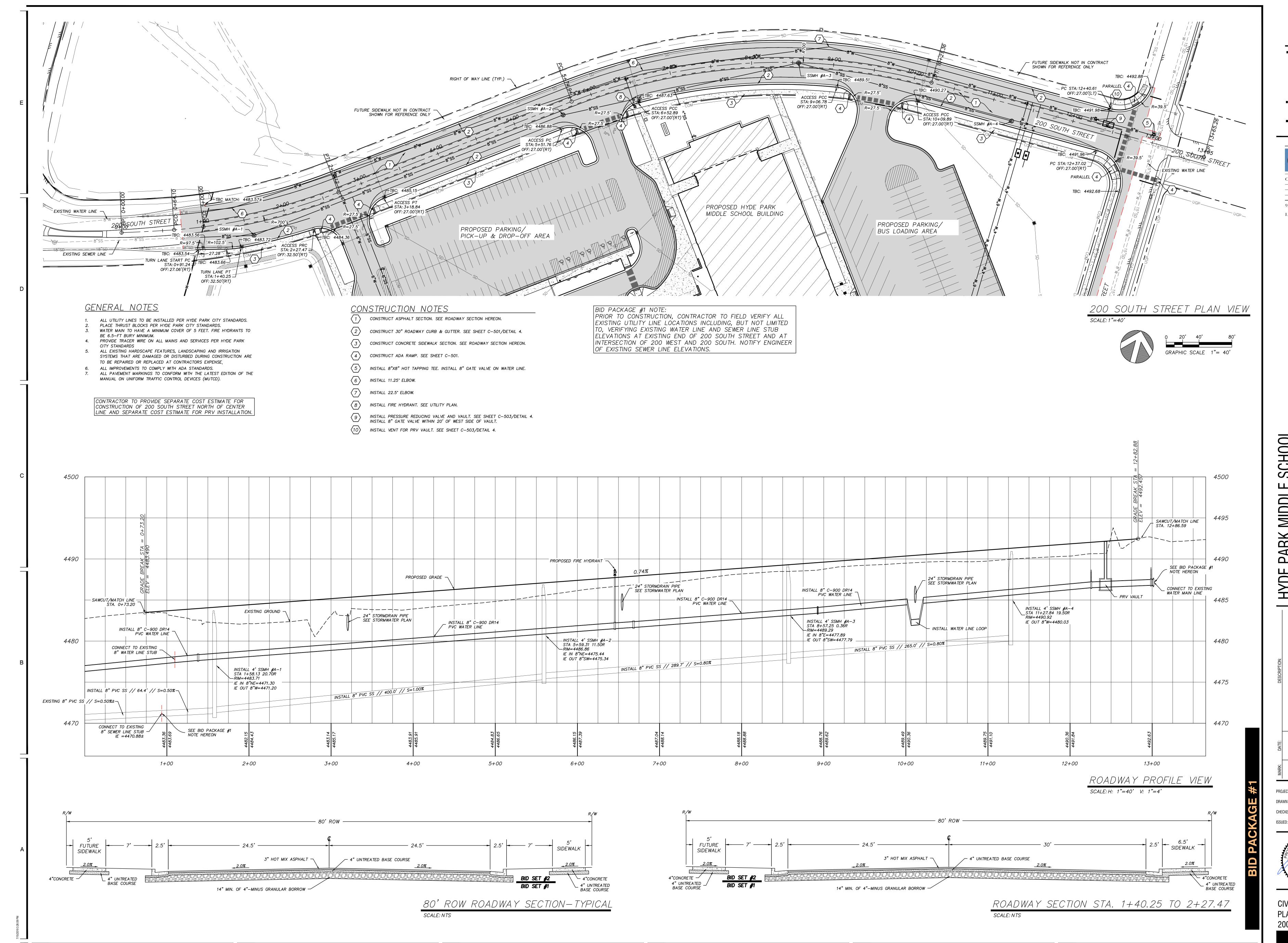
BEARING STRENGTH DIFFER THAN THOSE

LISTED BELOW, CONTACT ENGINEER FOR

ALL 11.25° BENDS OR GREATER

5. IF STATIC PRESSURE AND/OR SOIL





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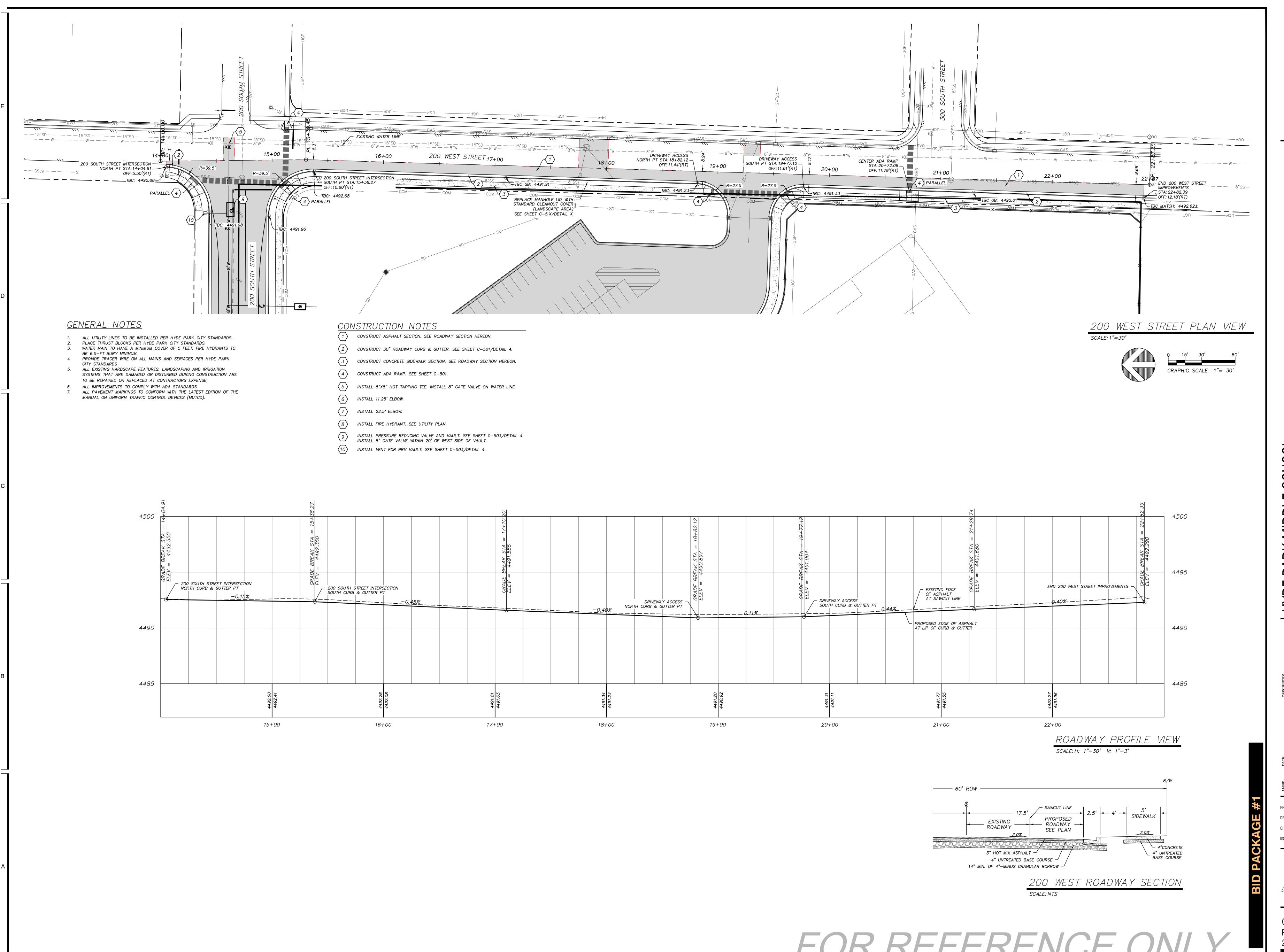
Cache • Landmark Engineers Surveyors Planners 95 Golf Course Rd. Suite 101 Logan, UT 84321 435.713.0099

MIDDI



CIVIL- ROADWAY PLAN & PROFILE 200 SOUTH

C-601



Vest architects
LOGAN UT 84321
SALT LAKE CITY UT 84103

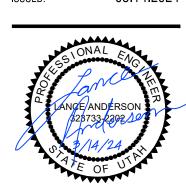
design We 255 SOUTH 300 WEST 795 NORTH 400 WEST



E PARK MIDDLE SCHOOL

DATE: DESCRIPTION:

JECT #: 123005
WN BY: J. JENSEN
CKED BY: L. ANDERSON
ED: 03.14.2024



CIVIL- ROADWAY PLAN & PROFILE 200 WEST

C-602

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**a** 

### A. GENERAL

**STRUCTURAL NOTES:** 

- THE STRUCTURAL NOTES ARE INTENDED TO COMPLEMENT THE PROJECT SPECIFICATIONS WHICH ARE PART OF THE CONSTRUCTION DOCUMENTS. SPECIFIC NOTES AND DETAILS ON THE DRAWINGS SHALL GOVERN OVER THE STRUCTURAL NOTES AND TYPICAL DETAILS. THESE DRAWINGS (AND, WHERE APPLICABLE, ACCOMPANYING WRITTEN SPECIFICATIONS) ARE THE ONLY CONTRACT DOCUMENTS PROVIDED BY ARW ENGINEERS FOR THE PROJECT REPRESENTED
- HEREIN. NOTHING IN ANY DIGITAL MODEL OR DIGITAL FILE RELATED TO THIS PROJECT SHALL BE TAKEN TO SUPERSEDE ANY INFORMATION SHOWN IN THESE DRAWINGS (INCLUDING, BUT NOT LIMITED TO. DIMENSIONS, SIZES, ETC). 3. THE ARCHITECTURAL DRAWINGS ARE THE PRIME CONTRACT DRAWINGS. THE STRUCTURAL DRAWINGS ARE SUPPLEMENTARY TO AND MUST BE USED IN CONJUNCTION WITH THE ARCHITECTURAL DRAWINGS AND OTHER CONSULTANTS DRAWINGS. ALL OMISSIONS OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR SPECIFICATIONS SHALL BE BROUGHT TO THE

ATTENTION OF THE ARCHITECT AND STRUCTURAL ENGINEER BEFORE PROCEEDING WITH ANY WORK

- INVOLVED. IN CASE OF CONFLICT, FOLLOW THE MOST STRINGENT REQUIREMENT AS DIRECTED BY THE ARCHITECT AT NO ADDITIONAL COST TO THE OWNER. 4. SEE SPECIFICATIONS FOR REQUIRED SUBMITTALS. SUBMITTALS SHALL BE MADE IN A TIMELY MANNER AS INDICATED IN SPECIFICATIONS. REVIEW OF SUBMITTALS BY ARW ENGINEERS IS FOR GENERAL COMPLIANCE ONLY AND IS NOT INTENDED AS APPROVAL. CONTRACTOR IS RESPONSIBLE FOR VERIFYING ALL SIZES, DIMENSIONS, AND ELEVATIONS ON SUBMITTALS AS RELATED TO DESIGN DOCUMENTS. PREPARATION OF SHOP DRAWINGS FOR STRUCTURAL ELEMENTS WILL REQUIRE INFORMATION (I.E. DIMENSIONS, ETC.) FOUND IN THE ARCHITECTURAL, STRUCTURAL, AND OTHER
- CONSULTANTS DRAWINGS. THE CONTRACTOR SHALL VERIFY ALL CONDITIONS AND DIMENSIONS AT THE SITE. IF ACTUAL CONDITIONS DIFFER FROM THOSE SHOWN ON CONTRACT DOCUMENTS, CONTRACTOR SHALL NOTIFY
- ARCHITECT PRIOR TO FABRICATION OR CONSTRUCTION OF ANY AFFECTED ELEMENTS. 6. THE CONTRACTOR SHALL COORDINATE AND VERIFY ALL LOCATIONS AND SIZES OF MECHANICAL EQUIPMENT OR OTHER EQUIPMENT BEFORE FABRICATING AND ERECTING STRUCTURAL ELEMENTS. SIZES AND LOCATIONS THAT DIFFER FROM THOSE SHOWN ON THE CONTRACT DOCUMENTS SHALL BE REPORTED TO THE ARCHITECT.
- THE CONTRACTOR SHALL SUBMIT A WRITTEN REQUEST TO THE ARCHITECT FOR ARCHITECT AND/OR ENGINEER APPROVAL BEFORE PROCEEDING WITH ANY CHANGES, MODIFICATIONS, OR
- OBSERVATION VISITS TO THE SITE BY ARW ENGINEERS FIELD REPRESENTATIVES SHALL NEITHER BE CONSTRUED AS INSPECTION NOR APPROVAL OF CONSTRUCTION.
- 9. DURING AND AFTER CONSTRUCTION, BUILDER AND/OR OWNER SHALL KEEP LOADS ON STRUCTURE WITHIN THE LIMITS OF DESIGN LOADS AS NOTED IN THESE DOCUMENTS.
- 10. TYPICAL OR SIMILAR DETAILS AND SECTIONS SHALL APPLY WHERE SPECIFIC DETAILS ARE NOT SHOWN. TYPICAL OR SIMILAR DETAILS REFER TO THE CONDITION ADDRESSED AND ARE NOT NECESSARILY DETAILS LABELED "TYPICAL" OR "SIMILAR" IN THE PLANS AND DOCUMENTS. 11. DRAWINGS AND DETAILS HAVE BEEN PREPARED WITH THE INTENT TO VISUALLY REPRESENT
- PLANS OR DETAILS FOR DIMENSIONAL INFORMATION. 12. THE CONTRACTOR SHALL PROVIDE ADEQUATE TEMPORARY SHORING AND BRACING FOR ALL STRUCTURAL ELEMENTS UNTIL THE ENTIRE STRUCTURAL SYSTEM IS COMPLETED. DESIGN OF ALL

INFORMATION PROVIDED IN SCALED FORM; HOWEVER CONTRACTOR/SUPPLIERS SHOULD NOT SCALE

- SHORING AND BRACING IS BY OTHERS AT NO ADDITIONAL COST TO THE OWNER. 13. ENGINEER SHALL NOT BE RESPONSIBLE FOR ACTIVITIES UNDER CONTROL OF THE CONTRACTOR SUCH AS CONSTRUCTION SITE SAFETY, MEANS, METHODS AND SEQUENCING OF CONSTRUCTION. ENGINEER SHALL NOT BE RESPONSIBLE FOR FABRICATION, ERECTION AND CONSTRUCTION REQUIREMENTS AS PRESCRIBED BY OSHA OR OTHER REGULATORY AGENCIES REGARDLESS OF INDICATIONS IN THESE
- 14. NOTICE OF COPYRIGHT: THESE STRUCTURAL DRAWINGS ARE HEREBY COPYRIGHTED BY ARW ENGINEERS, ALL RIGHTS RESERVED. THESE DOCUMENTS DEFINE A STRUCTURE AND ARE INSTRUMENTS OF SERVICE, FOR ONE USE ONLY. REPRODUCTION AND DISTRIBUTION OF THESE DRAWINGS IS ONLY ALLOWED AS REQUIRED FOR REGULATORY AGENCIES AND FOR CONVEYANCE OF INFORMATION TO PARTIES INVOLVED IN THE CONSTRUCTION OF THIS PROJECT. THESE DOCUMENTS SHALL NOT BE REPRODUCED OR COPIED, IN PART OR WHOLE BY ANY PARTY FOR USE IN
- PREPARATION OF SHOP DRAWINGS OR OTHER SUBMITTALS. 15. WHERE THE WORD "SHALL" OCCURS IN THESE DRAWINGS AND ANY ACCOMPANYING SPECIFICATIONS. IT IS CONSIDERED A MANDATORY OBLIGATION AND SYNONYMOUS WITH THE PHRASE "HAS DUTY TO".

# B. STATEMENT OF SPECIAL INSPECTIONS AND SPECIAL INSPECTIONS

- 1. THE DESIGNATED SEISMIC/WIND SYSTEMS AND SEISMIC/WIND-FORCE-RESISTING SYSTEMS THAT ARE SUBJECT TO SPECIAL INSPECTIONS IN ACCORDANCE WITH IBC SECTION 1705.12 AND 1705.13 ARE IDENTIFIED ON THESE DOCUMENTS WITH A CIRCLE "L". ALL OTHER ITEMS REQUIRING SPECIAL INSPECTION ARE IDENTIFIED IN THE SPECIAL INSPECTION SCHEDULE ON SHEETS S-012 AND S-013. SPECIAL INSPECTIONS AND TESTING ARE TO BE PROVIDED AS REQUIRED BY IBC SECTIONS 1704
- THROUGH 1705 AND OTHER APPLICABLE SECTIONS OF THE IBC. THE TYPE AND FREQUENCY OF TESTING AND SPECIAL INSPECTIONS SHALL BE AS NOTED IN THE SPECIAL INSPECTION SCHEDULE. JOB SPECIFICATIONS, AND ACCORDANCE WITH IBC SECTION 110 AND CHAPTER 17. CONTRACTOR SHALL COORDINATE AND COOPERATE WITH REQUIRED INSPECTIONS. 3. ALL TESTING AND SPECIAL INSPECTION SHALL BE PROVIDED BY A QUALIFIED INDEPENDENT SPECIAL
- INSPECTION AGENCY IN ACCORDANCE WITH IBC 1704 AND AS OUTLINED IN THE JOB SPECIFICATIONS. REPORTS OF FINDINGS OR DISCREPANCIES SHALL BE NOTED AND FORWARDED TO THE CONTRACTOR, ARCHITECT, ENGINEERS, AND BUILDING OFFICIAL IN A TIMELY MANNER.
- STRUCTURAL OBSERVATION VISITS SHALL BE PERFORMED BY A REPRESENTATIVE FROM ARW ENGINEERS IN ACCORDANCE WITH THE CONTRACT AS NEEDED TO OBSERVE THE CONSTRUCTION OF CRITICAL BUILDING ELEMENTS (I.E. FOOTINGS, BRACED FRAMES, MOMENT FRAMES, DRAG STRUTS AND THEIR CONNECTIONS, COLLECTORS, AND ROOF AND FLOOR DIAPHRAGMS). STRUCTURAL OBSERVATION REPORTS FOR EACH VISIT SHALL BE SENT DIRECTLY TO THE ARCHITECT FOR DISTRIBUTION TO THE CONTRACTOR AND BUILDING OFFICIAL. STRUCTURAL OBSERVATION VISITS SHALL NEITHER BE CONSTRUED AS SPECIAL INSPECTION NOR APPROVAL OF COMPLETED CONSTRUCTION
- 5. IN ACCORDANCE WITH IBC 1704.4, THE CONTRACTOR SHALL SUBMIT A WRITTEN CONTRACTOR'S STATEMENT OF RESPONSIBILITY TO THE BUILDING OFFICIAL AND OWNER. THE STATEMENT SHALL BE SUBMITTED PRIOR TO THE CONSTRUCTION OF ANY SEISMIC/WIND-FORCE-RESISTING SYSTEM. DESIGNATED SEISMIC/WIND SYSTEM. OR COMPONENT IDENTIFIED IN THESE DOCUMENTS WITH A

# C. BASIS OF DESIGN

- 1. GOVERNING BUILDING CODE: INTERNATIONAL BUILDING CODE (IBC) 2021 RISK CATEGORY: III SUSPENDED FLOOR LOADS
- a. LIVE LOAD = 40 PSF AT CLASSROOMS, 80 PSF AT CORRIDORS (UNREDUCED) b. DEAD LOAD = 70 PSF
- ROOF LOADS a. FLAT-ROOF SNOW LOAD, Pf: 33 PSF
  - 1. GROUND SNOW LOAD, Pg: 43 PSF 2. SNOW EXPOSURE FACTOR, C<sub>e</sub>: 1.0
  - SNOW LOAD IMPORTANCE FACTOR, Is: 1.1 THERMAL FACTOR, Ct: 1.0
  - 5. SLOPE FACTOR, C<sub>S</sub>: 1.0 6. SNOW DRIFT: SHOWN ON PLANS WHERE APPLICABLE.
- b. LIVE LOAD = 20 PSF c. DEAD LOAD = 20 PSF 4. WIND DESIGN
- a. BASIC WIND SPEED (3 SECOND GUST): 110 MPH b. WIND EXPOSURE: C
- c. INTERNAL PRESSURE COEFFICIENT, GCPI: 0.18
- d. COMPONENT AND CLADDING DESIGN WIND PRESSURE SHALL BE AS REQUIRED PER ASCE 7-16. 5. SEISMIC DESIGN:
- a. SEISMIC IMPORTANCE FACTOR, I<sub>E</sub>: 1.25
- b. SITE CLASS : E MAPPED SPECTRAL RESPONSE ACCELERATIONS:  $S_S = 1.079$ ,  $S_1 = 0.363$
- d. SPECTRAL RESPONSE COEFFICIENTS:  $S_{DS} = 0.863$ ,  $S_{D1} = 0.617$ e. SEISMIC DESIGN CATEGORY: D
- f. BASIC SEISMIC-FORCE-RESISTING SYSTEM: SPECIAL REINFORCED CONCRETE/MASONRY SHEARWALLS
- SEISMIC RESPONSE COEFFICIENT, Cs: .216 RESPONSE MODIFICATION FACTOR, R: 5 i. ANALYSIS PROCEDURE: EQUIVALENT LATERAL FORCE

# D. FOUNDATION

- GENERAL a. DESIGN SOIL PRESSURE: 3000 PSF
- b. SOILS REPORTS BY: A CACHE CORP REPORT #: 1220008 (HYDE PARK SITE) DATED NOVEMBER 12, 2022
- REPORT #: 1230024 (NIBLEY SITE) DATED DECEMBER 11, 2023 c. SOIL PREPARATION UNDER FOUNDATIONS AND SLABS-ON-GRADE SHALL BE IN ACCORDANCE WITH
- THE SOILS REPORT d. TOP OF FOOTING ELEVATIONS SHOWN ON THE FOOTING AND FOUNDATION PLAN ARE BASED ON PRELIMINARY GRADING INFORMATION AND SHALL BE VERIFIED PRIOR TO CONSTRUCTION. STEPS WHERE SHOWN ARE AT APPROXIMATE LOCATIONS. ACTUAL STEP LOCATIONS SHALL BE AT THE
- CONTRACTOR'S DISCRETION BASED UPON FIELD CONDITIONS. ALL EXTERIOR FOUNDATIONS SHALL BEAR A MINIMUM OF 30" INCHES BELOW LOWEST ADJACENT FINAL GRADE. e. ALL WALLS (EXCEPT CANTILEVERED RETAINING WALLS) SHALL BE ADEQUATELY BRACED AGAINST LATERAL MOVEMENT PRIOR TO BACKFILLING. DESIGN AND ERECTION OF BRACING/SHORING SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR. BRACING SHALL REMAIN IN PLACE UNTIL
- SUPPORTING STRUCTURAL ELEMENTS ARE IN PLACE AND HAVE ATTAINED FULL STRENGTH. UNLESS NOTED OTHERWISE, ALL FOOTINGS AT COLUMNS SHALL BE CENTERED BELOW COLUMNS. UNLESS NOTED OTHERWISE, ALL FOOTINGS SHALL HAVE VERTICAL FACES FORMED WITH STANDARD FORMING MATERIALS (WOOD, METAL, ETC.). WITH PRIOR APPROVAL OF ARCHITECT AND ENGINEER, CONCRETE FOR FOOTINGS CAN BE PLACED IN EXCAVATED SOIL "FORMS" PROVIDED THAT THE DIMENSIONS ARE INCREASED 3" ON ALL SIDE.

#### E. CONCRETE

- 1. ALL CONCRETE MIX DESIGNS SHALL COMPLY WITH THE PROJECT SPECIFICATIONS AND THE
- REQUIREMENTS LISTED BELOW: a. FOOTINGS, GRADE BEAMS, FOUNDATION WALLS: 1. WHERE THE TOP OF THE ELEMENT IS EXPOSED OR IS LOCATED WITHIN 30" OF THE LOWEST ADJACENT GRADE (EXPOSURE CATEGORY F2):
  - a. 28 DAY COMPRESSIVE STRENGTH: 4500 PSI b. MAXIMUM W/C RATIO: c. MAXIMUM AGGREGATE SIZE :
- d. AIR CONTENT: SEE SCHEDULE BELOW 2. WHERE THE TOP OF THE ELEMENT IS NOT EXPOSED OR IS NOT LOCATED WITHIN 30" OF THE LOWEST ADJACENT GRADE (EXPOSURE CATEGORY F0): a. 28 DAY COMPRESSIVE STRENGTH: 3000 PSI
- b. INTERIOR SLABS ON GRADE (EXPOSURE CATEGORY F0): 1. 28 DAY COMPRESSIVE STRENGTH: 3000 PSI c. INTERIOR SUSPENDED SLABS (EXPOSURE CATEGORY F0):
- 1. 28 DAY COMPRESSIVE STRENGTH: 3000 PSI
- 1. 28 DAY COMPRESSIVE STRENGTH: 4500 PSI e. TOTAL AIR CONTENT FOR CONCRETE EXPOSED TO CYCLES OF FREEZING AND THAWING SHALL BE DETERMINED IN ACCORDANCE WITH THIS SCHEDULE. TOLERANCE ON AIR CONTENT AS DELIVERED SHALL BE +/- 1.5 PERCENT. NOMINAL MAXIMUM TARGET AIR CONTENT, PERCENT
- AGGREGATE SIZE, IN. F2 AND F3
- 2. WATER USED IN MIXING CONCRETE SHALL CONFORM TO ASTM C1602 3. NO CONDUIT, PIPES, DUCTS, SLEEVES, ETC. SHALL BE PLACED IN STRUCTURAL CONCRETE UNLESS SPECIFICALLY DETAILED OR APPROVED BY THE STRUCTURAL ENGINEER. NO ALUMINUM PRODUCTS SHALL BE EMBEDDED IN CONCRETE. PENETRATIONS THRU STRUCTURAL CONCRETE ELEMENTS MUST BE APPROVED BY THE ENGINEER AND SHALL BE BUILT INTO THE ELEMENT PRIOR TO CONCRETE
- 4. REFER TO ARCHITECTURAL DRAWINGS FOR MOLDS, GROOVES, ORNAMENTS, ETC. TO BE CAST IN TO CONCRETE, AND FOR EXTENT AND LOCATION OF DEPRESSIONS, CURBS, RAMPS, ETC. 5. UNLESS NOTED OTHERWISE, MINIMUM REINFORCING IN ALL CONCRETE FOUNDATION WALLS SHALL BE AS FOLLOWS:
- THICKNESS BOTTOM BARS VERTICAL HORIZONTA #4 AT 18"O.C. #4 AT 12"O.C. #4 AT 18"O.C. EACH FACE #4 AT 12"O.C. EACH FACE UNLESS NOTED OTHERWISE, CONCRETE SLABS ON EARTH SHALL BE UNREINFORCED AS FOLLOWS: WHERE REINFORCING IS PROVIDED REINFORCING SHALL BE CONTINUOUSLY SUPPORTED AT 36"O.C.
- MAXIMUM SPACING. 6. UNLESS NOTED OTHERWISE, FOR NON-DETAILED OPENINGS IN CONCRETE WALLS LARGER THAN 12" AND SMALLER THAN 24" IN ANY DIRECTION ADD (2) #5 BARS ON ALL SIDES IN ADDITION TO REGULAR WALL REINFORCING AND EXTEND 24" EACH WAY BEYOND OPENING. IF 24" IS NOT AVAILABLE ON EVERY SIDE, NOTIFY STRUCTURAL ENGINEER FOR FURTHER DIRECTION. OPENINGS SHALL HAVE A MINIMUM OF 12" OF CONCRETE ABOVE THE OPENING, TYP.
- 7. CONSTRUCTION JOINTS NOT SHOWN ON THE PLANS SHALL BE MADE AND LOCATED SO AS TO NOT IMPAIR THE STRENGTH OF THE STRUCTURE AND AS APPROVED BY THE STRUCTURAL ENGINEER. PROVIDE 2 X 4 (SHAPED) KEYWAY IN ALL VERTICAL AND HORIZONTAL JOINTS UNLESS NOTED OR DETAILED OTHERWISE. ALL STEEL REINFORCING SHALL BE CONTINUOUS THROUGH COLD JOINTS UNLESS NOTED OTHERWISE. SEE TYPICAL DETAILS FOR COLD/CONSTRUCTION JOINTS FOR SLABS ON
- 8. WHERE NEW CONCRETE IS PLACED AGAINST PREVIOUSLY HARDENED CONCRETE, THE JOINT SHALL BE CLEAN AND FREE OF LAITANCE. IMMEDIATELY BEFORE NEW CONCRETE IS PLACED, CONSTRUCTION JOINTS SHALL BE PREWETTED AND STANDING WATER REMOVED.

### F. ANCHOR BOLTS/EMBEDDED BOLTS

PLACEMENT.

- 1. ALL ANCHOR BOLTS SHALL HAVE ASTM A-563 HEAVY HEX NUT AND ASTM F-436 WASHERS AT STANDARD OR OVERSIZED HOLES PER AISC SPECIFICATION TABLE J3.3. WHERE HOLE SIZES DO NOT COMPLY WITH THE LIMITATIONS FOR OVERSIZED HOLES THE STRUCTURAL ENGINEER SHALL BE NOTIFIED TO DETERMINE STEEL PLATE WASHER REQUIREMENTS. ANCHOR BOLTS SHALL COMPLY a. AT ALL ANCHOR BOLTS (UNLESS NOTED OTHERWISE) - ASTM F1554 GRADE 36 HEADED BOLTS.
- (ASTM A36 THREADED ROD MAY BE USED WITH DOUBLE NUT AND WASHER.) 2. EMBEDDED BOLTS IN MASONRY SHALL BE (UNLESS NOTED OTHERWISE) ASTM A-307 GRADE HEADED 3. SEE TYPICAL ANCHOR BOLT DETAIL FOR DEFINITIONS OF EMBEDMENT LENGTH, ETC
- 4. FURNISH TEMPLATES AND OTHER DEVICES AS NECESSARY FOR PRESETTING ALL BOLTS PRIOR TO PLACING CONCRETE AND/OR GROUT. 5. IF THREADED RODS ARE USED AS PERMITTED ABOVE, THEY SHALL BE CLEAR OF SOIL AND DIRT.

### 6. WHERE REQUIRED FOR ERECTION, HOLES LARGER THAN OVERSIZED MAY BE PERMITTED WITH THE USE OF STEEL PLATE WASHERS AT THE DISCRETION OF THE STRUCTURAL ENGINEER.

# G. ADHESIVE/MECHANICAL ANCHORS

- 1. WITHOUT WRITTEN APPROVAL OF THE ENGINEER, CONTRACTOR SHALL NOT SUBSTITUTE POST-INSTALLED ANCHORS WHERE CAST-IN-PLACE ANCHORS ARE SPECIFIED IN THE DRAWINGS. 2. WHERE STRUCTURAL DETAILS SPECIFY SPECIFIC BRANDS AND/OR TYPES OF ADHESIVES OR ANCHORS, SUBSTITUTIONS OF OTHER BRANDS AND/OR TYPES IS NOT ALLOWED, WITHOUT WRITTEN
- APPROVAL OF THE ENGINEER. 3. SUBSTITUTION REQUESTS FOR ALTERNATE PRODUCTS SHALL BE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER OF RECORD PRIOR TO USE. SUBSTITUTION REQUESTS SHALL INCLUDE AN
- ICC ESR OR IAPMO REPORT AND SUPPORTING CALCULATIONS INDICATING COMPLIANCE WITH DESIGN 4. ALL ADHESIVE/MECHANICAL ANCHORS SHALL BE INSTALLED, INCLUDING HOLE DRILLING AND PREPARATION, IN ACCORDANCE WITH AN APPROVED INDEPENDENT EVALUATION REPORT (ICC-ES,
- IAPMO, OR APPROVED EQUAL), AS INDICATED BELOW, AND IN ACCORDANCE WITH ALL MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII). 5. INSTALLERS SHALL BE, AT A MINIMUM, TRAINED FOR THE SPECIFIC APPLICATION INSTALLATION TECHNIQUE FOR THE SPECIFIC PRODUCT BY THE PRODUCT MANUFACTURERS FIELD EMPLOYEE OR SHALL POSSESS A TRAINING CARD OBTAINED BY THE MANUFACTURERS ONLINE TRAINING PROGRAM.
- 6. ADHESIVE ANCHORS SHALL BE INSTALLED IN CONCRETE HAVING A MINIMUM AGE OF 21 DAYS AT TIME OF ANCHOR INSTALLATION. ADHESIVE ANCHORS SHALL NOT BE FULLY LOADED UNTIL CONCRETE HAS 7. ADHESIVE ANCHORS SHALL CONSIST OF REINFORCING BAR OR THREADED RODS AS INDICATED IN THESE DOCUMENTS.
- 8. UNLESS APPROVED BY THE ENGINEER OF RECORD, CONCRETE AND DRILLED ANCHOR HOLES SHALL BE DRY AND FREE OF WATER FOR 14 DAYS PRIOR TO ADHESIVE INSTALLATION. CONTACT THE ENGINEER OF RECORD FOR GUIDANCE IF THE CONTRACTOR CHOOSES TO INSTALL IN DAMP, WATER-SATURATED, OR WATER-FILLED HOLES.
- 9. CONCRETE TEMPERATURE AT THE TIME OF INSTALLATION SHALL BE MONITORED BY THE CONTRACTOR. CONTRACTOR SHALL COMPLY WITH ALL MANUFACTURER'S PRINTED INSTALLATION
- INSTRUCTIONS (MPII) RELATIVE TO SUBSTRATE TEMPERATURE. 10. INSTALLATION OF ADHESIVE ANCHORS HORIZONTALLY OR UPWARDLY INCLINED TO SUPPORT SUSTAINED TENSION LOADS SHALL BE PERFORMED BY PERSONNEL CERTIFIED BY AN APPLICABLE CERTIFICATION PROGRAM. CERTIFICATION SHALL INCLUDE WRITTEN AND PERFORMANCE TESTS IN ACCORDANCE WITH THE ACI/CRSI ADHESIVE ANCHOR INSTALLER CERTIFICATION PROGRAM. OR EQUIVALENT IN ACCORDANCE WITH ACI 318-19 26.7.2 (e) PROOF OF CURRENT CERTIFICATION SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO INSTALLATION. CONTINUOUS SPECIAL INSPECTION SHALL BE PROVIDED FOR THESE ANCHORS.
- 11. UNLESS NOTED OTHERWISE, ALL ADHESIVE ANCHORS INTO CONCRETE SHALL BE: a. HILTI HIT-RE 500V3 (ESR-3814), OR HILTI HIT-HY 200-V3 (ESR-4868).
- SIMPSON SET-3G (ESR-4057), OR AT-XP (ER-263). c. DEWALT PURE 110+ (ESR-3298), OR AC200+ GOLD (ESR-4027-COLD WEATHER). 12. UNLESS NOTED OTHERWISE, ALL ADHESIVE ANCHORS INTO GROUTED MASONRY (CMU) SHALL BE: a. HILTI HIT-HY 270 (ESR-4143). b. SIMPSON SET-XP (ER-265), OR AT-XP (ER-281).
- c. DEWALT AC100+ GOLD (ESR-3200). 13. UNLESS NOTED OTHER WISE, ALL MECHANICAL ANCHORS INTO CONCRETE SHALL BE: a. HILTI KWIK BOLT-TZ2 (ESR-4266).
- b. SIMPSON STRONG-BOLT 2 (ESR-3037). 14. UNLESS NOTED OTHERWISE, ALL MECHANICAL ANCHORS INTO GROUTED MASONRY (CMU) SHALL BE: a. HILTI KWIK BOLT-TZ2 (ESR-4561).
- b. SIMPSON STRONG BOLT 2 (ER-240). c. DEWALT SCREWBOLT+ (ESR-4042). 15. UNLESS NOTED OTHERWISE, ALL SCREW ANCHORS INTO CONCRETE SHALL BE:
- a. SIMPSON TITEN HD (ESR-2713). b. DEWALT SCREWBOLT+ (ESR-3889). c. HILTI KH-EZ (ESR-3027) 16. UNLESS NOTED OTHERWISE, ALL SCREW ANCHORS INTO GROUTED MASONRY (CMU) SHALL BE: a. SIMPSON TITEN HD (ESR-1056).
- b. DEWALT SCREWBOLT+ (ESR-1678). c. HILTI KH EZ (ESR-3056). 17. ALL MASONRY CELLS WITHIN 8" OF THE ANCHOR SHALL BE SOLID GROUTED.

MEMBERS, OR OTHER STEEL ASSEMBLIES ATTACHED WITH MECHANICAL ANCHORS.

- 18. THE TESTING LABORATORY WILL PERFORM VISUAL INSPECTION OF ANCHORS AND DOWELS AS SPECIFIED IN THE SPECIAL INSPECTION SCHEDULE AND THE APPROVED INDEPENDENT EVALUATION REPORT. TENSION TESTING CAN BE REQUIRED AT THE DIRECTION OF THE STRUCTURAL ENGINEER OF RECORD OR THE SPECIAL INSPECTOR. 19. IF REINFORCEMENT IS ENCOUNTERED DURING DRILLING, ABANDON THAT HOLE AND SHIFT THE
- APPROVED ANCHORING ADHESIVE. AT CONTRACTORS OPTION, LOCATE EXISTING REINFORCEMENT PRIOR TO DRILLING/CORING. IF THE ANCHOR OR DOWEL CANNOT BE SHIFTED AS NOTED ABOVE, THE NGINEER WILL DETERMINE A NEW LOCATION. 20. LOCATE REINFORCEMENT AND CONFIRM FINAL ANCHOR LOCATIONS PRIOR TO FABRICATING PLATES

ANCHOR LOCATION TO AVOID THE REINFORCEMENT. PROVIDE A MINIMUM SPACE OF (2) ANCHOR HOLE DIAMETERS OR 2 INCHES, WHICH EVER IS LARGER, OF SOUND CONCRETE/MASONRY BETWEEN THE ANCHOR AND THE ABANDONED HOLE. FILL THE ABANDONED HOLE WITH NON-SHRINK GROUT OR AN

# H. SUSPENDED CONCRETE SLABS / SLABS ON METAL DECK

- 1. UNLESS NOTED OTHERWISE, ALL CONCRETE SLABS ON METAL DECK SHALL BE 5 1/2" TOTAL THICKNESS NORMAL WEIGHT CONCRETE WITH A WEIGHT LESS THAN 145 POUNDS PER CUBIC FOOT, REINFORCED WITH 6 X 6 - W1.4 X W1.4 WELDED WIRE FABRIC. REINFORCING STEEL SHALL BE CHAIRED TO 1" TOP COVER AT ALL BEAM LOCATIONS. EXCEPT WHERE SPECIFICALLY DETAILED, FIBER MESH MAY BE USED IN PLACE OF REINFORCEMENT IN SLABS ON DECK WHEN USED IN ACCORDANCE WITH AN APPROVED ICC RESEARCH REPORT AND WHERE APPROVED BY THE ENGINEER. WHERE THE SLAB CONSTRUCTION IS USED TO OBTAIN A UL FIRE RATING, THE PROPOSED FIBER MESH SHALL HAVE UL ACCEPTANCE AS AN APPROVED ALTERNATIVE TO WELDED WIRE FABRIC. AROUND OPENINGS IN SUSPENDED CONCRETE SLABS, ADD REINFORCING BARS EQUIVALENT TO BARS CUT BY OPENING WITH HALF ON EACH SIDE OF OPENING. BARS PARALLEL TO PRINCIPAL REINFORCING
- BEYOND OPENING. 3. SLAB PENETRATIONS LESS THAN 6" IN ALL DIRECTIONS WITH A CLEAR SPACING OF AT LEAST 3 TIMES THE LONGEST DIMENSION, DO NOT REQUIRE SUPPLEMENTAL REINFORCING. OTHERWISE, THE PENETRATIONS SHALL BE FRAMED ON 4 SIDES WITH STEEL ANGLES OR BENT PLATES (SEE TYPICAL DETAIL) UNLESS NOTED OTHERWISE.

SHALL RUN FULL LENGTH OF SPAN. BARS PARALLEL TO TEMPERATURE REINFORCING SHALL RUN 24"

- 4. EVERY EFFORT SHALL BE MADE TO PROVIDE A LEVEL FINISHED FLOOR WHILE MAINTAINING THE MINIMUM INDICATED SLAB THICKNESS. 5. CONTROL JOINTS IN SUSPENDED CONCRETE SLABS AND CONCRETE SLABS ON DECK SHALL NOT BE
- USED UNLESS SPECIFICALLY APPROVED AND DETAILED BY THE ENGINEER. 6. SEE TYPICAL DETAILS WHEN SLABS ARE MADE COMPOSITE WITH STEEL BEAMS. 7. NO CONDIUT IS ALLOWED IN CONCRETE SLABS ON METAL DECK. 8. WHERE CONDUIT IS CLUSTERED TOGETHER TO RISE ABOVE SLAB OR PENETRATE SLAB, PENETRATION

10. IN ALL SLABS NOT COVERED BY CARPET INSTALL #4 REINFORCING STEEL @ 12"O.C. EACH WAY.

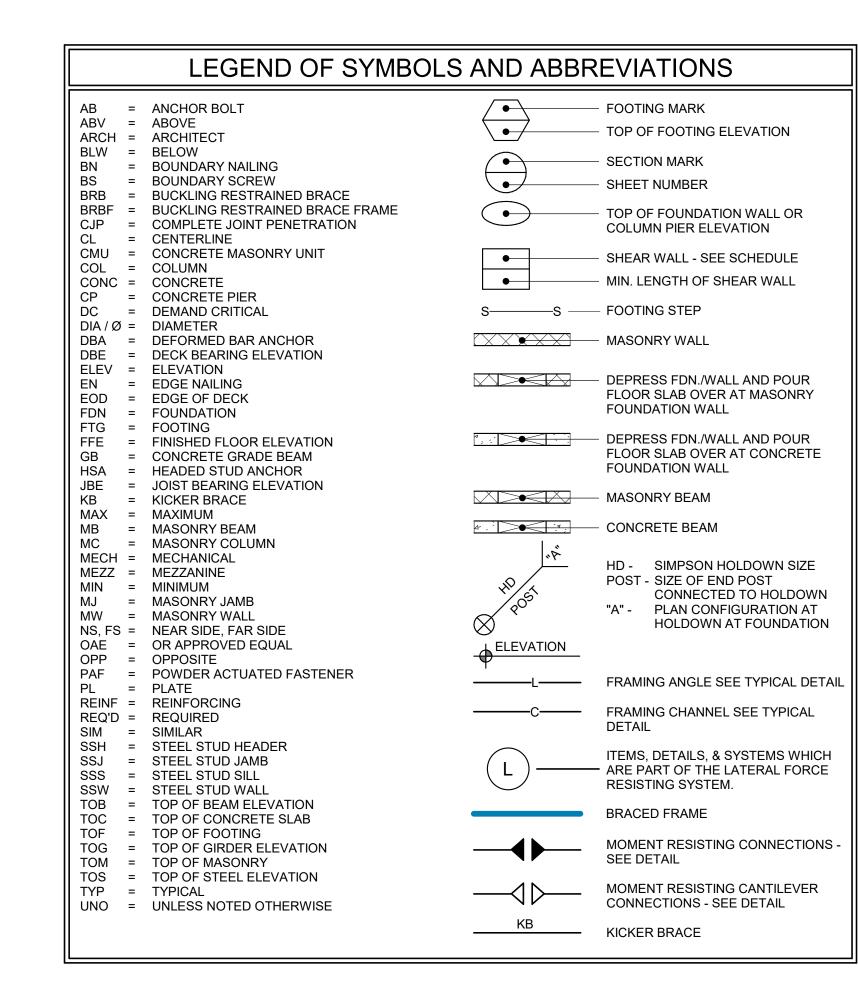
IN SLAB MUST BE SUPPORTED AS NOTED IN NOTE H.3 ABOVE. 9. CONTRACTOR SHALL PROVIDE ALL TEMPORARY SHORING, BRACING, AND GUYING AS REQUIRED DURING ERECTION AND PLACEMENT OF SUSPENDED CONCRETE SLABS ON METAL DECK.

## I. REINFORCING STEEL

- 1. REINFORCING BAR STRENGTH REQUIREMENTS:
- a. ALL REINFORCING BARS SHALL CONFORM TO ASTM STANDARD A-615 GRADE 60 AND ALL WELDED WIRE FABRIC SHALL CONFORM TO ASTM STANDARD A-1064 AND SHALL BE SUPPLIED IN FLAT SHEETS. ADEQUATELY TIE AND SUPPORT ALL REINFORCING STEEL AS SPECIFIED BY ACI 117, TO MAINTAIN EXACT REQUIRED POSITION. HEADED SHEAR STUD ASSEMBLIES SHALL CONFORM TO ASTM A1044.
- 3. STEEL DISCONTINUOUS FIBER REINFORCEMENT SHALL BE DEFORMED AND CONFORM TO ASTM A820 AND SHALL HAVE A LENGTH TO DIAMETER RATIO NOT SMALLER THAN 50 AND NOT GREATER THAN 100. 4. HEADED DEFORMED BARS SHALL CONFORM TO ASTM A970. OBSTRUCTIONS OR INTERRUPTIONS OF THE BAR DEFORMATIONS, IF ANY, SHALL NOT EXTEND MORE THAN 2 BAR DIAMETERS FROM THE
- BEARING FACE OF THE HEAD. 5. ALL REINFORCING STEEL SHALL BE TIED IN PLACE AND ADEQUATELY SUPPORTED PRIOR TO PLACING CONCRETE. WET STABBING OF ANY REINFORCING STEEL IS NOT PERMITTED, UNLESS SPECIFICALLY DETAILED OTHERWISE OR APPROVED BY THE ENGINEER. 6. ALL FIELD BENT DOWELS SHALL BE GRADE 40 WITH SPACING INDICATED REDUCED BY 1/3.
- 7. UNLESS NOTED OTHERWISE, REINFORCEMENT SHALL HAVE THE FOLLOWING CONCRETE COVERAGE a. CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH ..... 3" b. EXPOSED TO EARTH OR WEATHER:
- 1. #6 & LARGER ..... 2" 2. #5 & SMALLER .....1-1/2"
- c. NOT EXPOSED TO WEATHER OR EARTH SLABS, WALLS, JOISTS, #11 & SMALLER ..... 3/4" BEAMS, COLUMNS: MAIN REINFORCING OR TIES ..... 1-1/2"
- d. SLAB ON GRADE: 1. PLACE REINFORCING AT CENTER OF SLAB UNLESS INDICATED OTHERWISE.
- 8. EXCEPT WHERE NOTED ON PLANS OR DETAILS CONTINUOUS REINFORCEMENT SHALL BE SPLICED AT POINTS OF MINIMUM STRESS BY LAPPING PER THE REBAR LAP SCHEDULE. 9. REINFORCING STEEL MAY BE SPLICED WITH MECHANICAL COUPLERS THAT HAVE A TENSION CAPACITY OF AT LEAST 125% OF THE STRENGTH OF THE BAR. MECHANICAL COUPLERS SHALL BE A POSITIVE CONNECTING TYPE COUPLER, AND SHALL BE INSTALLED IN ACCORDANCE WITH AN APPROVED ICC
- RESEARCH REPORT. WHERE THESE ARE USED, SPLICES ON ADJACENT BARS SHALL BE STAGGERED AT LEAST 24 INCHES ALONG THE LENGTH OF THE BARS. 10. ALL VERTICAL REINFORCING IN STRUCTURAL ELEMENTS ABOVE SHALL BE SPLICED WITH MATCHING DOWELS EMBEDDED WITHIN THE FOOTINGS OR STRUCTURE BELOW. SPLICE LENGTHS SHALL COMPLY WITH REBAR LAP SCHEDULE. DOWELS INTO FOOTINGS SHALL TERMINATE WITH A STANDARD HOOK, AND SHALL EXTEND TO WITHIN 4" OF THE BOTTOM OF THE FOOTING, BUT NEED NOT EXTEND MORE
- THAN 20" INTO FOOTING. FOR MASONRY CONSTRUCTION SEE STRUCTURAL NOTE L.6.A. 11. DO NOT WELD REINFORCING EXCEPT AS NOTED ON PLANS, WHERE REINFORCING IS WELDED, USE ASTM A-706 REINFORCING. 12. REINFORCING BARS, TIES, AND TENDONS SHALL BE SUPPORTED BY NYLON CONES, PLASTIC-COATED
- TIE-WIRES, OR PLASTIC-COATED CHAIRS. REINFORCING IN FOOTINGS IS PERMITTED TO BE SUPPORTED 13. UNLESS NOTED OTHERWISE, HOOKS, STIRRUPS, TIES, AND OTHER BENDS IN REINFORCING STEEL SHALL MEET THE STANDARDS SET FORTH IN ACI 318/318R-19. UNLESS OTHERWISE PERMITTED BY THE ENGINEER, ALL REINFORCEMENT SHALL BE BENT COLD. REINFORCEMENT PARTIALLY EMBEDDED IN CONCRETE SHALL NOT BE FIELD BENT, EXCEPT AS SHOWN ON THESE DRAWINGS OR OTHERWISE
- 14. UNLESS SPECIFICALLY NOTED AND/OR DETAILED IN THE STRUCTURAL DRAWINGS CONDUIT SHALL NOT BE IN CONTACT WITH REINFORCING STEEL.

(STRUCTURAL NOTES CONTINUED ON SHEET S-002)

PERMITTED BY THE ENGINEER.



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### K. OPEN WEB JOISTS AND GIRDERS

- 1. ALL OPEN WEB STEEL JOISTS AND GIRDERS SHALL BE FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF "STANDARD SPECIFICATIONS AND CODE OF STANDARD PRACTICE" OF
- 2. (###/###) DENOTES APPLIED TOTAL AND LIVE UNIFORMLY DISTRIBUTED LOADS IN POUNDS PER LINEAR FOOT OF JOIST, RESPECTIVELY.
- 3. SEE JOIST LOAD PROFILES FOR SPECIALLY LOADED JOISTS. 4. CONCENTRATED POINT LOADS, NOT SPECIFICALLY SHOWN ON THE PLANS, OF LESS THAN 100 POUNDS FOR MECHANICAL UNITS. FIRE SPRINKLER MAINS, AND OTHER EQUIPMENT SHALL BE ALLOWED WITHOUT REQUIRING ADDITIONAL WEB MEMBERS TO BE INSTALLED. WHERE THE LOAD EXCEEDS 100 POUNDS, THE LOAD SHALL BE SUPPORTED WITHIN 6" OF A CHORD PANEL POINT. SUPPORT BEYOND 6" FROM PANEL POINTS CAN BE PROVIDED BY ADDING (2) L2 x 2 x 1/4 DIAGONALS TO THE NEAREST OPPOSITE CHORD PANEL POINT PER THE TYPICAL DETAIL. CONNECTIONS SHALL BE MADE CONCENTRIC TO THE CHORD ANGLES. BEAM CLAMPS, OR SIMILAR ECCENTRIC ATTACHMENTS, ARE NOT ALLOWED, EXCEPT AS INDICATED BELOW. BEAM CLAMPS, OR SIMILAR ATTACHMENTS THAT ARE NOT CENTERED ON THE CHORD ANGLES MAY ONLY BE USED FOR LOADS LESS THAN 10 POUNDS. SEE JOIST SUBMITTAL FOR ADDITIONAL REQUIREMENTS. ALL LOADS PROVIDED FOR IN THIS NOTE SHALL
- BE ACCOUNTED FOR IN THE SPECIFIED DESIGN LOADS. 5. ANY BRACING REQUIRED FOR MISCELLANEOUS ITEMS (I.E. DUCTWORK, PIPING, ETC.) MUST CONNECT TO THE TOP CHORD OF THE JOIST OR GIRDER. BRACING TO THE BOTTOM CHORD IS NOT ALLOWED
- UNLESS SPECIFICALLY DETAILED THAT WAY ON THE PLANS. 6. PROVIDE SPECIAL BEARING ENDS AS REQUIRED AT SLOPED BEARING CONDITIONS. CONTRACTOR
- SHALL COORDINATE WITH OTHER STRUCTURAL ELEMENTS. ALL JOISTS SHALL BE CAMBERED PER SJI SPECIFICATIONS, UNLESS NOTED OTHERWISE.
- 8. FIELD MODIFICATIONS (INCLUDING HOLES IN THE CHORD OR WEB MEMBERS) SHALL NOT BE MADE TO ANY JOIST OR GIRDER WITHOUT PRIOR APPROVAL BY THE MANUFACTURER. 9. FABRICATORS AND SUPPLIERS SHALL COORDINATE PAINT/FINISHES WITH REQUIREMENTS FOR DIRECT
- APPLIED INSULATION, FIREPROOFING, ETC. AS NOTED IN THE PROJECT SPECIFICATIONS. 10. JOIST BRIDGING SHALL BE PROVIDED AS REQUIRED BY THE JOIST MANUFACTURER AND SJI STANDARDS. BRIDGING WHERE SHOWN ON THE STRUCTURAL DRAWINGS IS A SCHEMATIC
- REPRESENTATION ONLY. SEE JOIST MANUFACTURER FOR BRIDGING SIZE, CONNECTIONS, TYPE AND 11. WHERE ADDED LOADS ARE SHOWN ON THE JOISTS BUT NOT SPECIFICALLY DIMENSIONED. THE JOIST
- STRESS IN THE MEMBERS. THE DESIGNER MAY ASSUME THAT THE LOAD OCCURS WITHIN 10 FEET OF A SCALED DIMENSION. 12. FABRICATOR MUST SUBMIT A CERTIFICATE OF COMPLIANCE TO THE BUILDING OFFICIAL PER IBC 2207.5
- STATING THAT WORK WAS PERFORMED IN ACCORDANCE WITH APPROVED CONSTRUCTION DOCUMENTS AND WITH SJI SPECIFICATIONS. 13. UNLESS NOTED OTHERWISE, ROOF JOISTS AND GIRDERS SHALL BE DESIGNED FOR A NET WIND UPLIFT

DESIGNER SHALL PLACE THOSE LOADS ON THE JOIST AT A LOCATION THAT RESULTS IN THE HIGHEST

- 14. ALL ROOF JOISTS BEARING ON EXTERIOR WALLS SHALL BE DESIGNED TO TRANSFER 3.75 KIPS (ULTIMATE) TOP CHORD AXIAL FORCE THROUGH THE BEARING SHOE. 15. ALL JOISTS AT GRID LINES SHALL BE DESIGNED TO RESIST A 10 KIP TOP CHORD AXIAL FORCE
- 16. ALL STANDARD AND NON STANDARD SJI JOISTS SHALL BE DESIGNED FOR THE FOLLOWING
- DEFLECTION CRITERIA: a. LIVE LOAD: L/240 TOTAL LOAD: L/180 17. JOIST MANUFACTURER SHALL APPLY ADDITIONAL POINT OR LINE LOAD AS REQUIRED TO SUPPORT FIRE PROTECTION MAINLINES 4" DIAMETER OR GREATER. JOIST MANUFACTURER SHALL COORDINATE WITH GENERAL CONTRACTOR TO OBTAIN LOCATIONS AND WEIGHTS OF THESE LINES. SEISMIC

BRACING LOADS FOR FIRE PROTECTION MAINLINES SHALL ALSO BE ACCOUNTED FOR IN THE JOIST

### L. MASONRY

MANUFACTURER'S DESIGN.

- 1. ALL HOLLOW MASONRY UNITS SHALL CONFORM TO ASTM C-90.
- f'm (MINIMUM, FACTORED) 2,000 PSI MINIMUM UNIT STRENGTH 2,000 PSI (TESTED IN ACCORDANCE WITH ASTM C-140) ACCEPTABLE RANGE OF UNIT WEIGHT: 105 PCF TO 125 PCF
- 2. ALL GROUT (SITE MIXED OR PRE-MIXED) SHALL CONFORM TO ASTM C-476 OR SECTION 2.2A OF TMS 602-16. GROUT SHALL BE PLACED WITH SUFFICIENT WATER FOR POURING WITHOUT SEGREGATION. DO
- NOT USE MORTAR FOR GROUT. MECHANICALLY VIBRATE ALL GROUT. . GROUT STOPS SHALL BE AN APPROVED PRODUCT DESIGNED AND MANUFACTURED FOR USE AS A GROUT STOP. GROUT STOP SUBMITTALS SHALL BE SUBMITTED TO THE ARCHITECT AND ENGINEER
- FOR REVIEW. OTHER GROUT STOP MATERIALS SUCH AS ASPHALT IMPREGNATED MATERIALS ARE NOT MORTAR SHALL BE TYPE S AND SHALL CONFORM TO ASTM C 270.
- ALL MASONRY WORK SHALL CONFORM TO CHAPTER 21 OF THE IBC. . UNLESS NOTED OTHERWISE, MINIMUM REINFORCING IN ALL 8" MASONRY WALLS SHALL BE AS FOLLOWS
- a. VERTICAL: #5 BARS IN GROUTED CELLS ADJACENT TO ALL OPENINGS GREATER THAN 24 INCHES WIDE, ON EACH SIDE OF CONTROL JOINTS, AT ENDS OF WALL, AND AT A MAXIMUM SPACING OF 32" THROUGHOUT THE WALL. AT CORNERS, PROVIDE A MINIMUM OF (4) VERTICAL BARS AT "T" JOINTS AND (3) VERTICAL BARS AT "L" JOINTS. SEE THE TYPICAL DETAIL / SCHEDULE FOR MORE INFORMATION. ALL VERTICAL REINFORCEMENT SHALL BE DOWELED INTO THE FOUNDATION WALL UNLESS SPECIFICALLY DETAILED OTHERWISE. b. HORIZONTAL: (2) #4 BARS IN 8" DEEP "H" BLOCK BOND BEAM UNITS AT 48"O.C. AND AT FLOORS.
- ROOF, BELOW OPENINGS, AND TOP OF WALL. BOND BEAMS AT ROOF SHALL SLOPE TO MATCH SLOPING ROOF. SEE THE MASONRY REINFORCING SCHEDULE FOR MASONRY BEAMS ABOVE
- 7. SEE THE MASONRY REINFORCING SCHEDULE FOR OPENINGS WHICH EXCEED 32 INCHES IN EITHER

CONTROL CRACKING OF FACE SHELLS.

- 8. ALL BLOCK CELLS CONTAINING REINFORCING, BOLTS, OR ANCHORS SHALL BE GROUTED SOLID. 9. PROVIDE (1) #5 (MINIMUM), IN GROUTED SPACE, ON ALL SIDES AND ADJACENT TO EVERY OPENING WHICH EXCEEDS 24" IN EITHER DIRECTION. HORIZONTAL BARS SHALL EXTEND 24" BEYOND THE CORNERS OF THE OPENING AND VERTICAL BARS SHALL EXTEND TO TOP OF WALL. VERTICAL REINFORCING SHALL BE PROVIDED AT ENDS, CORNERS AND EACH SIDE OF CONTROL JOINTS. SEE
- TYPICAL DETAILS FOR OPENINGS WHICH EXCEED 32" IN EITHER DIRECTION. 10. SOLID GROUTING OF MASONRY IS UNACCEPTABLE EXCEPT AS SPECIFICALLY NOTED ON PLANS AND
- SCHEDULES. 11. WHERE WALLS ARE NOT GROUTED SOLID, EACH GROUT POUR SHALL TERMINATE FLUSH WITH THE TOP OF THE UPPERMOST UNIT EXCEPT AT CELLS WITH VERTICAL REINFORCING WHERE GROUT SHALL BE 1-1/2" BELOW TOP OF UNIT TO PROVIDE CONSTRUCTION KEY. WHERE WALLS ARE GROUTED SOLID, EACH GROUT POUR SHALL TERMINATE 1-1/2" BELOW TOP OF UNIT.
- 12. GROUT POURS SHALL NOT EXCEED 5'-0" UNLESS HIGH LIFT GROUTING PROCEDURES ARE FOLLOWED. 13. THE USE OF HIGH LIFT GROUTING PROCEDURES REQUIRE THE APPROVAL OF THE ARCHITECT AND ENGINEER AND SHALL NOT EXCEED THE MAXIMUM HEIGHTS GIVEN IN TABLE 3.2.1 OF TMS 402-16. GROUT DEMONSTRATION PANELS, AS PRESCRIBED BY THE ARCHITECT AND ENGINEER, SHALL BE REQUIRED WHERE REQUESTED GROUTING PROCEDURES DO NOT MEET THE LIMITS OF TABLE 3.2.1. ADDITIONALLY, ALL HIGH LIFT GROUTING SHALL REQUIRE SPECIAL INSPECTION PROCEDURES NEEDED TO VERIFY GROUT PLACEMENT DURING CONSTRUCTION. DURING THE SUBMITTAL FOR APPROVAL PROCESS, SUBMITTAL SHALL INCLUDE, BUT NOT BE LIMITED TO: STATEMENT OF PROCEDURE FOR MECHANICAL VIBRATION OF HIGH LIFT GROUT; NEW MIX DESIGNS FOR HIGH SLUMP, HIGH LIFT GROUT; FOR SELF-CONSOLIDATING GROUT, SUBMIT MIX DESIGNS, SLUMP FLOW RATES, VISUAL STABILITY INDEX (VSI), AND QUANTITIES OF ADMIXTURES BEING USED.
- 14. ALL MASONRY BEAMS SHALL BE BUILT INTEGRAL WITH SUPPORT. NO TOOTHING OR DOWELING PERMITTED. UNITS WITH ONE END OPEN SHALL BE USED FOR ALL MASONRY BEAMS. 15. PROVIDE VERTICAL CONTROL JOINTS AT MAXIMUM SPACINGS NOTED BELOW UNLESS NOTED
- OTHERWISE IN THE SPECIFICATIONS AND/OR ON ARCHITECTURAL ELEVATIONS AND AT ALL CHANGES IN WALL ELEVATION AND MASONRY THICKNESS. CONTROL JOINTS SHALL NOT BE LOCATED DIRECTLY OVER OR CLOSER THAN 24" TO WALL OPENINGS (DOORS, WINDOWS, MECHANICAL OPENINGS, ETC.), OR WITHIN MASONRY JAMBS. REINFORCED MASONRY: 40 FT
- VENEER: 30 FT AND AT INTERFACE BETWEEN VENEER SUPPORTED BY FOUNDATIONS AND SUSPENDED STRUCTURAL ELEMENTS.
- 16. HORIZONTAL REINFORCEMENT SHALL TERMINATE AT EACH SIDE OF CONTROL JOINTS EXCEPT AT FLOOR AND ROOF LEVEL BOND BEAMS AND AT TOP OF PARAPET. 17. CONTROL JOINTS SHALL BE PROVIDED AT THE MASONRY SIDE OF EMBEDDED STEEL COLUMNS TO
- 18. SUPPORT NON-BEARING, NON-STRUCTURAL WALLS AT TOP OF MASONRY AS PER TYPICAL DETAILS AT LOCATIONS WHERE INTERSECTING OR PERPENDICULAR WALLS ARE 12'-0" OR MORE APART OR WHERE
- END OF WALL OCCURS 6'-0" OR MORE FROM INTERSECTING WALL
- 19. EMBED CHANNELS AND PLATES TO BE PLACED SO AS TO CREATE FLUSH SURFACE WITH FACE OF MASONRY. FLANGES ON CHANNEL EMBEDS SHALL BE HORIZONTAL.
- 20. ALL VERTICAL REINFORCING SHALL BE SECURED IN PLACE PRIOR TO GROUTING USING WIRE POSITIONERS OR OTHER ACCEPTABLE DEVICES. REINFORCING SHALL BE SECURED AT BAR-SPLICE LOCATIONS AND AT A SPACING NOT MORE THAN 120 BAR DIAMETERS.
- RUNNING-BOND WITH FULLY MORTARED BED JOINTS AROUND GROUTED CELLS.\ 22. MASONRY VENEER SHALL BE ANCHORED USING THE HOHMANN AND BARNARD VENEER ANCHOR

21. UNLESS NOTED OTHERWISE, MASONRY WALLS SHALL BE CONSTRUCTED UTILIZING COMMON

- ASSEMBLY SYSTEM, OR AN APPROVED EQUAL. REGARDLESS OF BACK-UP SYSTEM, PROVIDE A CONTINUOUS HORIZONTAL 9 GAUGE WIRE AT 16"O.C. IN VENEER MORTAR JOINTS FOR ANCHOR ATTACHMENT. POSITIVE ANCHORAGE TO THE WIRE USING THE SEISMICLIP INTERLOCK SYSTEM SHALL BE PROVIDED TO SUPPORT NOT MORE THAN 2 SQUARE FEET OF WALL, WITH A HORIZONTAL SPACING NOT EXCEEDING 18".
- a. METAL STUDS; USE HOHMANN AND BARNARD HB-213 S.I.S. (SEISMICLIP INTERLOCK SYSTEM) HEAVY DUTY ANCHORS OR AN APPROVED EQUAL. THE HB-213 ASSEMBLY SHALL BE ATTACHED TO WOOD STUDS USING A # 12 X 2" WOOD SCREWS OR TO METAL STUDS USING #10 SCREWS. BRICK AND BLOCK WALLS; USE HOHMANN AND BARNARD 270-ML-S.I.S. (SEISMICLIP INTERLOCK
- SYSTEM) MIGHTY-LOK SEISMIC ANCHORS OR AN APPROVED EQUAL, AT SPACINGS NOTED ABOVE. INSTALL A 2 WIRE 9 GAUGE LADDER TYPE JOINT REINFORCEMENT AT 16"O.C. IN THE BACK-UP WALL FOR ANCHORAGE ATTACHMENT c. CONCRETE WALLS; USE HOHMANN AND BARNARD HB 303SV SEISMIC NOTCH DOVE TAIL ANCHOR
- SYSTEM OR AN APPROVED EQUAL AT SPACINGS NOTED ABOVE 23. ELECTRICAL CONDUIT SHALL NOT BE PLACED IN CELLS THAT CONTAIN REBAR. CONDUIT IS ALLOWED TO PASS THROUGH REINFORCED CELLS WHEN IT OCCURS PERPENDICULAR TO THE REBAR. CONDUIT SHALL NOT CONTACT REBAR AS IT PASSES. THERE SHALL BE 1" CLEAR BETWEEN CONDUIT AND

#### M. STRUCTURAL DELEGATED DESIGNS AND DEFERRED SUBMITTALS

- 1. STRUCTURAL DELEGATED DESIGNS AND SUBSEQUENT DEFERRED SUBMITTALS ARE FOR ELEMENTS, PARTS, OR PORTIONS OF THE OVERALL STRUCTURAL SYSTEM THAT ARE INDICATED OR REFERRED TO ON THESE DRAWINGS AND THAT ARE CRITICAL TO THE PERFORMANCE OF THE OVERALL STRUCTURAL SYSTEM. DESIGN CRITERIA HAS BEEN PROVIDED FOR THESE ITEMS IN THE STRUCTURAL NOTES, 2. STRUCTURAL DEFERRED SUBMITTALS ARE COMPLETE PACKAGES TO BE SUBMITTED FOR REVIEW THAT INCLUDE DRAWINGS AND CALCULATIONS FOR ALL DELEGATED DESIGN ITEMS AND THEIR
- CONNECTIONS. DEFERRED SUBMITTALS SHALL BEAR THE STAMP AND SIGNATURE OF THE DESIGN PROFESSIONAL RESPONSIBLE FOR THEIR DESIGN. 3. ARW ENGINEERS WILL REVIEW STRUCTURAL DEFERRED SUBMITTALS TO VERIFY DESIGN CRITERIA IS
- 4. STRUCTURAL DELEGATED DESIGN COMPONENTS SHALL NOT BE INSTALLED UNTIL APPROVED BY THE BUILDING OFFICIAL. 5. STRUCTURAL DELEGATED DESIGN ITEMS REQUIRING DEFERRED SUBMITTALS INCLUDE, BUT ARE NOT a. OPEN WEB JOISTS & GIRDERS, BRIDGING, BRACING, CONNECTIONS, AND RELATED COMPONENTS.

#### N. NON-STRUCTURAL DELEGATED DESIGNS AND DEFERRED SUBMITTALS

COMPLIANT WITH THE APPROVED CONSTRUCTION DOCUMENTS.

 NON-STRUCTURAL DELEGATED DESIGNS AND SUBSEQUENT DEFERRED SUBMITTALS ARE FOR ITEMS NOT INCLUDED IN THE STRUCTURAL DELEGATED DESIGN SECTION. THESE ARE ITEMS THAT ARE NOT CRITICAL TO THE OVERALL PERFORMANCE OF THE STRUCTURAL SYSTEM BUT THAT IMPART LOADS AND FORCES TO THE STRUCTURAL SYSTEM.

4. IF THE STRUCTURAL DRAWINGS INCLUDE LOADS TO ACCOMMODATE NON-STRUCTURAL ELEMENTS.

2. NON-STRUCTURAL DEFERRED SUBMITTALS SHALL BEAR THE STAMP AND SIGNATURE OF THE DESIGN PROFESSIONAL RESPONSIBLE FOR THE DESIGN. 3. ARW ENGINEERS WILL REVIEW NON-STRUCTURAL DEFERRED SUBMITTALS TO VERIFY DESIGN

CRITERIA IS COMPLIANT WITH THE APPROVED CONSTRUCTION DOCUMENTS.

- THE CONTRACTOR SHALL SUBMIT DOCUMENTATION INDICATING THAT THE NON-STRUCTURAL ELEMENTS COMPLY WITH THE LOADING CRITERIA PROVIDED HEREIN. SUCH DOCUMENTATION SHALL BEAR THE STAMP AND SIGNATURE OF THE DESIGN PROFESSIONAL RESPONSIBLE FOR THE DESIGN. 5. WHEN THE NON-STRUCTURAL DEFERRED SUBMITTAL INDICATES THAT THE ELEMENT WILL IMPART FORCES IN EXCESS OF LOADS THAT ARE INDICATED ON THE STRUCTURAL DRAWINGS, THE CONTRACTOR SHALL SUBMIT A DETAILED GRAPHICAL REPRESENTATION OF THOSE DESIGN LOADS. INCLUDING MAGNITUDE, AND LOCATION. THE GRAPHIC SHALL BE ACCOMPANIED BY DOCUMENTATION
- INDICATING THAT THE NON-STRUCTURAL ELEMENT DESIGN COMPLIES WITH THE LOADING CRITERIA PROVIDED HEREIN. THE LETTER SHALL BEAR THE STAMP AND SIGNATURE OF THE DESIGN PROFESSIONAL RESPONSIBLE FOR THE DESIGN. 6. NON-STRUCTURAL DELEGATED DESIGN ITEMS REQUIRING DEFERRED SUBMITTALS SHALL INCLUDE,
- a. COLD FORMED STEEL STUDS / JOISTS / HEADERS / JAMBS / TRUSSES. b. SEISMIC BRACING OF ALL ARCHITECTURAL, MECHANICAL, PLUMBING, AND ELECTRICAL ITEMS WHERE REQUIRED BY THE MOST RECENT VERSION OF ASCE 7 AND THE PROJECT CONTRACT

DOCUMENTS. c. STRUCTURAL STEEL STAIRS.

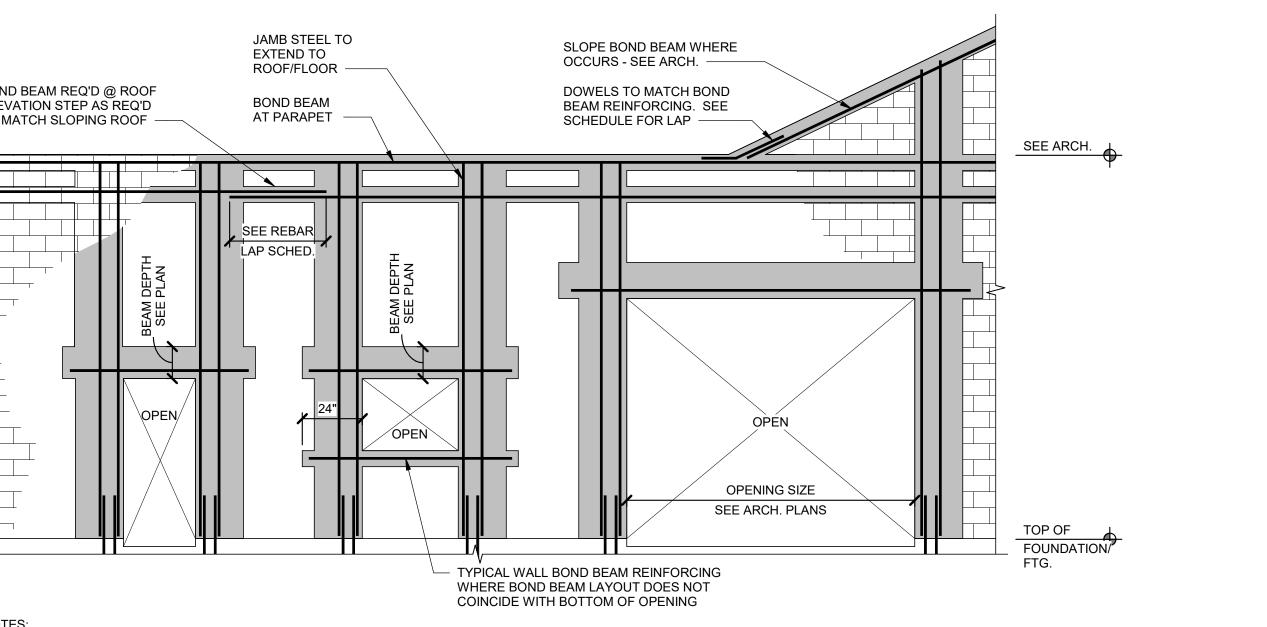
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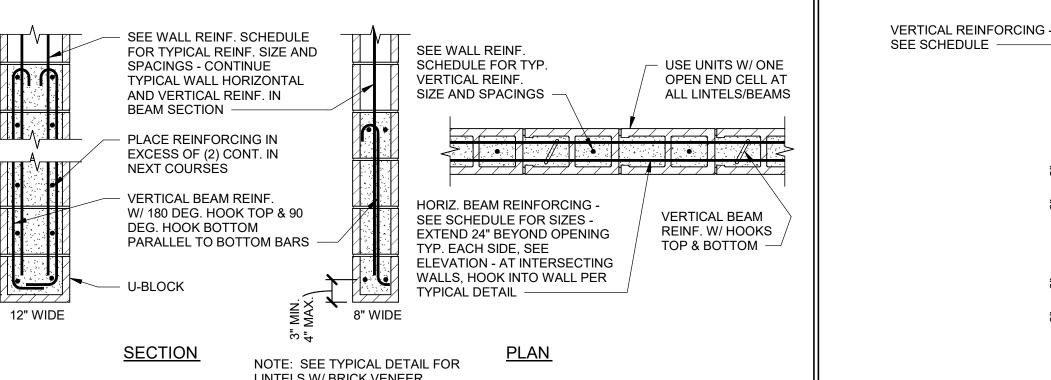


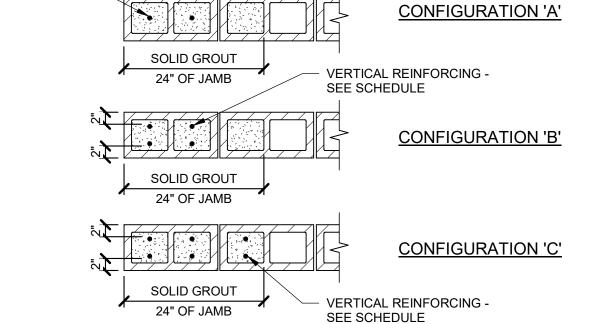


1. USE OPEN-END UNITS AT INTERSECTIONS OF BEAMS AND JAMBS. 2. TYPICAL HORIZONTAL BOND BEAMS MAY BE ADJUSTED UP OR DOWN BY ONE COURSE PROVIDED THE OVERALL NUMBER OF REQUIRED BOND BEAMS ARE INSTALLED. 3. TYPICAL HORIZONTAL AND VERTICAL WALL REINFORCING NOT SHOWN FOR CLARITY, SEE PLAN AND SCHEDULE FOR TYPICAL WALL REINFORCING.

			MA	ASONRY	BEAM S	CHEDUI	LE				MASON	IRY JAI	MB SCH	EDULE
	NOMINAL THICKNESS	BOTTOM REINF.	TOP REINF.	VERTICAL REINF.	MIN. GROUT DEPTH	OPENING ① SIZE	COMMENTS		NOMINAL THICKNESS	VERTICAL REINF.	TIES	CONFIG.	OPENING ① SIZE	COMMENTS
MB-1	8"	(2) #5	(2) #4	#4 @ 16"o.c.	16"	UP TO 5'-0"		MJ-1	8"	(2) #5		А	2'-8" TO 5'-0"	
MB-2	8"	(2) #5	(2) #4	#4 @ 16"o.c.	32"	5'-1" TO 9'-0"		MJ-2	8"	(4) #5		В	5'-1" TO 7'-0"	
MB-3	12"	(2) #5	(2) #4	#4 @ 16"o.c.	24"	UP TO 5'-0"		MJ-3	8"	(6) #5		С	SEE PLAN	
MB-4	12"	(2) #5	(2) #4	#4 @ 16"o.c.	32"	5'-1" TO 7'-0"		MJ-4	8"	(8) #5			SEE PLAN	SIMILAR TO CONFIG. C - SOLID GROUT 32"
								MJ-5	12"	(4) #5		В	2'-8" TO 5'-0"	
								M L-6	12"	(6) #5			5'_1" TO 7'_0"	

1. WHERE SPECIFIC JAMBS ARE NOT NOTED ON THE PLANS - REFER TO OPENING SIZE FOR REQUIRED REINFORCING AND CONFIGURATION. 2. ALL VERTICAL REINFORCING SHALL HAVE MATCHING DOWELS CAST INTO FOUNDATIONS. HORIZONTAL REINFORCING NOT SHOWN. 4. JAMBS TO BE GROUTED SOLID.

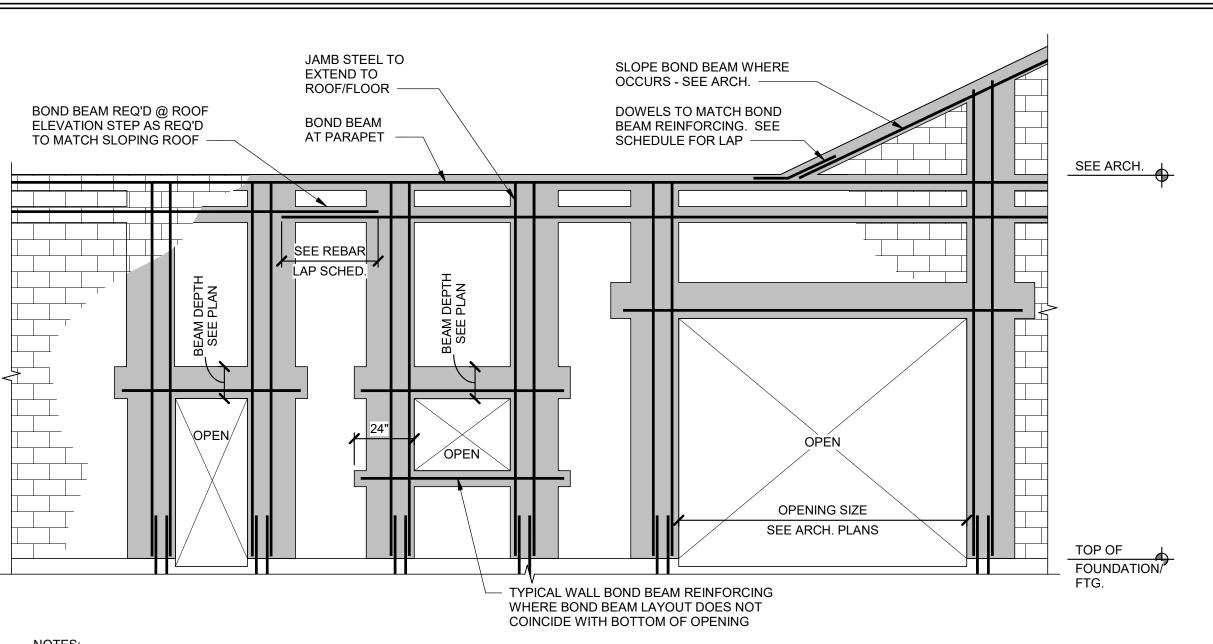




		VERT.	REINF.	HOI	RIZ. BOI	ND BEAM	REINF.		
MARK	THICK.	SIZE	SPACE	NO.	SIZE	SPACE	@ ROOF	@ ELEVATED FLOOR	COMMENTS
MW-8A	8"	#5	32"o.c.	(2)	#4	48"	(2) #4		
MW-8B	8"	#5	32"o.c.	(2)	#5	32"	(2) #5	(2) #5	
MW-12A	12"	(2) #5	16"o.c.	(2)	#5	48"			SOLID GROUTED
MW-12B	12"	(2) #5	24"o.c.	(2)	#5	48"	(4) #5	(4) #5	SOLID GROUTED

- 1	
	NOTES:
	1. FOR ANY CMU WALLS NOT SPECIFICALLY CALLED OUT IN PLANS, USE MW-8A.
	2. VERTICAL REINFORCING TO BE @ CENTERLINE OF WALL WHERE SINGLE BAR IS NOTED IN SCHEDULE. POSITION
	BARS 2" FROM EACH FACE OF WALL WHERE (2) BARS ARE SPECIFIED
	3. SOLID GROUTING OF WALLS IS UNACCEPTABLE EXCEPT WHERE SPECIFICALLY NOTED.
	4. SEE STRUCTURAL NOTES FOR ADDITIONAL INFORMATION.
	5. A BOND BEAM SHALL BE LOCATED IN THE FIRST COURSE ABOVE THE FOUNDATION IF VERTICAL DOWELS HAVE
	REEN BENT TO ALIGN WITH VERTICAL CELLS, WHETHER OR NOT MASONRY WERS HAVE BEEN CLIT

BEEN BENT TO ALIGN WITH VERTICAL CELLS, WHETHER OR NOT MASONRY WEBS HAVE BEEN CUT.

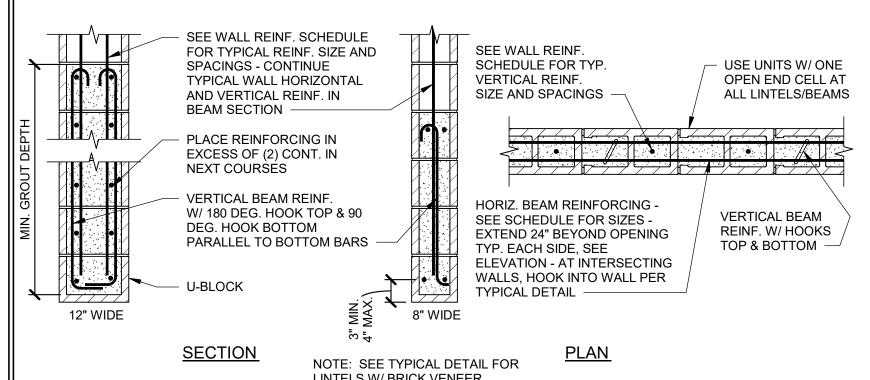


TYPICAL MASONRY / JAMB REINFORCING SCHEDULE

4. JAMB REINFORCING SHOWN IS SCHEMATIC. SEE SCHEDULE & DETAILS FOR ACTUAL JAMB REINFORCING. 5. ALL VERTICAL WALL REINFORCING SHALL BE CONTINUOUS BETWEEN THE LEVELS IN WHICH THE WALL OCCURS.

			M	ASONRY	BEAM S	CHEDU	LE				MASON	NRY JA	MB SCH	EDULE
MARK	NOMINAL THICKNESS	BOTTOM REINF.	TOP REINF.	VERTICAL REINF.	MIN. GROUT DEPTH	OPENING ① SIZE	COMMENTS	MARK	NOMINAL THICKNESS	VERTICAL REINF.	TIES	CONFIG.	OPENING ① SIZE	COMMENTS
MB-1	8"	(2) #5	(2) #4	#4 @ 16"o.c.	16"	UP TO 5'-0"		MJ-1	8"	(2) #5		А	2'-8" TO 5'-0"	
MB-2	8"	(2) #5	(2) #4	#4 @ 16"o.c.	32"	5'-1" TO 9'-0"		MJ-2	8"	(4) #5		В	5'-1" TO 7'-0"	
MB-3	12"	(2) #5	(2) #4	#4 @ 16"o.c.	24"	UP TO 5'-0"		MJ-3	8"	(6) #5		С	SEE PLAN	
MB-4	12"	(2) #5	(2) #4	#4 @ 16"o.c.	32"	5'-1" TO 7'-0"		MJ-4	8"	(8) #5			SEE PLAN	SIMILAR TO CONFIG. C - SOLID GROUT 32"
								MJ-5	12"	(4) #5		В	2'-8" TO 5'-0"	
								MJ-6	12"	(6) #5		С	5'-1" TO 7'-0"	

1. WHERE SPECIFIC BEAMS ARE NOT NOTED ON THE PLANS - REFER TO OPENING SIZE FOR REQUIRED BEAM 2. FIRST VERTICAL BAR TO BE WITHIN 8" OF END OF BEAM. . SEE TYPICAL ELEVATION - VIEW OF BEAM. 4. VERTICAL REINFORCING SHALL HAVE HOOKS TOP AND BOTTOM.



MIN. GROUT DEPTH	SEE WALL REINF. SCHEDULE FOR TYPICAL REINF. SIZE AND SPACINGS - CONTINUE TYPICAL WALL HORIZONTAL AND VERTICAL REINF. IN BEAM SECTION  PLACE REINFORCING IN EXCESS OF (2) CONT. IN NEXT COURSES  VERTICAL BEAM REINF. W/ 180 DEG. HOOK TOP & 90 DEG. HOOK BOTTOM PARALLEL TO BOTTOM BARS  U-BLOCK  SEE WALL REINF. SCHEDULE FOR TYP. VERTICAL REINF. SIZE AND SPACINGS  HORIZ. BEAM REINFORCING - SEE SCHEDULE FOR SIZES - EXTEND 24" BEYOND OPENING TYP. EACH SIDE, SEE ELEVATION - AT INTERSECTING WALLS, HOOK INTO WALL PER TYPICAL DETAIL  SEE WALL REINF. VERTICAL BEAM REINF. W/ HOOKS TOP & BOTTOM  REINF. W/ HOOKS TOP & BOTTOM  REINF. W/ HOOKS TOP & BOTTOM  SEE WALL REINF. SCHEDULE FOR TYP. VERTICAL BEAM REINF. W/ HOOKS TOP & BOTTOM  REINF. W/ HOOKS TOP & BOTTOM  SEE WALL REINF. SCHEDULE FOR TYP. VERTICAL BEAM REINF. W/ HOOKS TOP & BOTTOM  SEE WALL REINF. SCHEDULE FOR TYP. VERTICAL BEAM REINF. W/ HOOKS TOP & BOTTOM  SEE WALL REINF. SCHEDULE FOR TYP. VERTICAL BEAM REINF. W/ HOOKS TOP & BOTTOM  SEE WALL REINF. SCHEDULE FOR TYP. VERTICAL BEAM REINF. W/ HOOKS TOP & BOTTOM  SEE WALL REINF. SCHEDULE FOR TYP. VERTICAL BEAM REINF. W/ HOOKS TOP & BOTTOM  SEE WALL REINF. SCHEDULE FOR TYP. VERTICAL BEAM REINF. W/ HOOKS TOP & BOTTOM  SEE WALL REINF. SCHEDULE FOR TYP. VERTICAL BEAM REINF. W/ HOOKS TOP & BOTTOM  SEE WALL REINF. SCHEDULE FOR TYP. VERTICAL BEAM REINF. W/ HOOKS TOP & BOTTOM  SEE WALL REINF. SCHEDULE FOR TYP. VERTICAL BEAM REINF. W/ HOOKS TOP & BOTTOM  SEE WALL REINF. SCHEDULE FOR TYP. VERTICAL BEAM REINF. W/ HOOKS TOP & BOTTOM  SEE WALL REINF. SCHEDULE FOR TYP. VERTICAL BEAM REINF. VERTICAL BEAM REINF. W/ HOOKS TOP & BOTTOM  SEE WALL REINF. SCHEDULE FOR TYP. VERTICAL BEAM REINF. W/ HOOKS TOP & BOTTOM  SEE WALL REINF. SCHEDULE FOR TYP. VERTICAL BEAM REINF. W/ HOOKS TOP & BOTTOM  SEE WALL REINF. SCHEDULE FOR TYP. VERTICAL BEAM REINF. W/ HOOKS TOP & BOTTOM  SEE WALL REINF. SCHEDULE FOR TYP. VERTICAL BEAM REINF. W/ HOOKS TOP & BOTTOM  SEE WALL REINF. SCHEDULE FOR TYP. VERTICAL BEAM REINF. W/ HOOKS TOP & BOTTOM  SEE WALL REINF. SCHEDUL
	SECTION  NOTE: SEE TYPICAL DETAIL FOR LINTELS W/ BRICK VENEER  PLAN  PLAN

MARK         WIDTH         LENGTH           FC2         2'-0"         CONT.           FC2.5         2'-6"         CONT.           FC3         3'-0"         CONT.           FC4         4'-0"         CONT.           FC5         5'-0"         CONT.           FC6         6'-0"         CONT.	12" 12" 12" 12" 14"	(2) (3) (3)	#5 #5 #5	NO.	SSWISE R SIZE 	SPA.	REMARKS
FC2.5 2'-6" CONT. FC3 3'-0" CONT. FC4 4'-0" CONT. FC5 5'-0" CONT. FC6 6'-0" CONT.	12" 12" 14"	(3)	#5				
FC3 3'-0" CONT. FC4 4'-0" CONT. FC5 5'-0" CONT. FC6 6'-0" CONT.	12" 14"	` ′					
FC4 4'-0" CONT. FC5 5'-0" CONT. FC6 6'-0" CONT.	14"	(3)	#5				
FC5 5'-0" CONT. FC6 6'-0" CONT.			#J				
FC6 6'-0" CONT.		(4)	#5	(1)	#5	12"o.c.	
	16"	(5)	#6	(1)	#6	12"o.c.	
	20"	(6)	#6	(1)	#6	12"o.c.	REINFORCE TOP & BOTTO
FC7 7'-0" CONT.	22"	(7)	#7	(1)	#7	12"o.c.	REINFORCE TOP & BOTTO
FC8 8'-0" CONT.	24"	(8)	#8	(1)	#8	12"o.c.	REINFORCE TOP & BOTTO
F3 3'-0" 3'-0"	12"	(3)	#5	(3)	#5		
F3.5 3'-6" 3'-6"	12"	(3)	#5	(3)	#5		
F4 4'-0" 4'-0"	12"	(4)	#5	(4)	#5		
F4.5 4'-6" 4'-6"	12"	(5)	#5	(5)	#5		
F5 5'-0" 5'-0"	14"	(5)	#5	(5)	#5		
F5.5 5'-6" 5'-6"	14"	(6)	#6	(6)	#6		
F6 6'-0" 6'-0"	16"	(6)	#6	(6)	#6		
F6.5 6'-6" 6'-6"	18"	(7)	#6	(7)	#6		
F7 7'-0" 7'-0"	18"	(7)	#6	(7)	#6		
F7.5 7'-6" 7'-6"	20"	(7)	#7	(7)	#7		
F8 8'-0" 8'-0"	22"	(8)	#7	(8)	#7		
F8.5 8'-6" 8'-6"	22"	(8)	#7	(8)	#7		
F9 9'-0" 9'-0"	24"	(9)	#7	(9)	#7		
F9.5 9'-6" 9'-6"	26"	(10)	#7	(10)	#7		
F10 10'-0" 10'-0"	26"	(10)	#8	(10)	#8		
FS36 3'-0" 6'-0"	12"	(3)	#5	(6)	#5		
FS6.5 6'-6" 6'-6"	24"	(7)	#7	(7)	#7		REINFORCE TOP AND BOTT

TYP. FOOTING SECTION

TYP. FOOTING SECTION

W/ TOP & BOTTOM REINF

	BEAM / WALL HORIZONTAL	19"	2	6"	32"	38"	45"	
	WALL VERTICAL COLUMN AND JAMB	12"	12"	20"	18" 31"	34" 54"	47"	63"
	COUPLER CAPA AS INDICATED A 2. DEVELOPMENT 3. WHEN SPLICING 4. ALL REBAR #8 A	LENGTHS SHALL B BARS OF DIFFERE	CHANICAL CO E INCREASEI ENT SIZES, US SONRY SHALI	UPLERS ARE D BY 50% WHI SE LAP SPLIC	USED, STAGGER A ERE EPOXY COATE ELENGTH OF LARC	ADJACENT SPLICE ED REBAR IS USEI GER BARS UNO.	S A MINIMUM OF D.	F 24"
	BEAN	M CONNEC	CTION	SCHED	DULE @ M	ASONRY	WALL	
NOTE: SEE ARCHITECTURAL DRAWINGS FOR ACOUSTICAL DECK LOCATIONS.	EQUAL SPACING	MASONF  2" TYP.  EQUAL  SPACING  O  O  O  P. H.S.A.  ERN LAYOUT	EMBED CHANNEL	2 1/4"	3" TYP.  1 1/2" T  (2 H	25 BOLTS AS PER WASHERS THAT E SLOTS PER THI D NOT USE TC BO BEAM - SEE PLAN	COMPLETELY CO E STRUCTURAL N LTS) SLOTTED DULE N/ 3/4"DIA x 5"	OM, DP OVER
EDULE	BEAM SIZE	ANGLE SIZE (EA. SIDE)	WELD 'A'	BOLTS	EMBED CHANNE	# OF L H.S.A.	H.S.A. PATTER	lN .
	W/0 C0	2 v E v 1/4"	2/46"	(2) 2/4"(X	C0 v 12 75 v	01 011 4		

MAX. FLEXIBILITY

14.3

13.2

12.1

11

10.1

BAR LOCATION

BEAM / WALL

HORIZONTAL WALL VERTICAL

COLUMN AND JAMB

BAR LOCATION

BEAM / WALL

HORIZONTAL

WALL VERTICAL

COLUMN AND JAMB

BAR LOCATION

CASE#

1 2

12" 15"

#3

CASE#

12" 13"

CASE#

CAPACITY (ASD) FACTOR

764 PLF

908 PLF

1168 PLF

1499 PLF

	(EA. SIDE)	WELD 'A'	BOLTS	EMBED CHANNEL	# OF H.S.A.	H.S.A. PATTER
W8, C8	3 x 5 x 1/4"	3/16"	(2) 3/4"Ø	C8 x 13.75 x 0'-8"	4	0 0
W10, C10	3 x 5 x 5/16"	1/4"	(2) 3/4"Ø	C12 x 20.7 x 1'-4"	6	000
W12, C12	3 x 5 x 5/16"	1/4"	(3) 3/4"Ø	C12 x 20.7 x 1'-4"	6	000
W14	3 x 5 x 5/16"	1/4"	(3) 3/4"Ø	C15 x 33.9 x 1'-4"	9	000
W16	3 x 5 x 5/16"	1/4"	(4) 3/4"Ø	C15 x 33.9 x 2'-0"	12	0 0 0 0 0 0 0 0 0 0
W18	3 x 5 x 5/16"	1/4"	(5) 3/4"Ø	(2) C12 x 20.7 x 2'-0"	12	• • • • • • • • • • • • • • • • • • •

2021 IBC MASONRY REBAR LAP SPLICE SCHEDULE

FOR MASONRY APPLICATIONS (TMS 402/602 - 16)

CASE #1 = SINGLE BAR, CENTERED IN CEL

CASE #2 = WHEN REINFORCING BAR IS PLACED ADJACENT TO FACE SHELL

MASONRY REINFORCING & SPLICE LENGTHS (IN) (f'm = 1500psi)

CASE#

1 2

15" 26"

CASE#

1 2

13" 22"

CASE#

**BAR SIZE** 

CASE#

1 2

23" 40"

MASONRY REINFORCING & SPLICE LENGTHS (IN) (f'm = 2000psi) **BAR SIZE** 

CASE#

1 2

32"

20" 35"

CASE#

1 2

MASONRY REINFORCING & SPLICE LENGTHS (IN) (f'm = 2500psi)

**BAR SIZE** 

CASE#

1 2

43" 54"

CASE#

1 2

38" 54"

CASE#

CASE#

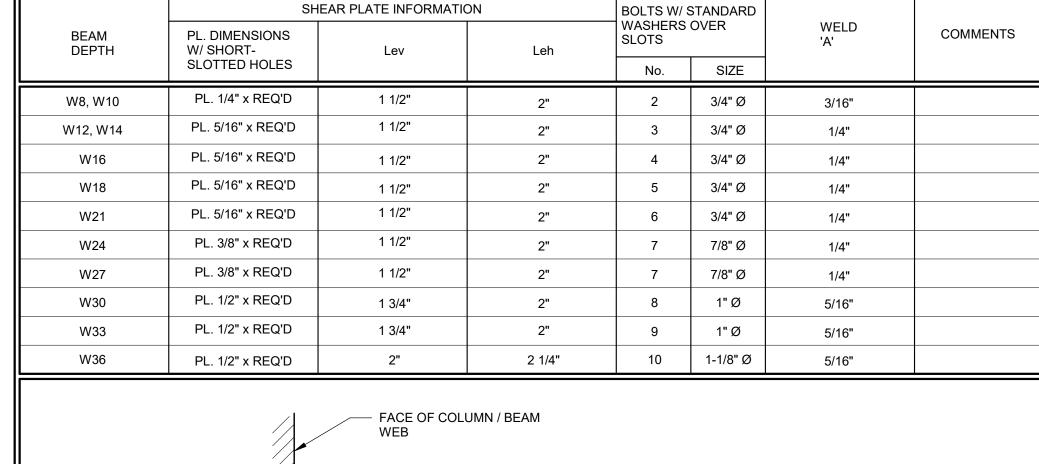
60" 63"

CASE#

1 2

52" 63"

CASE#



BASE BID KEY PLAN

BEAM CONNECTION SCHEDULE

ROOF DECK SCHEDULE

24"o.c. 3/4"

18"o.c.

--- 8"o.c. 3/4"

**FASTENING PATTERNS** 

WELD PATTERN: 32/5

•

WELD PATTERN: 36/4

WELD PATTERN: 36/7/4

B. IF N DECK IS NOT NESTABLE, N DECK END BUTT JOINTS OVER STEEL JOISTS SHALL USE 16 GA. x 6" CONTINUOUS SHEET BETWEEN DECK AND JOIST TOP CHORD ANGLES. DECK WELDS TO PENETRATE SHEET AND ENGAGE JOIST CHORD.

. ALTERNATE SYSTEMS SHALL MEET OR EXCEED THE MINIMUM SHEAR CAPACITY AND SHALL PROVIDE LESS THAN OR EQUAL TO THE

USE NESTABLE (OVERLAPPING) SIDE SEAMS AT SCREW ATTACHMENTS AND INTERLOCKING SIDE SEAMS AT WELDS.

. TOP SEAM WELDS SHALL BE 1-1/2" LONG AND SHALL BE ACCORDING TO SDI STANDARDS.

I. ALL DECK WITH A PROFILE DEPTH OF 2" OR LESS SHALL HAVE NESTED OR TELESCOPED END LAPS.

ALL ALTERNATE SYSTEMS SHALL BE SUBMITTED TO THE ENGINEER FOR REVIEW AND APPROVAL

--- 12"o.c.

3/4"

3/4"

12"o.c.

36/4 @ INTERIOR PANEL SUPPORTS
36/7 @ END PANEL SUPPORTS

SUPPORTS SIDE SEAMS

SUPPORTS PARALLEL TO SUPPORTS PARALLEL TO FLUTES

DEPTH TYPE GA. DIA. WELD PATTERN SCREWS WELD LOCK II (7) Ø WELD SPA.

MIN. SHEAR CAPACITY (AS

ATTACHMENT

B | 1 1/2" | PLB | 20 | 3/4" | 36/7/4 |

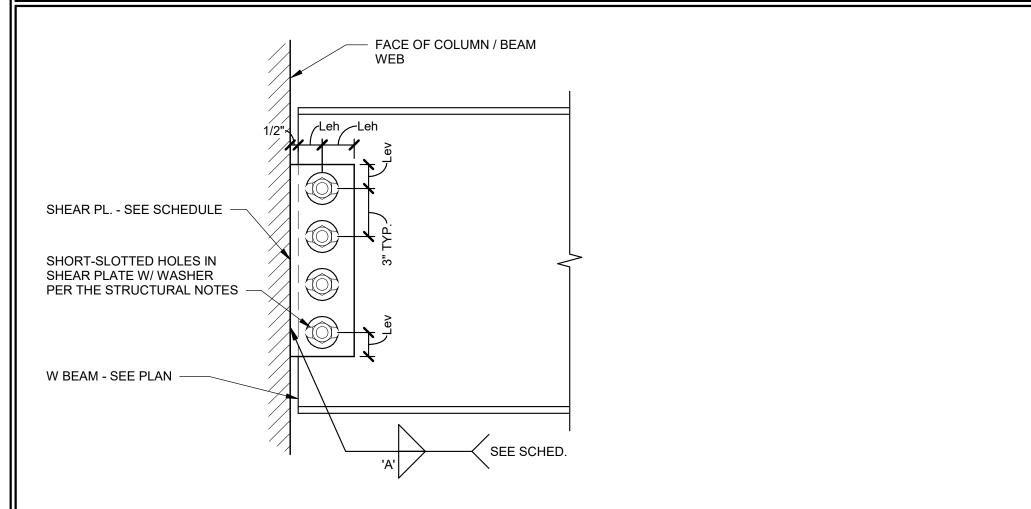
C | 1 1/2" | PLB | 20 | 3/4" | 36/7/4 |

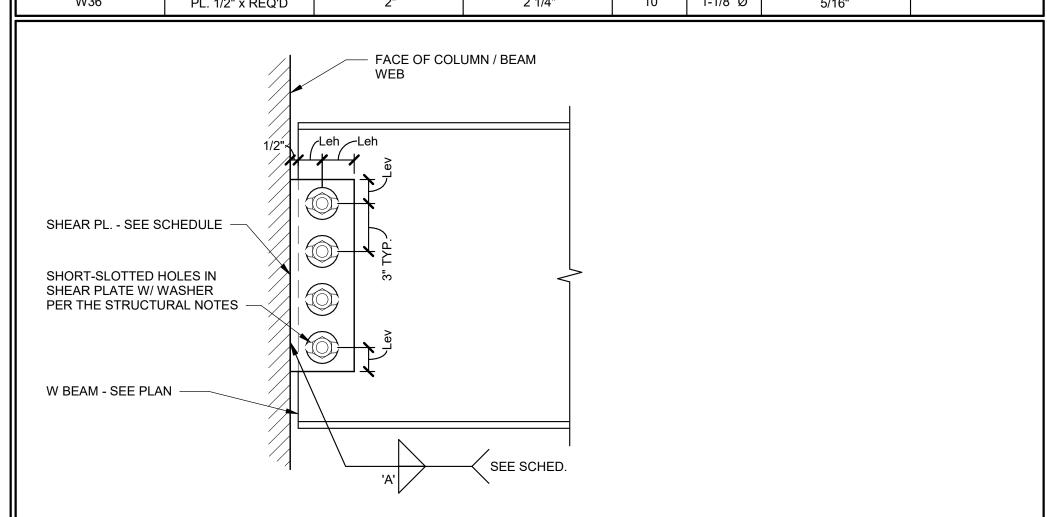
D | 1 1/2" | PLB | 20 | 3/4" | 36/7/4

E | 1 1/2" | PLB | 20 | 3/4" | 36/7/4 |

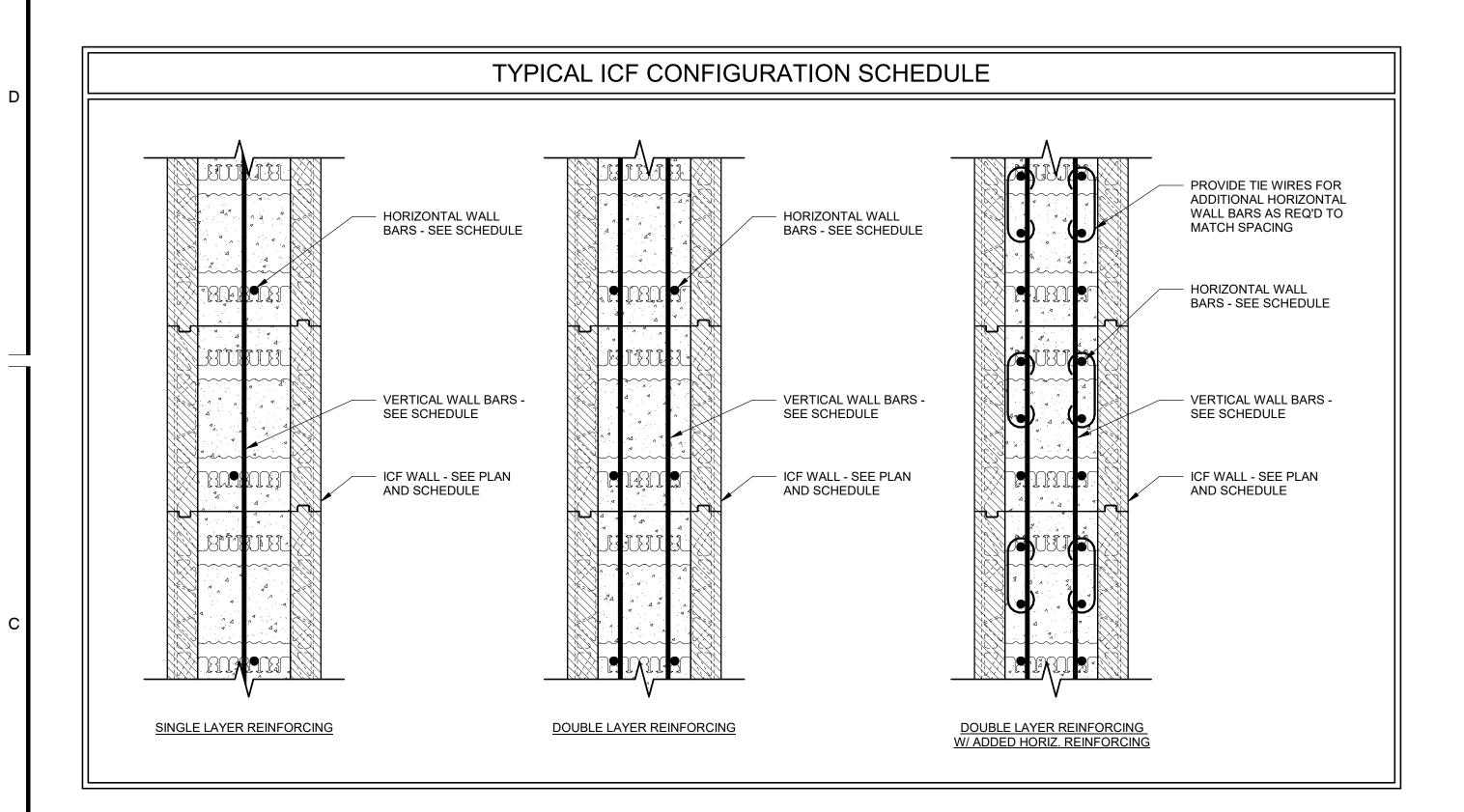
. SUBMIT CURRENT ICC APPROVAL FOR ALL DECKS.

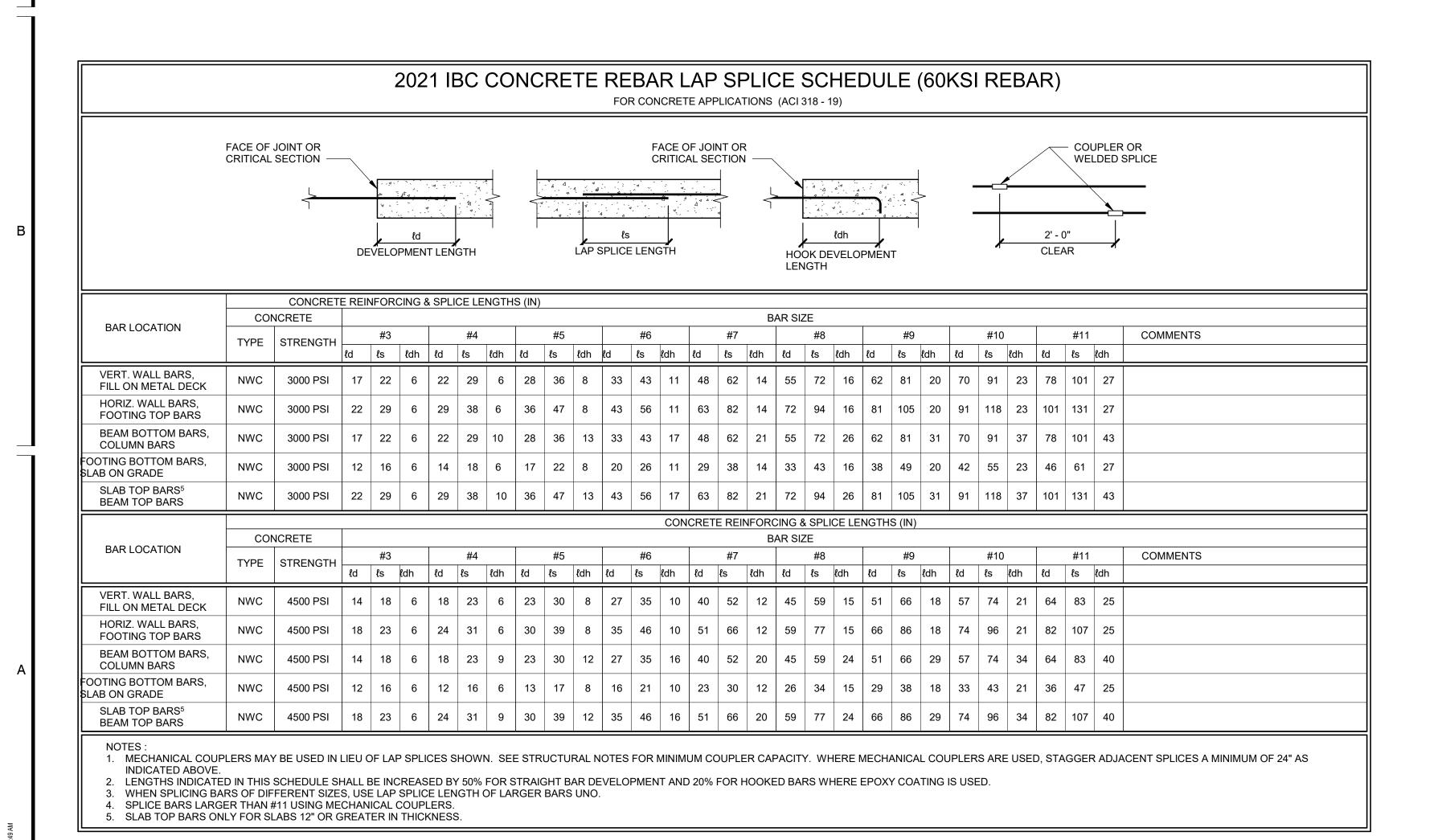
MAXIMUM FLEXIBILITY FACTOR LISTED IN THE SCHEDULE.

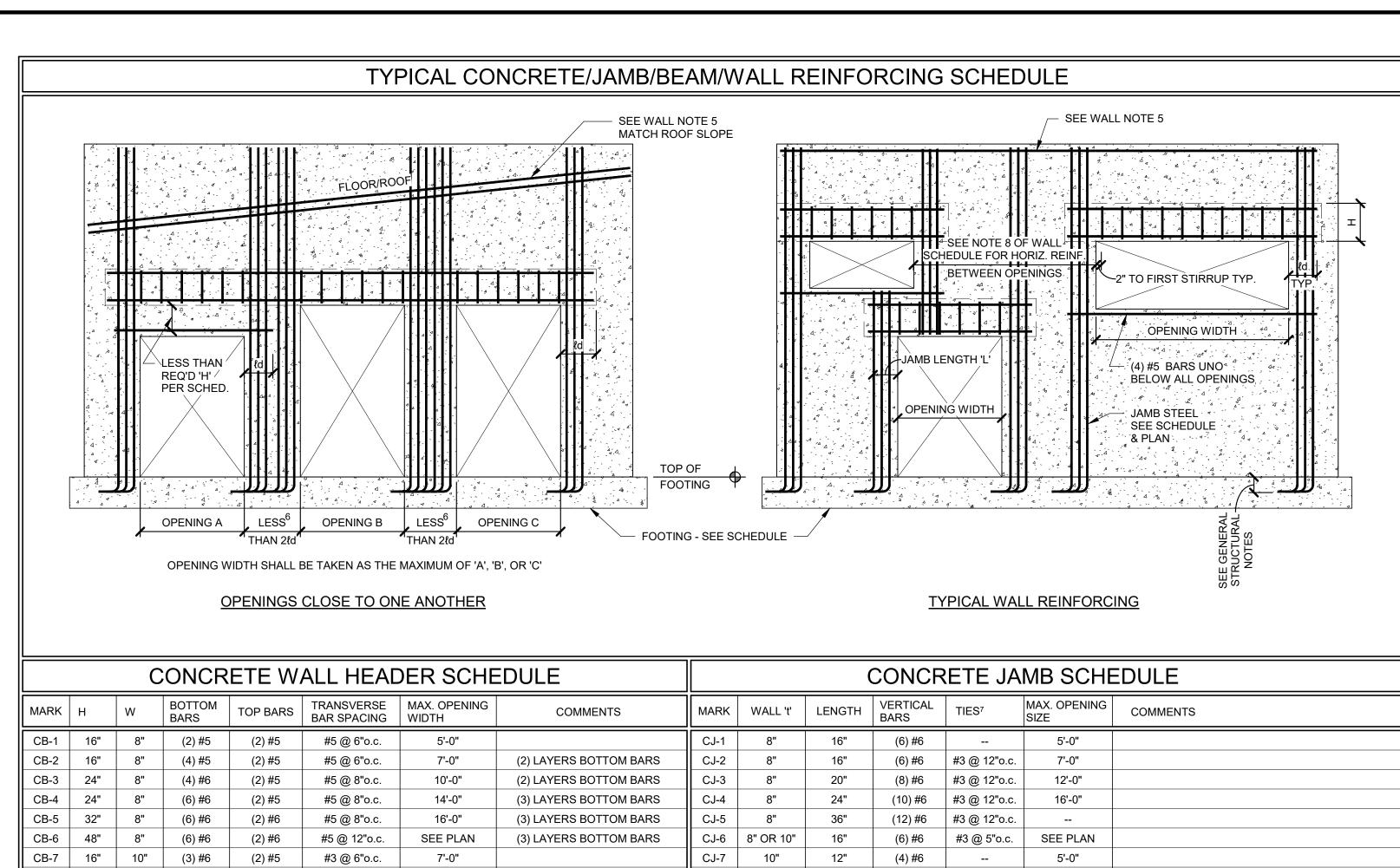


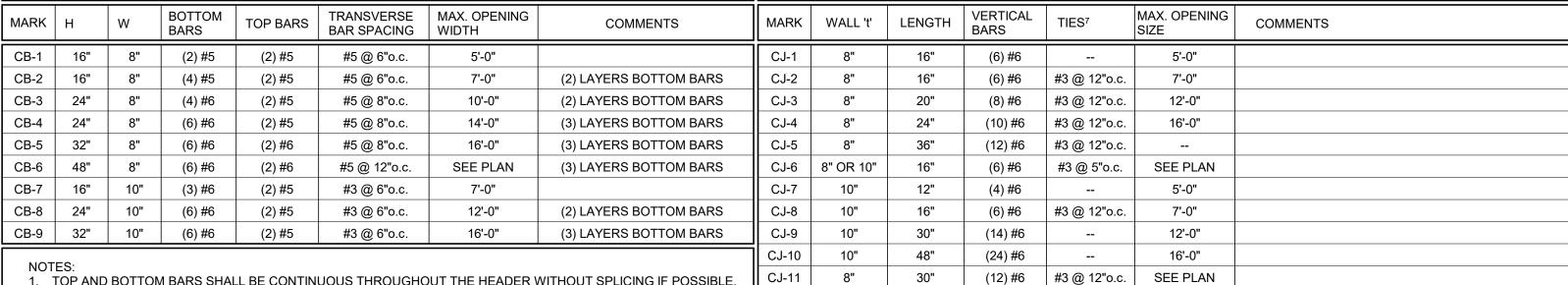


			STANDARD	HOOK & BEN	D SCHEDULE			
TYPE OF STANDARD HOOK	BAR SIZE	MIN. INSIDE BEND DIA. FOR STIRRUPS, TIES, AND HOOPS, in	STRAIGHT EXTENSION \$\ell_{\text{ext}}\$ FOR STIRRUPS, TIES, AND HOOPS in.	MIN. INSIDE BEND DIA. FOR OTHER BARS, in	STRAIGHT EXTENSION <sub>lext</sub> FOR OTHER BARS in.	TYPE OF STANDARD HOOK		
	#3 - #5	4d₀	GREATER OF 6d <sub>b</sub> AND 3"	Gd <sub>b</sub>		POINT AT WHICH BAR IS DEVELOPED —		
90° HOOK	#6 - #8	6d <sub>b</sub>	12d₀	Oup	12d₀	90° BEND		
SO HOOK	#9 - #11	N/A	N/A	8d <sub>b</sub>	12Ub	DIA.		
#14 - #18		N/A	N/A	10d₀		ℓ <sub>dh</sub>		
4050110014	#3 - #5	4d <sub>b</sub>	GREATER OF 6d <sub>b</sub> AND 3"	N/A	N/A	135° BEND		
135° HOOK	#6 - #8	6d <sub>b</sub>	GREATER OF 6db AND 3"	N/A	N/A	DIA.		
	#3 - #5	4d <sub>b</sub>	GREATER OF 4d₀ AND 2.5"	6d <sub>b</sub>		ф		
180° HOOK	#6 - #8	6d <sub>b</sub>	ORLATER OF AUDAND 2.3	Oup	GREATER OF 4d <sub>b</sub>	DIA. 180° BEND		
.55 .7551	#9 - #11	N/A	N/A	8d <sub>b</sub>	AND 2.5"	l <sub>ext</sub> l		
	#14 - #18	N/A	N/A	10d₀		**************************************		



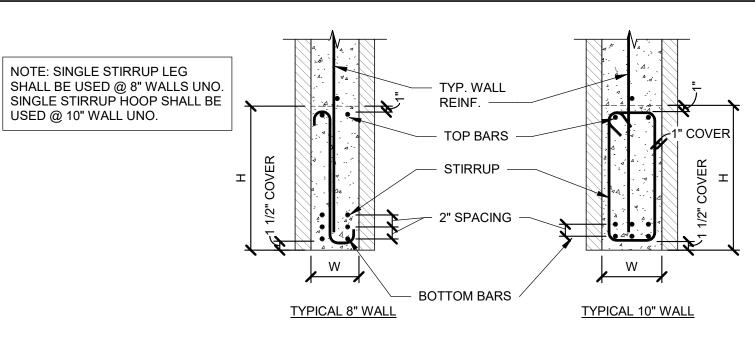


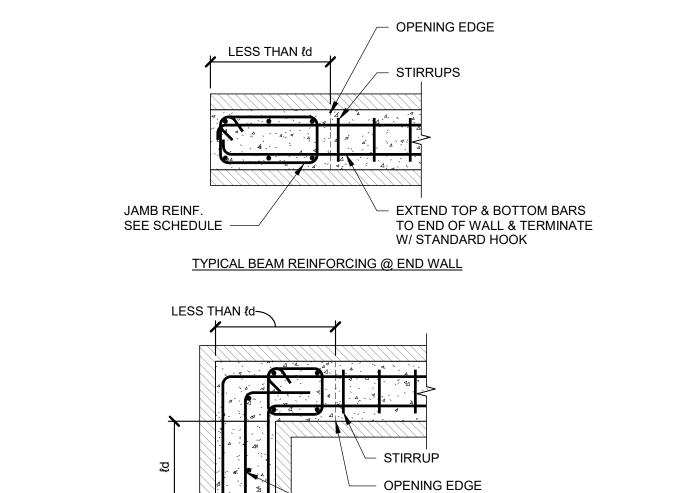




1. TOP AND BOTTOM BARS SHALL BE CONTINUOUS THROUGHOUT THE HEADER WITHOUT SPLICING IF POSSIBLE. . IF SPLICES ARE REQUIRED THEY SHALL NOT OCCUR IN THE MIDDLE HALF SPAN OF THE HEADER. TYPICAL HORIZONTAL WALL REINFORCING NEED NOT OCCUR IN DEPTH OF HEADER. 4. TOP AND BOTTOM BARS SHALL EXTEND PAST THE EDGE OF AN OPENING A MINIMUM DISTANCE OF THE

DEVELOPMENT LENGTH (d; SEE REBAR LAP/SPLICE SCHEDULE 5. NO CONSTRUCTION/EXPANSION JOINTS SHALL OCCUR THROUGH BEAM LENGTH.





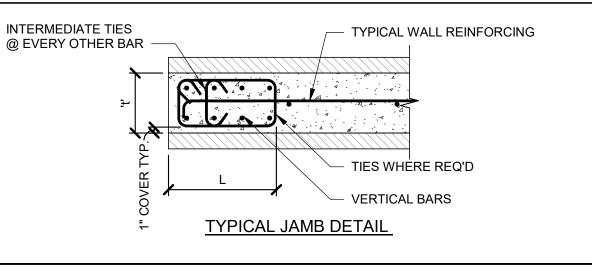
TYPICAL WALL REINFORCING TYPICAL BEAM REINFORCING @ WALL CORNER

CJ-4	8"	24"	(10) #6	#3 @ 12"o.c.	16'-0"					
CJ-5	8"	36"	(12) #6	#3 @ 12"o.c.						
CJ-6	8" OR 10"	16"	(6) #6	#3 @ 5"o.c.	SEE PLAN					
CJ-7	10"	12"	(4) #6		5'-0"					
CJ-8	10"	16"	(6) #6	#3 @ 12"o.c.	7'-0"					
CJ-9	10"	30"	(14) #6		12'-0"					
CJ-10	10"	48"	(24) #6		16'-0"					
CJ-11	J-11 8" 30" (12) #6 #3 @ 12"o.c. SEE PLAN									
NOTES:  1. JAMBS PER OPENING SIZE SHALL BE USED U.N.O. ON PLANS.  2. TIE SPACING IS A MAXIMUM.  3. NO VERTICAL CONSTRUCTION/EXPANSION JOINTS SHALL OCCUR IN JAMB LENGTH 'L'.  4. LAP SPLICES SHALL BE PER REBAR LAP SCHEDULE.  5. JAMB REINFORCING SHALL BE TIED TO FOOTING DOWELS.										

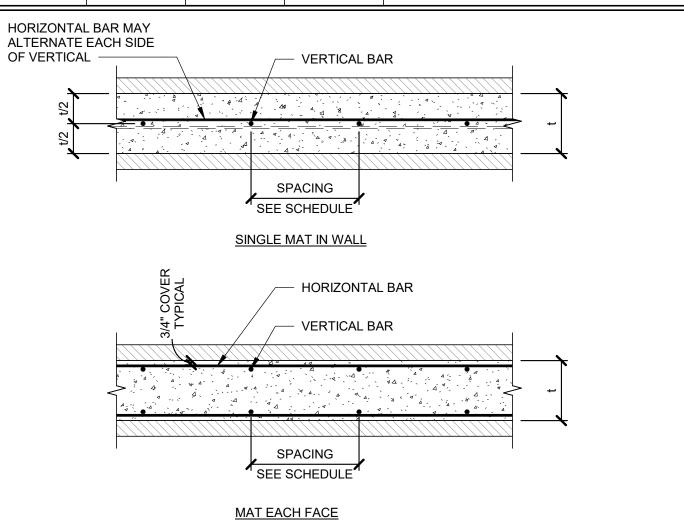
JAMB REINFORCING SHALL BE TIED TO FOOTING DOWELS.

JAMBS SHALL BE AT EACH SIDE OF OPENING. WHERE OPENINGS ARE CLOSE TOGETHER AND NOT SPECIFICALLY

CALLED OUT ON PLANS CONTACT ENGINEER.



CONCRETE WALL SCHEDULE								
	t	VERTICAL REINFORCING		HORIZONT	AL REINFORCING			
MARK		SIZE	SPACING	SIZE	SPACING	COMMENTS		
CW-1	8"	#6	16"o.c.	#5	16"o.c.	TYPICAL WALL UNLESS NOTED OTHERWISE		
CW-2	8"	#6	12"o.c.	#5	16"o.c.	MAT EACH FACE		
CW-3	10"	#6	8"o.c.	#6	16"o.c.			
CW-4	10"	#6	12"o.c.	#6	16"o.c.			
CW-5	8"	#6	16"o.c.	#6	16"o.c.			
CW-6	8"	#6	16"o.c.	#6	16"o.c.	HOOK BARS AROUND VERTICAL REINF. AT END OF WALLS		



 VERTICAL BARS IN SINGLE MAT WALLS SHALL BE CENTERED IN WALL.
 VERTICAL BARS IN DOUBLE MAT WALLS SHALL BE PLACED TIGHT AGAINST HORIZONTAL BARS AS SHOWN AND HELD IN PLACE WHILE PLACING CONCRETE. 3. TYPICAL VERTICAL WALL REINFORCING NEED NOT EXTEND INTO JAMB.

4. TYPICAL HORIZONTAL WALL REINFORCING SHALL EXTEND INTO JAMB BARS AS SHOWN. 5. PROVIDE (4) #5 BARS x CONT. AT ALL FLOOR AND ROOF ELEVATIONS TYPICAL UNLESS NOTED OTHERWISE.

6. U.N.O. USE CW-1 FOR ALL WALLS. 7. PROVIDE (2) #5 BARS x CONT. @ TOP OF WALL WHERE WALL EXTENDS PAST ROOF.

8. SPACE HORIZONTAL REINFORCING @ 8"o.c. FOR WALL SEGMENTS THAT ARE LESS THAN 4'-0" IN LENGTH.

SCHEDULES

MIDD

(6) 7/8"Ø

(7) 7/8"Ø

15" x 2'-0"

15" x 2'-0"

12

BEAM CONNECTION SCHEDULE @ CONCRETE WALL

CONCRETE WALL

3 x 4 x 3/8 x 1'-6"

3 x 4 x 3/8 x 1'-9"

W24x

(2) L 3 x 4 W/ 1 1/2" SLOTTED HÓLES - SEE SCHED.

A325 BOLTS PER THE SCHEDULE WITH WASHERS THAT COMPLETELY COVER THE SLOTS PER THE STRUCTURAL

NOTES. (DO NOT USE TC BOLTS)

					ESTABLISHE	
	FABRICATOR		SPECIAL INSPECTOR			
INSPECTION TASKS PRIOR TO WELDING (TABLE N5.4-1)	QUALITY CO		QUALITY AS:		NOTES	
	CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC		
WELDER QUALIFICATION RECORDS AND CONTINUITY RECORDS	•			•		
WELDING PROCEDURE SPECIFICATIONS (WPSs) AVAILABLE	•		•		PERIODIC - OBSERVE THESE ITEMS ON A RANDOM BASIS.	
MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE	•		•		OPERATIONS NEED NOT BE DELAYED PENDING THESE	
MATERIAL IDENTIFICATION (TYPE / GRADE)		•		•	INSPECTIONS. 2. CONTINUOUS - PERFORM THESE TASKS FOR EACH WELDED JOINT	
WELDER IDENTIFICATION SYSTEM <sup>1</sup>		•		•	OR MEMBER.	
FIT-UP OF GROOVE WELDS (INCLUDING JOINT GEOMETRY)					<ol> <li>QUALITY CONTROL (QC) SHALL BE PROVIDED BY THE FABRICATOR AND ERECTOR.</li> </ol>	
* JOINT PREPARATION					4. QUALITY ASSURANCE (QA) SHALL BE PROVIDED BY OTHERS WHEN	
* DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL)		•		•	REQUIRED BY THE AUTHORITY HAVING JURISDICTION (AHJ), APPLICABLE BUILDING CODE (ABC), PURCHASER, OWNER, OR	
* CLEANLINESS (CONDITION OF STEEL SURFACES)					ENGINEER OF RECORD (EOR). NONDESTRUCTIVE TESTING (NDT)	
* TACKING (TACK WELD QUALITY AND LOCATION)					SHALL BE PERFORMED BY THE AGENCY OR FIRM RESPONSIBLE FOR QUALITY ASSURANCE, EXCEPT AS PERMITTED IN	
* BACKING TYPE AND FIT (IF APPLICABLE)					ACCORDANCE WITH SECTION N6.	
FIT-UP OF CJP GROOVE WELDS OFHSS T-, Y-, AND K-JOINTS WITHOUT BACKING (INCLUDING JOINT GEOMETRY)					<ol> <li>QC AND QA INSPECTORS SHALL BE QUALIFIED IN ACCORDANCE WITH AISC 360-16 CHAPTER N4.</li> <li>NONDESTRUCTIVE TESTING PERSONNEL SHALL BE QUALIFIED IN</li> </ol>	
* JOINT PREPARATIONS				•	ACCORDANCE WITH AISC 360-16 CHAPTER N4.3. 7. NONDESTRUCTIVE TESTING OF WELDED JOINTS SHALL COMPLY	
* DIMENSIONS (ALIGNMENT, ROOT OPENING, ROOT FACE, BEVEL)					WITH AISC 360-16 CHAPTER N5.5a AND b.	
* CLEANLINESS (CONDITION OF STEEL SURFACES)					8. OBSERVATION OF WELDING OPERATIONS AND VISUAL INSPECTION OF IN-PROCESS AND COMPLETED WELDS SHALL BE THE PRIMARY	
* TACKING (TACK WELD QUALITY AND LOCATION)					METHOD TO CONFIRM THAT THE MATERIALS, PROCEDURES AND	
CONFIGURATION AND FINISH OF ACCESS HOLES		•		•	WORKMANSHIP ARE IN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS. FOR STRUCTURAL STEEL, ALL PROVISIONS OF AWS	
FIT-UP OF FILLET WELDS					D1.1 / D1.1M STRUCTURAL WELDING CODE - STEEL FOR	
* DIMENSIONS (ALIGNMENT, GAPS AT ROOT)				•	STATICALLY LOADED STRUCTURES SHALL APPLY.  9. THERMALLY CUT SURFACES OF ACCESS HOLES SHALL BE TESTED	
* CLEANLINESS (CONDITION OF STEEL SURFACES)					BY QA USING MT OR PT, WHEN THE FLANGE THICKNESS EXCEEDS	
* TACKING (TACK WELD QUALITY AND LOCATION)					2 IN. (50mm) FOR ROLLED SHAPES, OR WHEN THE WEB THICKNESS EXCEEDS 2 IN. (50mm) FOR BUILT-UP SHAPES. ANY CRACK SHALL	
CHECK WELDING EQUIPMENT		•			BE DEEMED UNACCEPTABLE REGARDLESS OF SIZE OR LOCATION.  10. WHEN REQUIRED BY APPENDIX 3, TABLE A-3.1, WELDED JOINTS	
<sup>1</sup> THE FABRICATOR OR ERECTOR, AS APPLICABLE, SHALL MAINTAIN A SYSTEM JOINT OR MEMBER CAN BE IDENTIFIED. STAMPS, IF USED, SHALL BE THE LOV			O HAS WELDED	) A	REQUIRING WELD SOUNDNESS TO BE ESTABLISHED BY RADIOGRAPHICS OR ULTRASONIC INSPECTION SHALL BE TESTED	
INSPECTION TASKS DURING WELDING (TABLE N5.4-2)	CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC	BY QA AS PRESCRIBED. REDUCTION IN THE RATE OF UT IS PROHIBITED.	
CONTROL AND HANDLING OF WELDING CONSUMABLES					11. REDUCTION OF RATE OF ULTRASONIC TESTING - THE RATE OF UT IS ONLY PERMITTED TO BE REDUCED IF APPROVED BY THE EOR	
* PACKAGING		•		•	AND THE AHJ PER AISC 360-16 CHAPTER N5.5e.  12. FOR STRUCTURES IN RISK CATEGORY II, WHERE THE INITIAL RATE	
* EXPOSURE CONTROL					FOR UT IS 10%, THE NDT RATE FOR AN INDIVIDUAL WELDER OR	
NO WELDING OVER CRACKED TACK WELDS		•		•	WELDING OPERATOR SHALL BE INCREASED TO 100% SHOULD THE REJECT RATE, THE NUMBER OF WELDS CONTAINING	
ENVIRONMENTAL CONDITIONS					UNACCEPTABLE DEFECTS DIVIDED BY THE NUMBER OF WELDS	
* WIND SPEED WITHIN LIMITS		•		•	COMPLETED, EXCEEDS 5% OF THE WELDS TESTED FOR THE WELDER OR WELDING OPERATOR. A SAMPLING OF AT LEAST 20	
* PRECIPITATION AND TEMPERATURE					COMPLETED WELDS FOR A JOB SHALL BE MADE PRIOR TO	
WPS FOLLOWED					IMPLEMENTING SUCH AN INCREASE. WHEN THE REJECT RATE FOR THE WELDER OR WELDING OPERATOR, AFTER A SAMPLING OF AT	
* SETTINGS ON WELDING EQUIPMENT					LEAST 40 COMPLETED WELDS, HAS FALLEN TO 5% OR LESS, THE	
* TRAVEL SPEED					RATE OF UT SHALL BE RETURNED TO 10%. FOR EVALUATING THE REJECT RATE OF CONTINUOUS WELDS OVER 3 FT (1M) IN LENGTH	
* SELECTED WELDING MATERIALS		•		•	WHERE THE EFFECTIVE THROAT IS 1 IN. (25mm) OR LESS, EACH 12 IN. (300mm) INCREMENT OR FRACTION THEREOF SHALL BE	
* SHIELDING GAS TYPE / FLOW RATE					CONSIDERED AS ONE WELD. FOR EVALUATING THE REJECT RATE	
* PREHEAT APPLIED					ON CONTINUOUS WELDS OVER 3 FT (1M) IN LENGTH WHERE THE EFFECTIVE THROAT IS GREATER THAN 1 IN. (25mm), EACH 6 IN.	
* INTERPASS TEMPERATURE MAINTAINED (MIN. / MAX)					(150mm) OF LENGTH OR FRACTION THEREOF SHALL BE	
* PROPER POSITION (F, V, H, OH)					CONSIDERED ON WELD.  13. ALL NDT PERFORMED SHALL BE DOCUMENTED. FOR SHOP	
WELDING TECHNIQUES					FABRICATION, THE NDT REPORT SHALL IDENTIFY THE TESTED	
* INTERPASS AND FINAL CLEANING					WELD BY PIECE MARK AND LOCATION IN THE PIECE. FOR FIELD WORK, THE NDT REPORT SHALL IDENTIFY THE TESTED WELD BY	
* EACH PASS WITHIN PROFILE LIMITATIONS		•		•	LOCATION IN THE STRUCTURE, PIECE MARK, AND LOCATION IN THE PIECE. WHEN A WELD IS REJECTED ON THE BASIS OF NDT, THE	
* EACH PASS MEETS QUALITY REQUIREMENTS					NDT RECORD SHALL INDICATE THE LOCATION OF THE DEFECT AND	
PLACEMENT AND INSTALLATION OF STEEL HEADED STUD ANCHORS	•		•		THE BASIS OF REJECTION  14. DEMAND CRITICAL WELDS SHALL MEET THE PROVISION FOUND IN	
INSPECTION TASKS AFTER WELDING (TABLE N5.4-3)	CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC	AISC 341-16 AND WELDING METHODS, PROCEDURES AND QUALITY	
WELDS CLEANED					CONTROL SHALL COMPLY WITH AWS D1.1 AND THE FOLLOWING:  a. ARC STRIKES, GOUGES AND OTHER IMPERFECTIONS WITHIN	
SIZE, LENGTH AND LOCATION OF WELDS					OR ADJACENT TO THE JOINT, SHALL BE REPAIRED OR	
WELDS MEET VISUAL ACCEPTANCE CRITERIA					REMOVED. b. PREHEAT AND INTER-PASS REQUIREMENTS AS OUTLINED IN	
* CRACK PROHIBITION					SECTION 3.5.	
* WELD / BASE-METAL FUSION					c. UNREPAIRED CRACKS, GOUGES, AND NOTCHES WILL NOT BE PERMITTED IN THE JOINT AREA.	
* CRATER CROSS SECTION					d. USE ELECTRODES WITH CHARPY V-NOTCH ABSORBED ENERGY EQUAL TO OR GREATER THAN 20 FT-LBS AT 20	
* WELD PROFILES	•		•		DEGREES FAHRENHEIT UNDER AWS A5 CLASSIFICATION TEST	
* WELD SIZE					METHODS, AND 40 FT-LBS AT 70 DEGREES FAHRENHEIT USING TEST PROCEDURES PRESCRIBED IN APPENDIX X OF AISC 358.	
* UNDERCUT					ACCEPTABLE ELECTRODES INCLUDE E70TG-K2, E71 T-1.	
* POROSITY						
ARC STRIKES	•					
K-AREA <sup>1</sup>			•			
WELD ACCESS HOLES IN ROLLED HEAVY SHAPES AND BUILT-UP HEAVY SHAPES <sup>2</sup>	•		•			

REPAIR ACTIVITIES

APPROVAL OF THE EOR

BACKING REMOVED AND WELD TABS REMOVED (IF REQUIRED)

NO PROHIBITED WELDS HAVE BEEN ADDED WITHOUT THE

VISUALLY INSPECT THE WELD ACCESS HOLE FOR CRACKS.

DOCUMENT ACCEPTANCE OR REJECTION OF WELDED JOINT OR MEMBER

VISUALLY INSPECT THE WEB K-AREA FOR CRACKS WITHIN 3 IN. (75mm) OF THE WELD)

1WHEN WELDING OF DOUBLER PLATES, CONTINUITY PLATES OR STIFFENERS HAS BEEN PERFORMED IN THE K-AREA,

<sup>2</sup>AFTER ROLLED HEAVY SHAPES (SEE SECTION A3.1c) AND BUILT-UP HEAVY SHAPES (SEE SECTION A3.1d) ARE WELDED,

**NOTES INSPECTION TASKS PRIOR TO BOLTING (TABLE N5.6-1)** CONTINUOUS | PERIODIC | CONTINUOUS | PERIODIC MANUFACTURER'S CERTIFICATIONS AVAILABLE FOR FASTENER MATERIALS • FASTENERS MARKED IN ACCORDANCE WITH ASTM REQUIREMENTS ODIC - OBSERVE THESE ITEMS ON A RANDOM BASIS. PROPER FASTENERS SELECTED FOR THE JOINT DETAIL (GRADE, TYPE, BOLT RATIONS NEED NOT BE DELAYED PENDING THESE LENGTH IF THREADS ARE TO BE EXCLUDED FROM SHEAR PLANE) TINUOUS - PERFORM THESE TASKS FOR EACH WELDED JOINT PROPER BOLTING PROCEDURES SELECTED FOR JOINT DETAIL • LITY CONTROL (QC) SHALL BE PROVIDED BY THE FABRICATOR CONNECTING ELEMENTS, INCLUDING THE APPROPRIATE FAYING SURFACE CONDITION AND HOLE PREPARATION, IF SPECIFIED, MEET APPLICABLE REQUIREMENTS LITY ASSURANCE (QA) SHALL BE PROVIDED BY OTHERS WHEN UIRED BY THE AUTHORITY HAVING JURISDICTION (AHJ), PRE-INSTALLATION VERIFICATION TESTING BY INSTALLATION PERSONNEL LICABLE BUILDING CODE (ABC), PURCHASER, OWNER, OR OBSERVED AND DOCUMENTED FOR FASTENER ASSEMBLIES AND METHODS USED INEER OF RECORD (EOR). NONDESTRUCTIVE TESTING (NDT) LL BE PERFORMED BY THE AGENCY OR FIRM RESPONSIBLE PROPER STORAGE PROVIDED FOR BOLTS, NUTS, WASHERS AND OTHER FASTENER QUALITY ASSURANCE, EXCEPT AS PERMITTED IN ORDANCE WITH SECTION N6. AND QA INSPECTORS SHALL BE QUALIFIED IN ACCORDANCE **INSPECTION TASKS DURING BOLTING (TABLE N5.6-2)** CONTINUOUS PERIODIC CONTINUOUS PERIODIC I AISC 360-16 CHAPTER N4. IDESTRUCTIVE TESTING PERSONNEL SHALL BE QUALIFIED IN FASTENER ASSEMBLIES, PLACED IN ALL HOLES AND WASHERS (IF REQUIRED) ARE ORDANCE WITH AISC 360-16 CHAPTER N4.3. POSITIONED AS REQUIRED IDESTRUCTIVE TESTING OF WELDED JOINTS SHALL COMPLY I AISC 360-16 CHAPTER N5.5a AND b. JOINT BROUGHT TO THE SNUG-TIGHT CONDITION PRIOR TO THE PRETENSIONING ERVATION OF WELDING OPERATIONS AND VISUAL INSPECTION OPERATION I-PROCESS AND COMPLETED WELDS SHALL BE THE PRIMARY HOD TO CONFIRM THAT THE MATERIALS, PROCEDURES AND FASTENER COMPONENT NOT TURNED BY THE WRENCH PREVENTED FROM ROTATING RKMANSHIP ARE IN CONFORMANCE WITH THE CONSTRUCTION CUMENTS. FOR STRUCTURAL STEEL, ALL PROVISIONS OF AWS FASTENERS ARE PRETENSIONED IN ACCORDANCE WITH THE RCSC SPECIFICATION. D1.1M STRUCTURAL WELDING CODE - STEEL FOR PROGRESSING SYSTEMATICALLY FROM THE MOST RIGID POINT TOWARD THE FREE TICALLY LOADED STRUCTURES SHALL APPLY. RMALLY CUT SURFACES OF ACCESS HOLES SHALL BE TESTED QA USING MT OR PT, WHEN THE FLANGE THICKNESS EXCEEDS (50mm) FOR ROLLED SHAPES, OR WHEN THE WEB THICKNESS

STRUCTURAL STEEL SPECIAL INSPECTION SCHEDULE

ESTABLISHED PER 2021 IBC SECTION 1705.2.1

**INSPECTION TASKS AFTER BOLTING (TABLE N5.6-3)** CONTINUOUS PERIODIC CONTINUOUS PERIODIC DOCUMENT ACCEPTANCE OR REJECTION OF BOLTED CONNECTIONS • PROVISIONS OF THE RCSC SPECIFICATION. **GENERAL STEEL SPECIAL INSPECTION NOTES:** 

PERIODIC - OBSERVE THESE ITEMS ON A RANDOM BASIS. OPERATIONS NEED NOT BE DELAYED PENDING THESE CONTINUOUS - PERFORM THESE TASKS FOR EACH BOLTED CONNECTION. QUALITY CONTROL (QC) SHALL BE PROVIDED BY THE FABRICATOR AND ERECTOR. QUALITY ASSURANCE (QA) SHALL BE PROVIDED BY OTHERS WHEN REQUIRED BY THE AUTHORITY HAVING JURISDICTION (AHJ), APPLICABLE BUILDING CODE (ABC), PURCHASER, OWNER, OR ENGINEER OF RECORD (EOR). NONDESTRUCTIVE TESTING (NDT) SHALL BE PERFORMED BY THE AGENCY OR FIRM RESPONSIBLE FOR QUALITY ASSURANCE, EXCEPT AS PERMITTED IN ACCORDANCE WITH SECTION N7. FOR SNUG-TIGHT JOINTS, PRE-INSTALLATION VERIFICATION TESTING AS SPECIFIED IN TABLE N5.6-1 AND MONITORING OF THE INSTALLATION PROCEDURES AS SPECIFIED IN TABLE N5.6-2 ARE NOT APPLICABLE. THE QCI AND QAI NEED NOT BE PRESENT DURING THE INSTALLATION OF FASTENERS IN SNUG-TIGHT JOINTS. FOR PRETENSIONED JOINTS AND SLIP-CRITICAL JOINTS, WHEN THE INSTALLER IS USING THE TURN-OF-NUT METHOD WITH MATCHMARKING TECHNIQUES, THE DIRECT-TENSION-INDICATOR METHOD, OR THE TWIST-OFF-TYPE TENSION CONTROL BOLT METHOD, MONITORING OF BOLT PRETENSIONING PROCEDURES SHALL BE AS SPECIFIED IN TABLE N5.6-2. THE QCI AND QAI NEED NOT BE PRESENT DURING THE INSTALLATION OF FASTENERS WHEN THESE METHODS ARE USED BY THE INSTALLER. FOR PRETENSIONED JOINTS AND SLIP-CRITICAL JOINTS, WHEN THE INSTALLER IS USING THE CALIBRATED WRENCH METHOD OR THE TURN-OF-NUT METHOD WITHOUT MATCHMARKING, MONITORING OF BOLT PRETENSIONING PROCEDURES SHALL BE AS SPECIFIED IN TABLE N5.6-2. THE QCI AND QAI SHALL BE ENGAGED IN THEIR ASSIGNED INSPECTION DUTIES DURING INSTALLATION OF FASTENERS WHEN THESE METHODS ARE USED BY THE INSTALLER. OBSERVATION OF BOLTING OPERATIONS SHALL BE THE PRIMARY METHOD USED TO CONFIRM THAT THE MATERIALS, PROCEDURES AND WORKMANSHIP INCORPORATED IN CONSTRUCTION ARE IN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS AND THE

NOTES

- QUALITY ASSURANCE (QA) INSPECTION OF FABRICATED ITEMS SHALL BE MADE AT THE FABRICATOR'S PLANT. THE QUALITY ASSURANCE INSPECTOR (QAI) SHALL SCHEDULE THIS WORK TO MINIMIZE INTERRUPTION TO THE WORK OF THE FABRICATOR. QA INSPECTION OF THE ERECTED STEEL SYSTEM SHALL BE MADE AT THE PROJECT SITE. THE QAI SHALL SCHEDULE THIS WORK TO MINIMIZE INTERRUPTION TO THE WORK OF THE ERECTOR. WHERE A TASK IS NOTED TO BE PERFORMED BY BOTH QC AND QA, IT IS PERMITTED TO COORDINATE THE INSPECTION FUNCTION BETWEEN THE QCI AND QAI SO THAT THE INSPECTION FUNCTIONS ARE
- PERFORMED BY ONLY ONE PARTY. WHERE QA RELIES UPON INSPECTION FUNCTIONS PERFORMED BY QC, THE APPROVAL OF THE ENGINEER OF RECORD AND THE AUTHORITY HAVING JURISDICTION IS THE FABRICATOR'S QCI SHALL INSPECT THE FABRICATED STEEL TO VERIFY COMPLIANCE WITH THE DETAILS SHOWN ON THE SHOP DRAWINGS, SUCH AS PROPER APPLICATION OF JOINT DETAILS AT EACH CONNECTION. THE ERECTOR'S QCI SHALL INSPECT THE ERECTED STEEL FRAME TO VERIFY COMPLIANCE WITH THE DETAILS SHOWN ON THE ERECTION DRAWINGS, SUCH AS BRACES, STIFFENERS, MEMBER LOCATIONS AND PROPER APPLICATION OF JOINT DETAILS AT EACH CONNECTION.
- THE QAI SHALL BE ON THE PREMISES FOR INSPECTION DURING THE PLACEMENT OF ANCHOR RODS AND OTHER EMBEDMENTS SUPPORTING STRUCTURAL STEEL FOR COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS. AS A MINIMUM, THE DIAMETER, GRADE, TYPE AND LENGTH OF THE ANCHOR ROD OR EMBEDDED ITEM, AND THE EXTENT OR DEPTH OF EMBEDMENT INTO THE CONCRETE, SHALL BE VERIFIED PRIOR TO PLACEMENT OF THE CONCRETE.
- THE QAI SHALL INSPECT THE FABRICATED STEEL OR ERECTED STEEL FRAME, AS APPROPRIATE, TO VERIFY COMPLIANCE WITH THE DETAILS SHOWN ON THE CONSTRUCTION DOCUMENTS, SUCH AS BRACES, STIFFENERS, MEMBER LOCATIONS AND PROPER APPLICATION OF JOINT DETAILS AT EACH CONNECTION. QUALITY ASSURANCE (QA) INSPECTIONS, EXCEPT NONDESTRUCTIVE TESTING (NDT), MAY BE WAIVED WHEN THE WORK IS PERFORMED IN A FABRICATING SHOP OR BY AN ERECTOR APPROVED BY THE AUTHORITY HAVING JURISDICTION (AHJ) TO PERFORM THE WORK WITHOUT QA. NDT OF WELDS COMPLETED IN AN APPROVED FABRICATOR'S SHOP MAY BE PERFORMED BY THAT FABRICATOR WHEN APPROVED BY THE AHJ. WHEN THE FABRICATOR PERFORMS THE NDT, THE QA AGENCY SHALL REVIEW THE FABRICATOR'S NDT REPORTS.
- AT COMPLETION OF FABRICATION, THE APPROVED FABRICATOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE AHJ STATING THAT THE MATERIALS SUPPLIED AND WORK PERFORMED BY THE FABRICATOR ARE IN ACCORDANCE WITH THE CONSTRUCTION DOCUMENTS. AT COMPLETION OF ERECTION, THE APPROVED ERECTOR SHALL SUBMIT A CERTIFICATE OF COMPLIANCE TO THE AHJ STATING THAT THE MATERIALS SUPPLIED AND WORK PERFORMED BY THE ERECTOR ARE IN ACCORDANCE WITH THE CONSTRUCTION DOCUMENTS.
- IDENTIFICATION AND REJECTION OF MATERIAL OR WORKMANSHIP THAT IS NOT IN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS, SHALL BE PERMITTED AT ANY TIME DURING THE PROGRESS OF THE WORK. HOWEVER, THIS PROVISION SHALL NOT RELIEVE THE OWNER OR THE INSPECTOR OF THE OBLIGATION FOR TIMELY, IN-SEQUENCE INSPECTIONS. NONCONFORMING MATERIAL AND WORKMANSHIP SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE FABRICATOR OR ERECTOR, AS APPLICABLE.

. NONCONFORMING MATERIAL OR WORKMANSHIP SHALL BE BROUGHT INTO CONFORMANCE, OR MADE SUITABLE FOR ITS INTENDED PURPOSE AS DETERMINED BY THE ENGINEER OF RECORD. CONCURRENT WITH THE SUBMITTAL OF SUCH REPORTS TO THE AHJ, EOR OR OWNER, THE QA AGENCY SHALL SUBMIT TO THE FABRICATOR AND ERECTOR: (1) NONCONFORMANCE REPORTS (2) REPORTS OF REPAIR, REPLACEMENT OR ACCEPTANCE OF NONCONFORMING ITEMS.

	PROJECT #:	12
<u> </u>	DRAWN BY:	
41	CHECKED BY:	J. Bla
	ISSUED:	03.14

CHEDULES	
S-013	

PRE-FAB CONSTRUCTION (IBC 1704.2)  REFERENCE NOTES P1 & P2  REFERENCE NOTE P1 & REFERE	REQUIRED WHERE THE WORK IS DONE ON THE PREMISES OF A FABRICATOR REGISTERED AND APPROVE //ITHOUT SPECIAL INSPECTION, PROVIDED THE FABRICATOR COMPLIES WITH IBC.  CATED CONSTRUCTION SHALL BE THE SAME AS IF THE MATERIAL USED IN THE CONSTRUCTION TOOK PLACED ON WILL NOT BE REQUIRED DURING PREFABRICATION IF THE APPROVED AGENCY CERTIFIES THE SHES EVIDENCE OF COMPLIANCE. (SEE NOTE 2).  REQUIRED FOR CONC. ISOLATED SPREAD FOOTINGS, CONTINUOUS FOOTINGS, NON-STRUCTURAL SLABS, DRIVEWAYS, AND SIDEWALKS PROVIDED THE REQUIREMENTS OF IBC 1705.3 ARE MET.  ON IS ALLOWED FOR VERIFICATION OF THE WELDABILITY OF REINFORCING STEEL RESISTING FLEXURAL A INTERPRETATION OF A CONTINUOUS SPECIAL REPORT OF SPECIAL REINFORCED CONCRETE SHEAR REMENT. PERIODIC SPECIAL INSPECTION IS ALLOWED FOR WELDING OF OTHER ASTM A 706 REINFORCING CONTINUOUS SPECIAL INSPECTION REQUIREMENTS NOTED ABOVE.  EMP. TESTS WHEN CONCRETE SAMPLES ARE CAST.  ON IS REQUIRED FOR VERIFICATION OF IN-SITU CONCRETE STRENGTH PRIOR TO STRESSING OF TENDON E AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.  HORS INTO MASONRY OR CONCRETE MAY BE USED ONLY WHEN APPROVED BY ARCHITECT. AND/OR WED PRODUCT WITH CURRENT PUBLISHED ICC RESEARCH REPORT NUMBERS. COORDINATE CIAL INSPECTION REQUIREMENTS WITH ICC REPORT AND ACI 318: 17.8.2.4.  ECTION IS REQUIRED FOR PRECAST CONCRETE DAPHRAGM CONNECTIONS OR REINFORCEMENT AT JOIN OR HIGH DEFORMABILITY ELEMENTS IN STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY C, D, E, OON IS REQUIRED FOR THE INSTALLATION TOLERANCES OF PRECAST CONCRETE IDAPHRAGM CONNECTION ON THE INSTALLATION TOLERANCES OF PRECAST CONCRETE IDAPHRAGM CONNECTION ON THE INSTALLATION TOLERANCES OF PRECAST CONCRETE IDAPHRAGM CONNECTION ON THE INSTALLATION TOLERANCES OF PRECAST CONCRETE IDAPHRAGM CONNECTION ON THE INSTALLATION TOLERANCES OF PRECAST CONCRETE IDAPHRAGM CONNECTION ON THE INSTALLATION TOLERANCES OF PRECAST CONCRETE IDAPHRAGM CONNECTION ON THE INSTALLATION TOLERANCES OF PRECAST CONCRETE IDAPHRAGM CONNECTION.	
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SPECIFICATIONS LISTED IN SECTION 2207.1  CONCRETE CONSTRUCTION (IBC 1705.3)  SEE IBC TABLE 1706.3 - REF. NOTE C1  C1. SPECIAL INSPECTION IS NOTE C1  C2. SPECIAL INSPECTION IS NOTE C2  WELDING OF REINFORCING STEEL	S, DRIVEWAYS, AND SIDEWALKS PROVIDED THE REQUIREMENTS OF IBC 1705.3 ÅRE MET.  ION IS ALLOWED FOR VERIFICATION OF THE WELDABILITY OF REINFORCING STEEL RESISTING FLEXURAL A  IATE AND SPECIAL MOMENT FRAMES, BOUNDARY ELEMENTS OF SPECIAL REINFORCED CONCRETE SHEAF  RCEMENT. PERIODIC SPECIAL INSPECTION IS ALLOWED FOR WELDING OF OTHER ASTM A 706 REINFORCING  E CONTINUOUS SPECIAL INSPECTION REQUIREMENTS NOTED ABOVE.  TEMP. TESTS WHEN CONCRETE SAMPLES ARE CAST.  ION IS REQUIRED FOR VERIFICATION OF IN-SITU CONCRETE STRENGTH PRIOR TO STRESSING OF TENDON  IE AND PRIOR TO REMOVAL OF SHORES AND FORMS FROM BEAMS AND STRUCTURAL SLABS.  CHORS INTO MASONRY OR CONCRETE MAY BE USED ONLY WHEN APPROVED BY ARCHITECT. AND/OR  VED PRODUCT WITH CURRENT PUBLISHED ICC RESEARCH REPORT NUMBERS. COORDINATE  CIAL INSPECTION REQUIREMENTS WITH ICC REPORT AND ACI 318: 17.8.2.4.  ECTION IS REQUIRED FOR PRECAST CONCRETE DIAPHRAGM CONNECTIONS OR REINFORCEMENT AT JOIN  OR HIGH DEFORMABILITY ELEMENTS IN STRUCTURES ASSIGNED TO SEISMIC DESIGN CATEGORY C, D, E, O  ION IS REQUIRED FOR THE INSTALLATION TOLERANCES OF PRECAST CONCRETE DIAPHRAGM CONNECTION  550.5.  ION IS REQUIRED FOR FORMWORK SHAPE, LOCATION AND DIMENSIONS OF THE CONCRETE MEMBER BEIN  ION IS ALLOWED FOR VERIFICATION OF THE WELDABILITY OF REINFORCING STEEL OTHER THAN ASTM A  NSI / AWS D1.4. CONTINUOUS SPECIAL INSPECTION IS REQUIRED FOR REINFORCING STEEL RESISTING	
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MORTAR JOINTS  MORTAR JOINTS  REINFORCEMENT / CONNECTORS  PRE-STRESSING TECHNIQUES  GRADE & SIZE OF TENDONS & ANCHORAGES  INSPECTION SHALL VERIFY:  M2. CONTINUOUS SPECIAL INSI M3. EPOXY AND EXPANSION AN ENGINEER USING AN APPR CONTINUOUS/PERIODIC SP	IND SHEAR REINFORCEMENT. PERIODIC SPECIAL INSPECTION IS ALLOWED FOR WELDING OF OTHER TEEL NOT INCLUDED IN THE CONTINUOUS SPECIAL INSPECTION REQUIREMENTS NOTED ABOVE.	
REINFORCEMENT / CONNECTORS  PRE-STRESSING TECHNIQUES  GRADE & SIZE OF TENDONS & ANCHORAGES  INSPECTION SHALL VERIFY:  ENGINEER USING AN APPR CONTINUOUS/PERIODIC SP	CONTINUOUS SPECIAL INSPECTION IS REQUIRED FOR ESSENTIAL FACILITIES (TMS 602-16/ACI 530.1 TABLE 3).  EPOXY AND EXPANSION ANCHORS INTO MASONRY OR CONCRETE MAY BE USED ONLY WHEN APPROVED BY ARCHITECT AND/OR ENGINEER USING AN APPROVED PRODUCT WITH CURRENT PUBLISHED ICC RESEARCH REPORT NUMBERS. COORDINATE CONTINUOUS/PERIODIC SPECIAL INSPECTION REQUIREMENTS WITH ICC REPORT.	
PRE-STRESSING TECHNIQUES  GRADE & SIZE OF TENDONS & ANCHORAGES  INSPECTION SHALL VERIFY:		
GRADE & SIZE OF TENDONS & ANCHORAGES  INSPECTION SHALL VERIFY:		
INSPECTION SHALL VERIFY:		
SIZE & LOCATION OF STRUCTURAL ELEMENTS		
TYPE, SIZE, & LOCATION OF ANCHORS  • REFERENCE NOTE M2		
SIZE, GRADE & TYPE OF REINFORCEMENT		
WELDING OF REINFORCING BARS    REFERENCE NOTE M1		
HOT OR COLD WEATHER PROTECTION ●		
MEASUREMENT OF PRE-STRESSING FORCE ● REFERENCE NOTE M2		
PRIOR TO GROUTING, VERIFY:		
CLEAN GROUT SPACE  • REFERENCE NOTE M2		
PLACEMENT OF REINFORCEMENT CONNECTORS, TENDONS AND ANCHORS.		
PROPORTIONS OF SITE PREPARED GROUT		
CONSTRUCTION OF MORTAR JOINTS		
GROUT PLACEMENT		
PREPARATION OF TEST SPECIMENS / PRISMS		
COMPLIANCE W/ CONST. DOCS. / SUBMITTALS		
EPOXY / EXPANSION ANCHOR PLACEMENT    REFERENCE NOTE M3		
VERIFICATION OF f'm AND f'aac		
SELF CONSOLIDATING GROUT:		
VERIFY SLUMP FLOW AND VSI  SOIL S (IDC 1705 6)	W O O O O O O O O O O O O O O O O O O O	
VERIFY ADEQUATE MATERIALS BELOW FOOTINGS   REFERENCE NOTE F1  F2. WHERE GEOTECHNICAL RED DENSITY OF THE COMPACT	SPECIAL INSPECTION OF SOILS SHALL REFERENCE THE APPROVED GEOTECHNICAL REPORT TO DETERMINE COMPLIANCE. WHERE GEOTECHNICAL REPORT IS NOT PROVIDED SPECIAL INSPECTIONS ARE REQUIRED TO VERIFY THAT THE IN-PLACE DEDENSITY OF THE COMPACTED FILL IS NOT LESS THAN 90 PERCENT OF THE MAXIMUM DRY DENSITY AT OPTIMUM MOISTURE	
EXCAVATIONS EXTEND TO PROPER DEPTH AND REACH PROPER MATERIAL  REFERENCE NOTE F2  CONTENT DETERMINED IN F3. CONTINUOUS SPECIAL INSI	CONTENT DETERMINED IN ACCORDANCE WITH ASTM D1557.	
	PLACEMENT AND COMPACTION OF COMPACTED FILL.	
FILL MATERIAL AND PLACEMENT   REFERENCE NOTE F3	and the same of th	
PROPERLY PREPARED SITE AND SUB-GRADE PRIOR TO FILL.  REFERENCE NOTE F1		

THE ITEMS MARKED WITH A "O" IN THE SPECIAL INSPECTION SCHEDULE SHALL BE INSPECTED IN ACCORDANCE WITH IBC CHAPTER 17 BY A CERTIFIED SPECIAL INSPECTOR FROM AN ESTABLISHED TESTING AGENCY. FOR MATERIAL SAMPLING AND TESTING REQUIREMENTS, REFER TO THE MATERIAL SAMPLING AND TESTING SECTION, THE PROJECT SPECIFICATIONS, AND THE SPECIFIC GENERAL NOTES SECTIONS. THE TESTING AGENCY SHALL SEND COPIES OF ALL STRUCTURAL TESTING AND INSPECTION REPORTS DIRECTLY TO THE ARCHITECT, ENGINEER, CONTRACTOR, AND BUILDING OFFICIAL. ANY ITEMS WHICH FAIL TO COMPLY WITH THE APPROVED CONSTRUCTION DOCUMENTS SHALL IMMEDIATELY BE BROUGHT TO THE ATTENTION OF THE BUILDING OFFICIAL, ARCHITECT, AND ENGINEER PRIOR TO COMPLETION OF THAT PHASE OF WORK. SPECIAL INSPECTION TESTING REQUIREMENTS APPLY EQUALLY TO ALL BIDDER DESIGNED COMPONENTS.

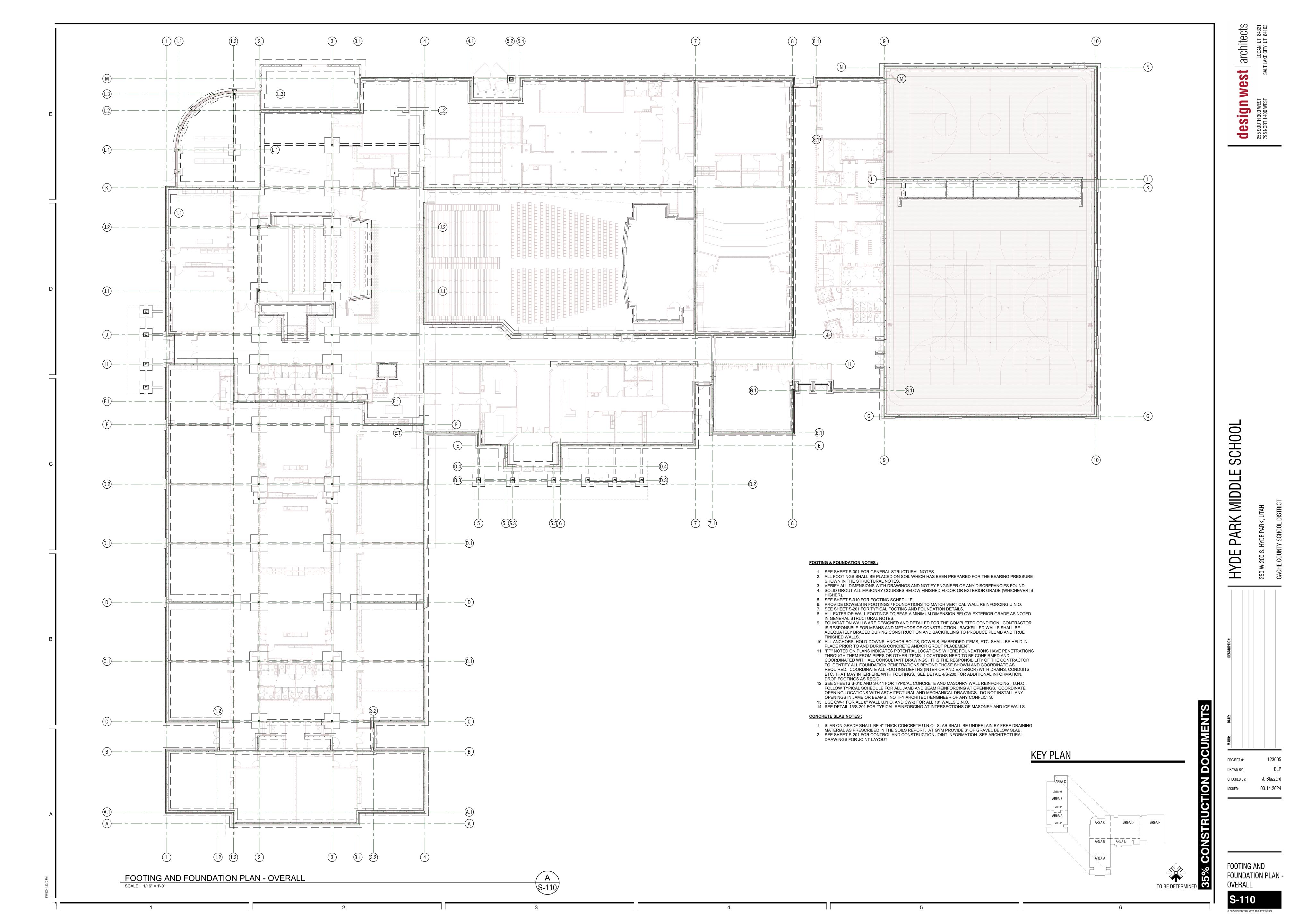
ANY CONSTRUCTION OR MATERIAL THAT HAS FAILED INSPECTION SHALL BE SUBJECT TO REMOVAL AND REPLACEMENT. CONTINUOUS SPECIAL INSPECTION MEANS THE FULL-TIME OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK IS BEING PERFORMED. PERIODIC SPECIAL INSPECTION MEANS THE PART-TIME OR INTERMITTENT OBSERVATION OF WORK REQUIRING SPECIAL INSPECTION BY AN APPROVED SPECIAL INSPECTOR WHO IS PRESENT IN THE AREA WHERE THE WORK HAS BEEN OR IS BEING PERFORMED AND AT THE COMPLETION OF THE WORK. (IBC SECTION 202)

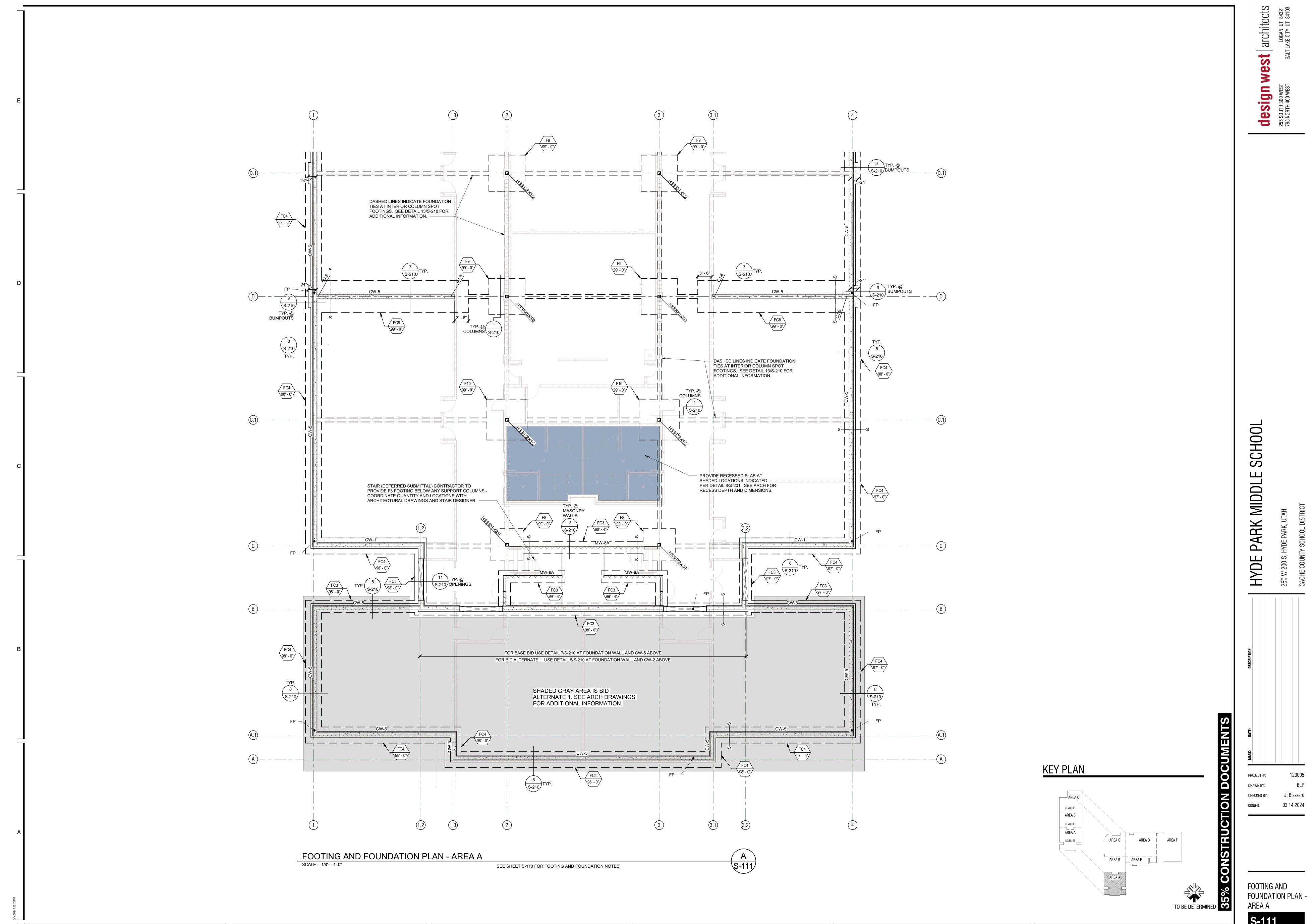
# COLD-FORMED STEEL DECK SPECIAL INSPECTION SCHEDULE

ESTABLISH	ED PER 2021	IBC SEC	TION 1705.2	2.2 AND S	DI QA/QC
INSPECTION TASKS PRIOR TO DECK PLACEMENT (TABLE 1.1)	INSTAL QUALITY C	ONTROL	TROL QUALITY ASSURANCE		NOTES
VERIFY COMPLIANCE OF MATERIALS (DECK AND ALL DECK ACCESSORIES) WITH CONSTRUCTION DOCUMENTS, INCLUDING PROFILES, MATERIAL PROPERTIES, AND BASE METAL THICKNESS	CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC	<ol> <li>PERIODIC - INSPECT THESE ITEMS ON AN INTERMITTENT BASIS. OPERATIONS NEED NOT BE DELAYED PENDING THESE INSPECTIONS. FREQUENCY OF OBSERVATIONS SHALL BE ADEQUATE TO CONFIRM THAT THE WORK HAS BEEN PERFORMED IN</li> </ol>
DOCUMENT ACCEPTANCE OR REJECTION OF DECK AND DECK ACCESSORIES	•		•		ACCORDANCE WITH THE APPLICABLE DOCUMENTS. ADDITIONAL INSPECTIONS SHALL BE PERFORMED TO DETERMINE THE EXTENT OF NON-CONFORMANCE.
INSPECTION TASKS AFTER DECK PLACEMENT (TABLE 1.2)	CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC	2. CONTINUOUS - PERFORM THESE TASKS PRIOR TO FINAL ACCEPTANCE FOR EACH ITEM OR ELEMENT.
VERIFY COMPLIANCE OF DECK AND ALL DECK ACCESSORIES INSTALLATION WITH CONSTRUCTION DOCUMENTS	•		•		3. WITHIN THE LISTED TASKS, "DOCUMENT" SHALL MEAN THE INSPECTOR SHALL PREPARE REPORTS OR OTHER APPROPRIATE WRITTEN DOCUMENTATION INDICATING THAT THE WORK HAS OR HAS NOT BEEN PERFORMED IN ACCORDANCE
VERIFY DECK MATERIALS ARE REPRESENTED BY THE MILL CERTIFICATIONS THAT COMPLY WITH THE CONSTRUCTION DOCUMENTS	•		•		WITH THE CONSTRUCTION DOCUMENTS.
DOCUMENT ACCEPTANCE OR REJECTION OF INSTALLATION OF DECK AND DECK ACCESSORIES	•		•		
INSPECTION TASKS PRIOR TO WELDING (TABLE 1.3)	CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC	
WELDING PROCEDURE SPECIFICATIONS (WPSs) AVAILABLE		•		•	
MANUFACTURER CERTIFICATIONS FOR WELDING CONSUMABLES AVAILABLE		•		•	
MATERIAL IDENTIFICATION (TYPE/GRADE)		•			
CHECK WELDING EQUIPMENT		•		•	
INSPECTION TASKS DURING WELDING (TABLE 1.4)	CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC	
USE OF QUALIFIED WELDERS					
CONTROL AND HANDLING OF WELDING CONSUMABLES		•		•	
ENVIRONMENTAL CONDITIONS (WIND SPEED, MOISTURE, TEMPERATURE)		•		•	
WPS FOLLOWED		•		•	
INSPECTION TASKS AFTER WELDING (TABLE 1.5)	CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC	
VERIFY SIZE AND LOCATIONS OF WELDS, INCLUDING SUPPORT, SIDELAP, AND PERIMETER WELDS	•		•		
WELDS MEET VISUAL ACCEPTANCE CRITERIA	•		•		
VERIFY REPAIR ACTIVITIES	•		•		
DOCUMENT ACCEPTANCE OR REJECTION OF WELDS	•		•		
INSPECTION TASKS PRIOR TO MECHANICAL FASTENING (TABLE 1.6)	CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC	
MANUFACTURER INSTALLATION INSTRUCTIONS AVAILABLE FOR MECHANICAL FASTENERS		•		•	
PROPER TOOLS AVAILABLE FOR FASTENER INSTALLATION		•		•	
PROPER STORAGE FOR MECHANICAL FASTENERS		•		•	
INSPECTION TASKS DURING MECHANICAL FASTENING (TABLE 1.7)	CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC	
FASTENERS ARE POSITIONED AS REQUIRED		•		•	
FASTENERS ARE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S INSTRUCTIONS		•		•	
INSPECTION TASKS AFTER MECHANICAL FASTENING (TABLE 1.8)	CONTINUOUS	PERIODIC	CONTINUOUS	PERIODIC	
CHECK SPACING, TYPE, AND INSTALLATION OF SUPPORT FASTENERS	•		•		
CHECK SPACING, TYPE, AND INSTALLATION OF SIDELAP FASTENERS	•		•		
CHECK SPACING, TYPE, AND INSTALLATION OF PERIMETER FASTENERS	•		•		
VERIFY REPAIR ACTIVITIES	•		•		
DOCUMENT ACCEPTANCE OR REJECTION OF MECHANICAL FASTENERS	•		•		

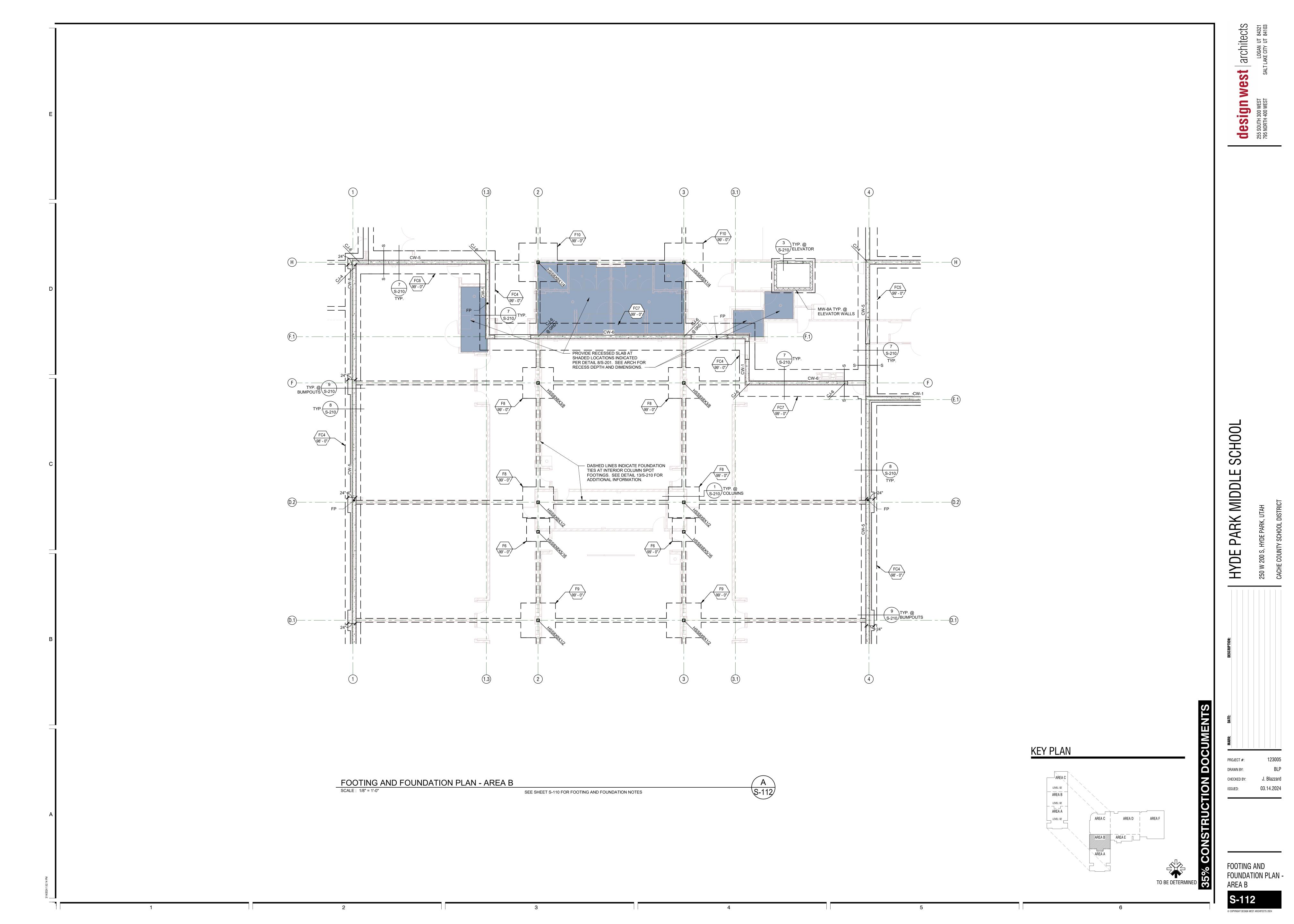
# **GENERAL STEEL DECK SPECIAL INSPECTION NOTES:**

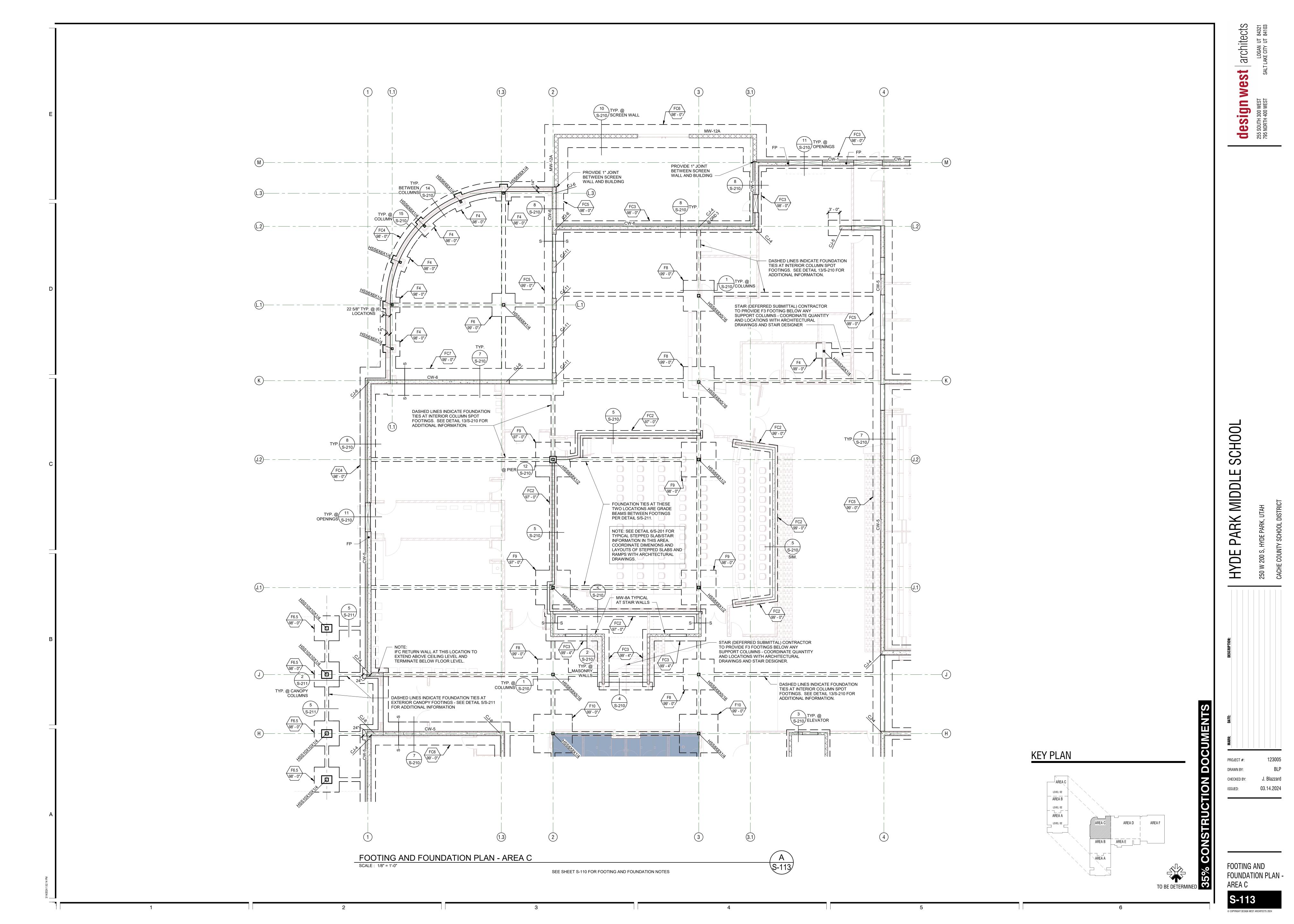
- FOR QUALITY CONTROL INSPECTION, THE CONSTRUCTION DOCUMENTS, INSTALLATION DRAWINGS, SHOP DRAWINGS, DESIGN DOCUMENTS AND THE APPLICABLE REFERENCED STANDARDS SHALL BE UTILIZED. 3. QUALITY ASSURANCE INSPECTION OF THE DECK SHALL BE MADE AT THE PROJECT SITE. THE OWNER'S DESIGNATED REPRESENTATIVE FOR CONSTRUCTION SHALL SCHEDULE THIS WORK WITH THE QUALITY ASSURANCE INSPECTOR (QAI) AND
- THE INSTALLER TO MINIMIZE INTERRUPTIONS TO THE WORK OF THE INSTALLER. 4. THE QAI SHALL REVIEW THE MATERIALS TEST REPORTS AND CERTIFICATIONS LISTED IN SECTION 2.2 OF SDI QA/QC FOR COMPLIANCE WITH THE CONSTRUCTION DOCUMENTS.
- 5. QUALITY ASSURANCE TASKS SHALL BE PERFORMED BY THE QAI. 6. WHERE A TASK IS TO BE PERFORMED BY BOTH QA AND QC, IT SHALL BE PERMITTED TO COORDINATE INSPECTION FUNCTIONS BETWEEN THE QCI AND QAI SO THAT THE INSPECTIONS ARE PERFORMED BY ONLY ONE PARTY WHEN APPROVED
- IN ADVANCE BY THE OWNER, DESIGNER, AND AHJ. WHEN QA TASKS ARE PERFORMED ONLY BY THE QCI, EACH INSPECTION IS TO BE DOCUMENTED IN A REPORT AND THE QAI SHALL PERIODICALLY REVIEW THE WORK OF THE QCI AT AN INTERVAL ACCEPTABLE TO THE OWNER, DESIGNER, AND THE AHJ.
- IN THE EVENT THAT THE REQUIREMENTS OF THE CONSTRUCTION DOCUMENTS CONFLICT WITH THE INSTALLATION DRAWINGS OR SHOP DRAWINGS, THE REQUIREMENTS OF THE CONSTRUCTION DOCUMENTS SHALL GOVERN. 8. IDENTIFICATION AND REJECTION OF MATERIALS AND WORKMANSHIP NOT IN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS SHALL BE PERMITTED AT ANY TIME DURING PROGRESS OF OR FOLLOWING THE COMPLETION OF THE WORK. HOWEVER, THIS PROVISION SHALL NOT RELIEVE THE OWNER OR THE INSPECTOR OF THE OBLIGATION FOR TIMELY, IN-SEQUENCE INSPECTIONS. NONCOMFORMING MATERIAL OR WORKMANSHIP SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE OWNERS DESIGNATED REPRESENTATIVE FOR CONSTRUCTION AND THE DECK INSTALLER. NONCONFORMING MATERIAL OR WORKMANSHIP SHALL BE BROUGHT IN CONFORMANCE, OR MADE SUITABLE FOR ITS INTENDED PURPOSE AS DETERMINED BY THE DESIGNER.

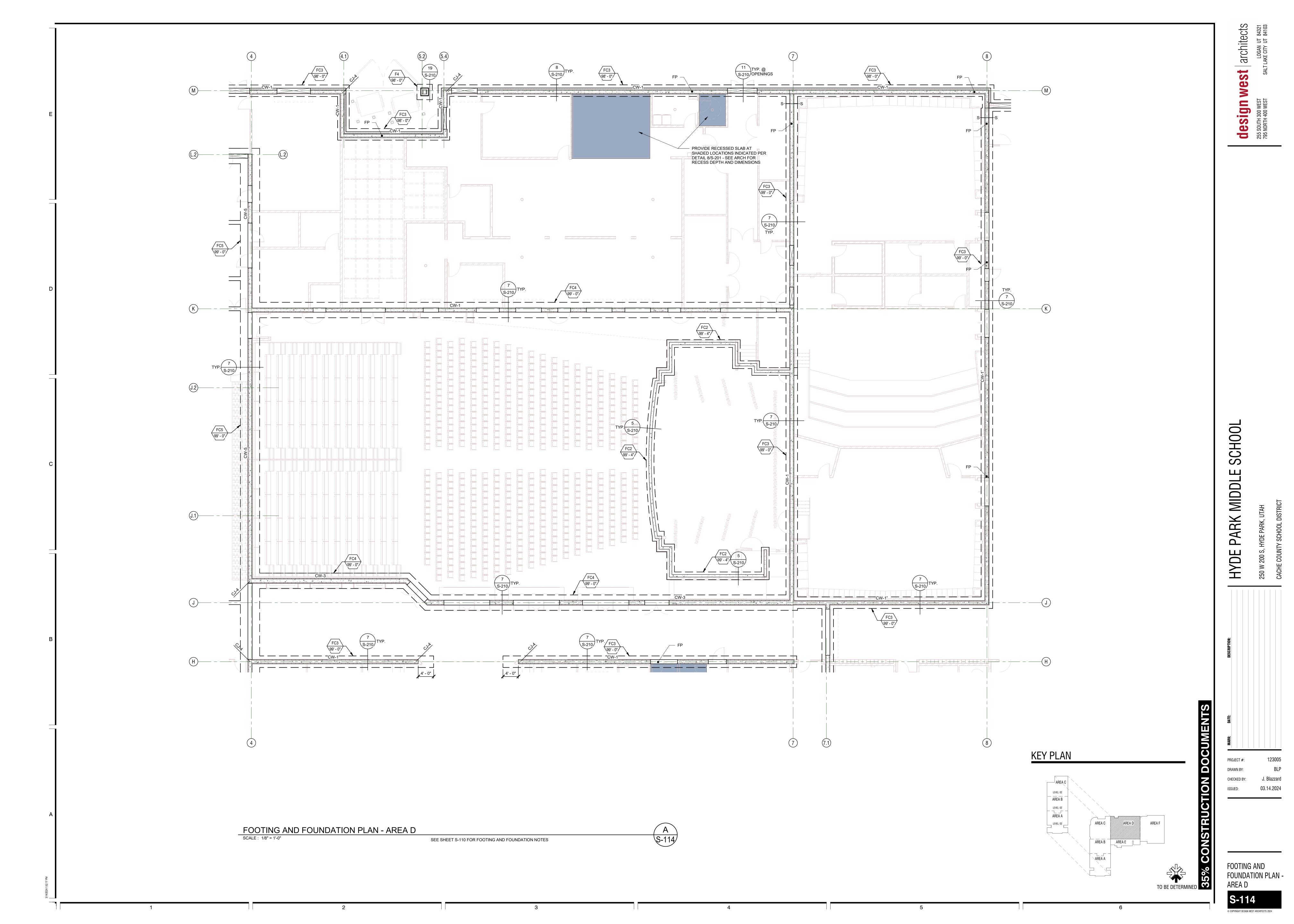


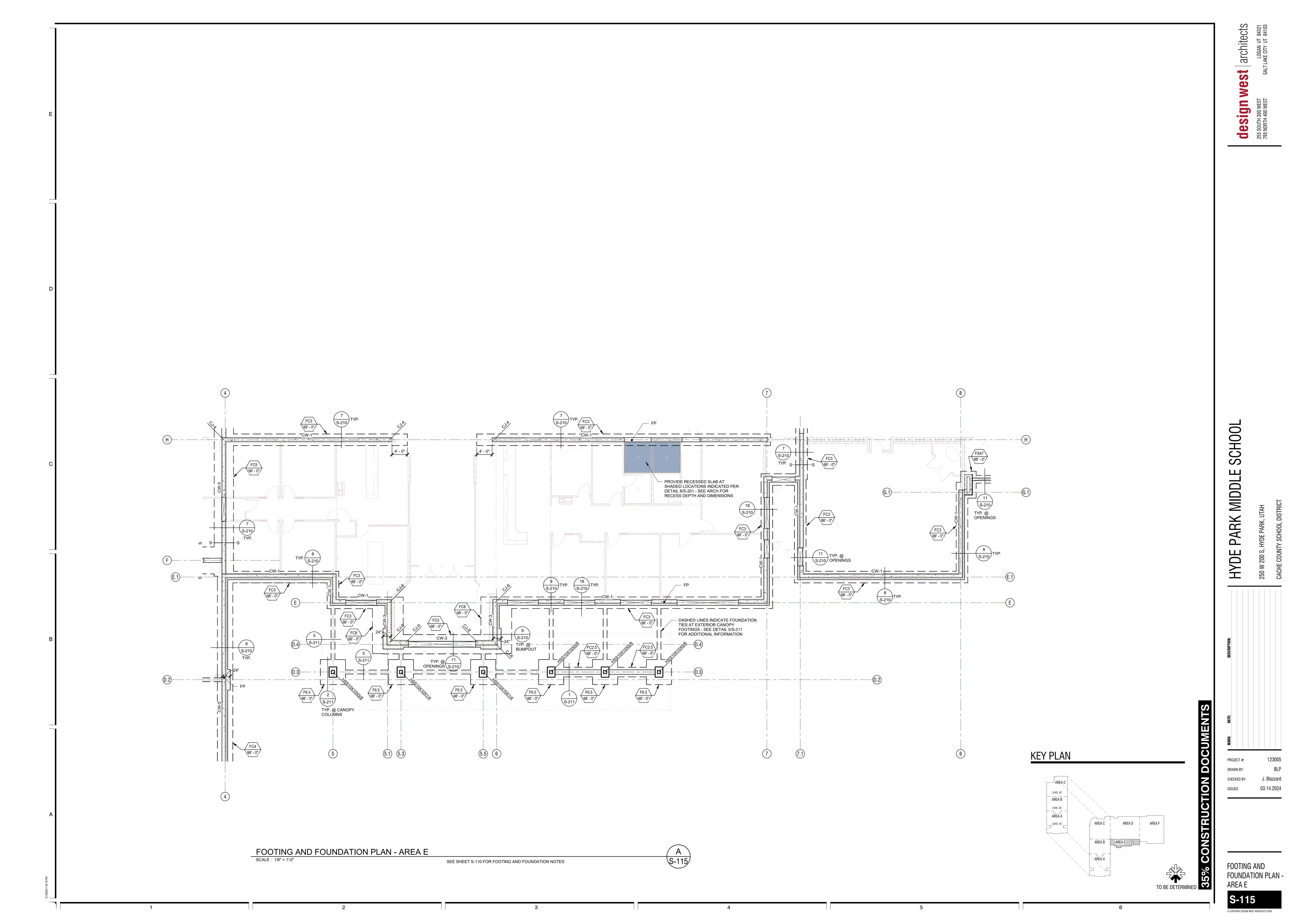


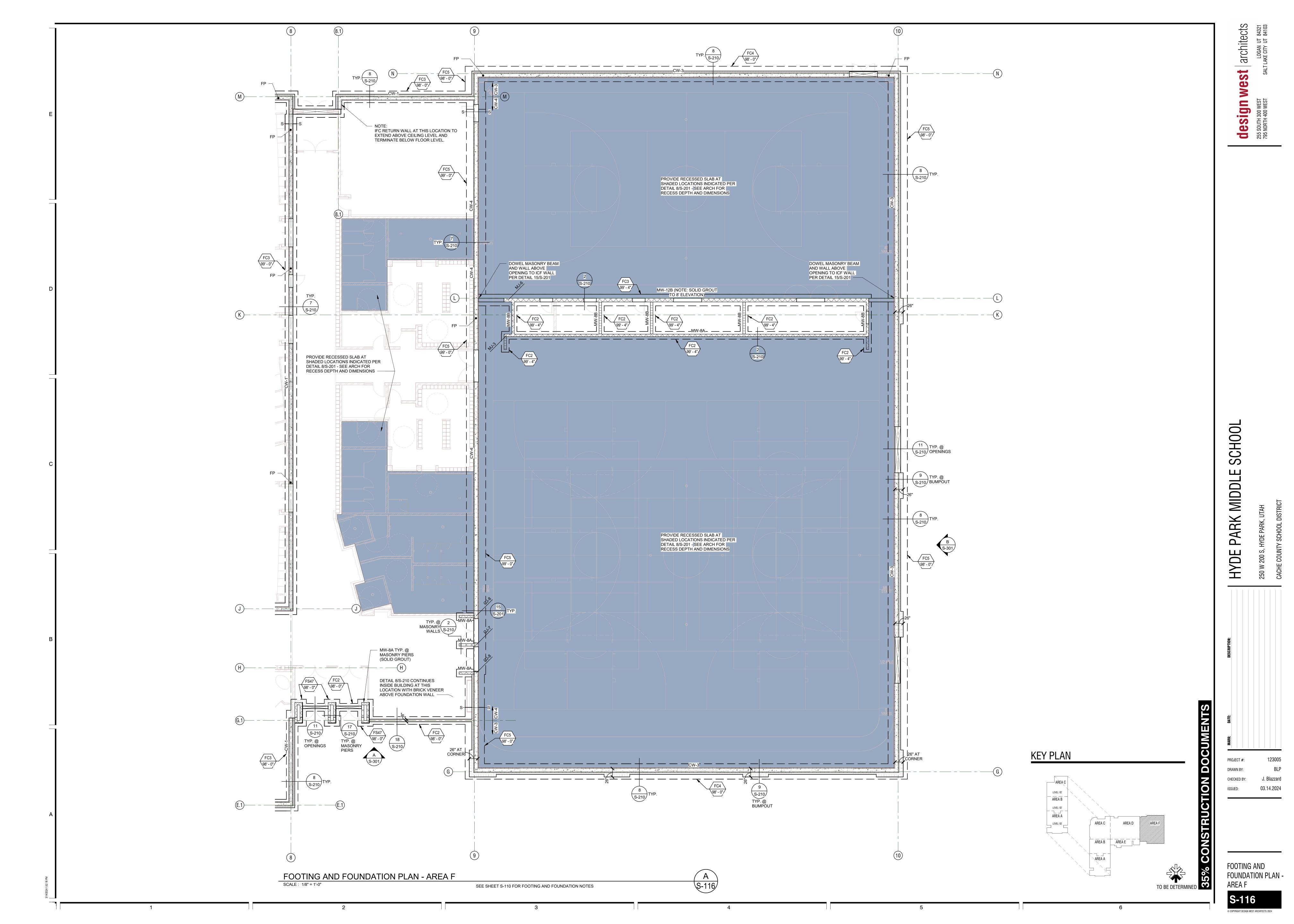
S-111

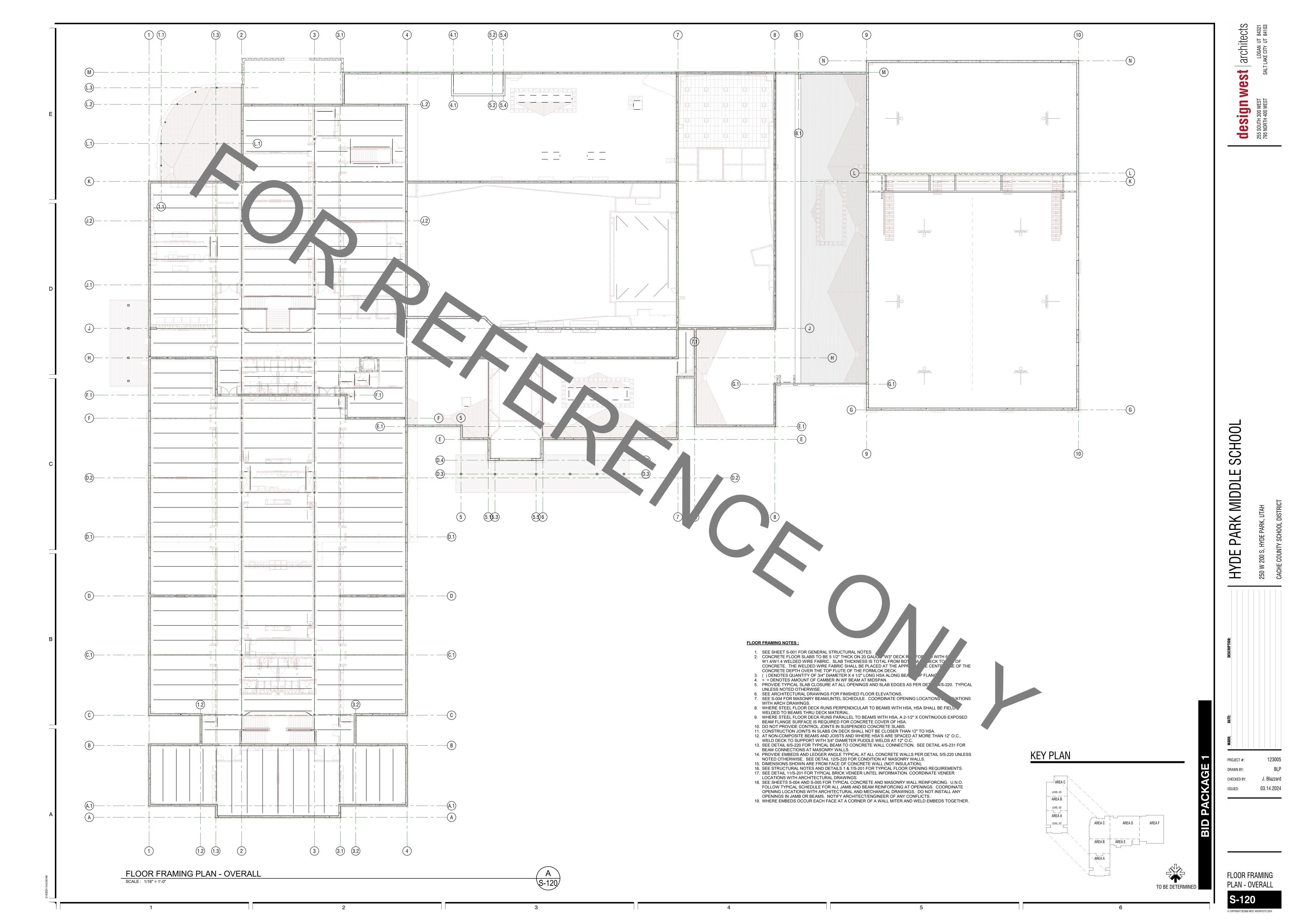


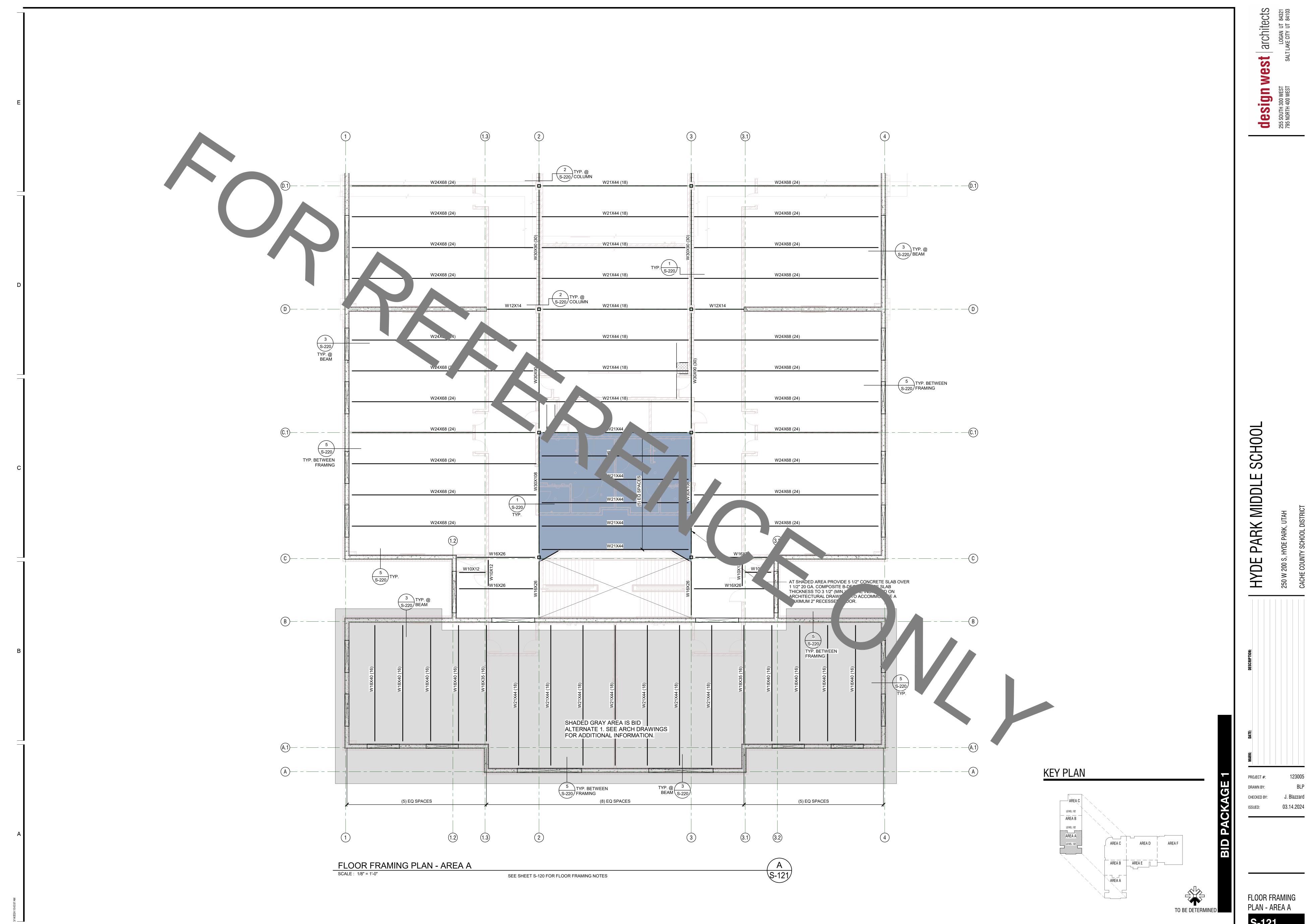


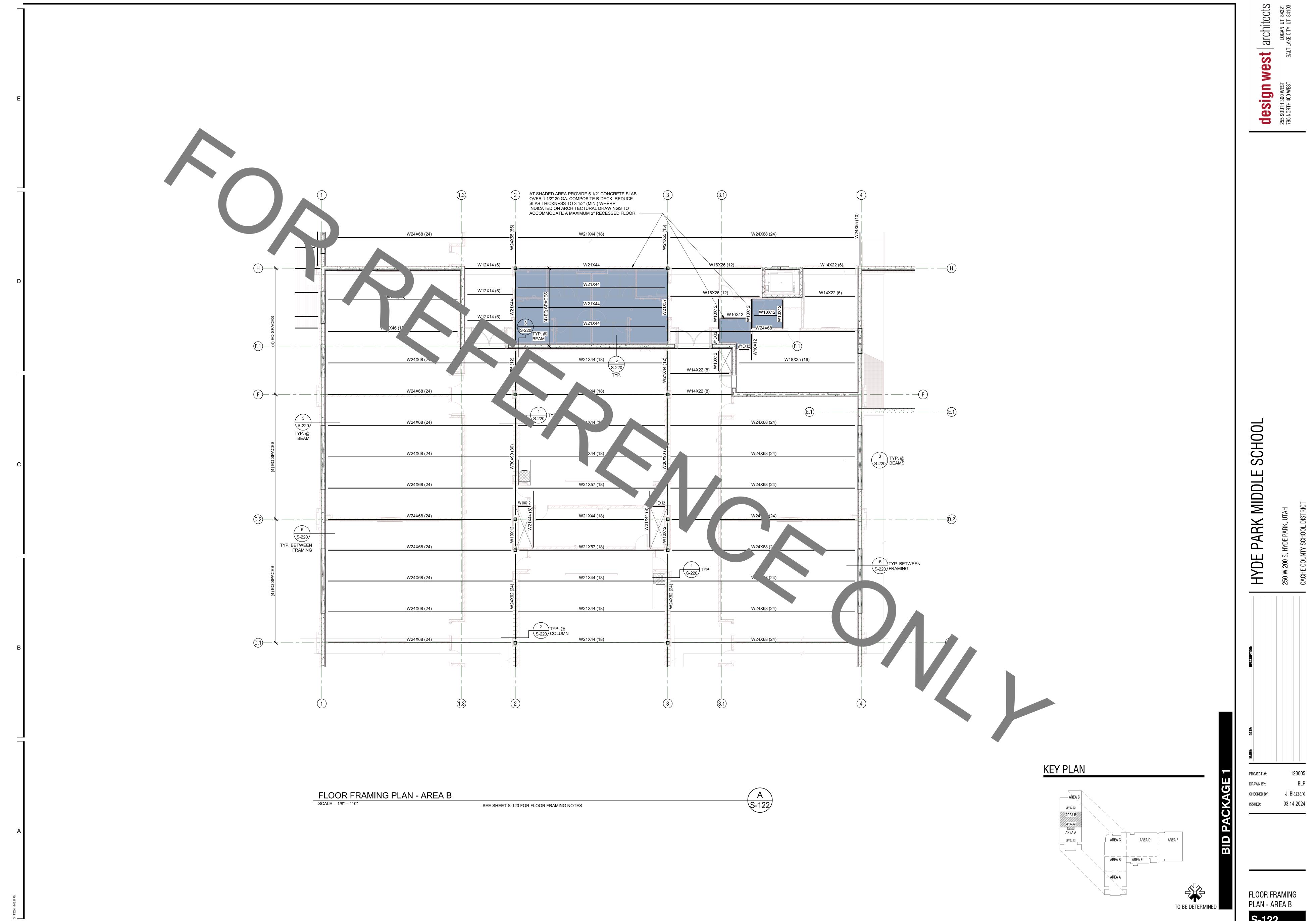


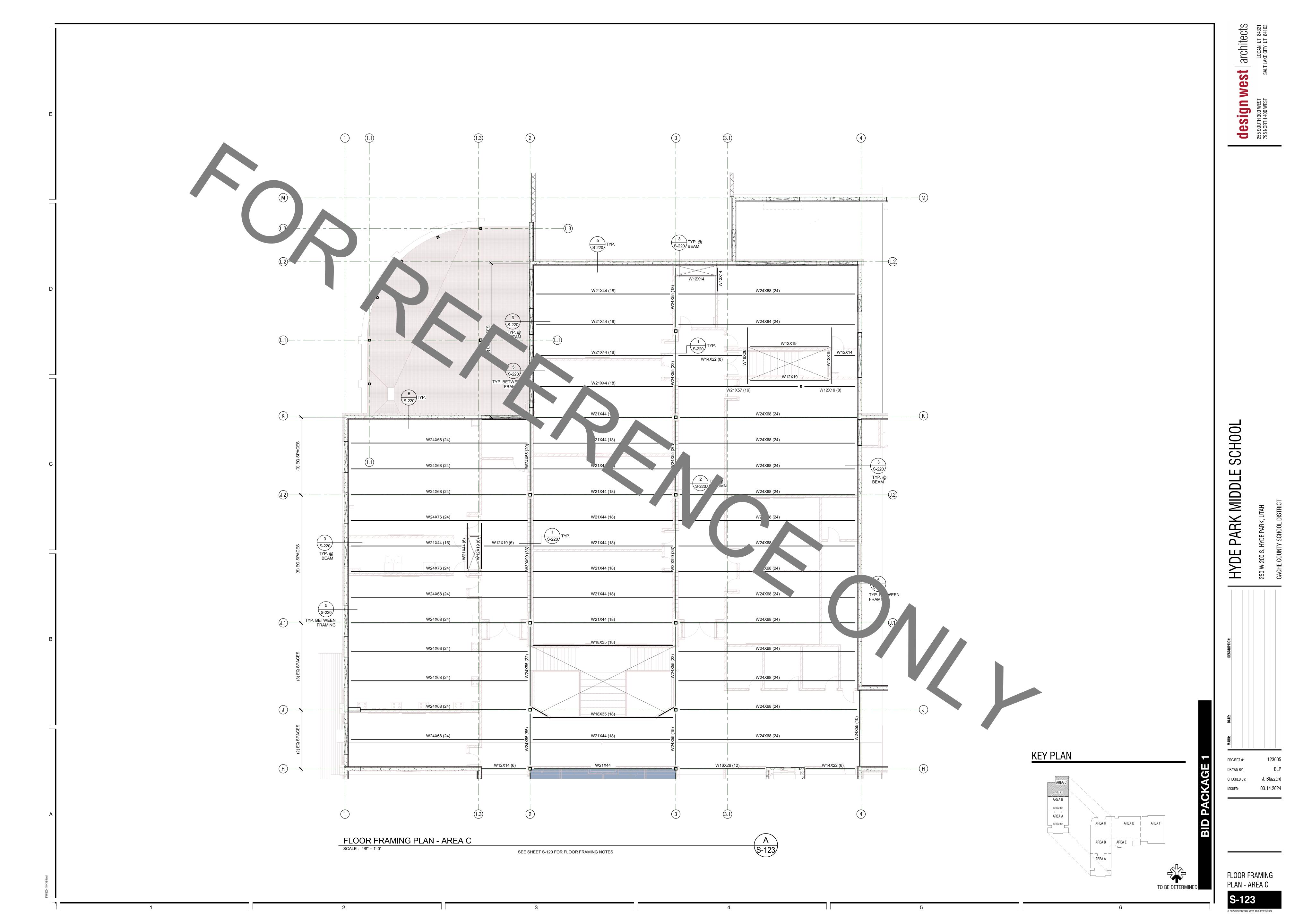


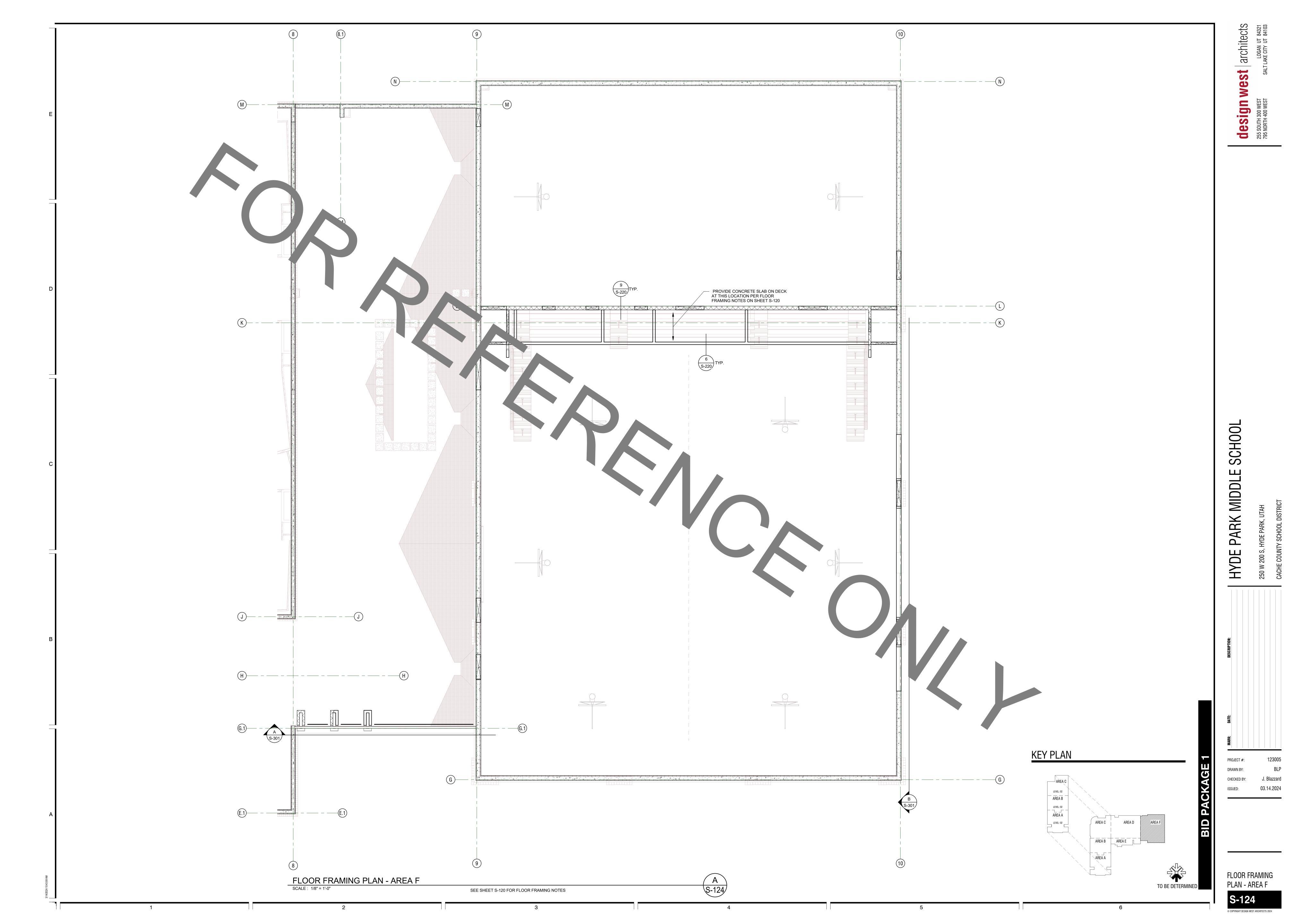


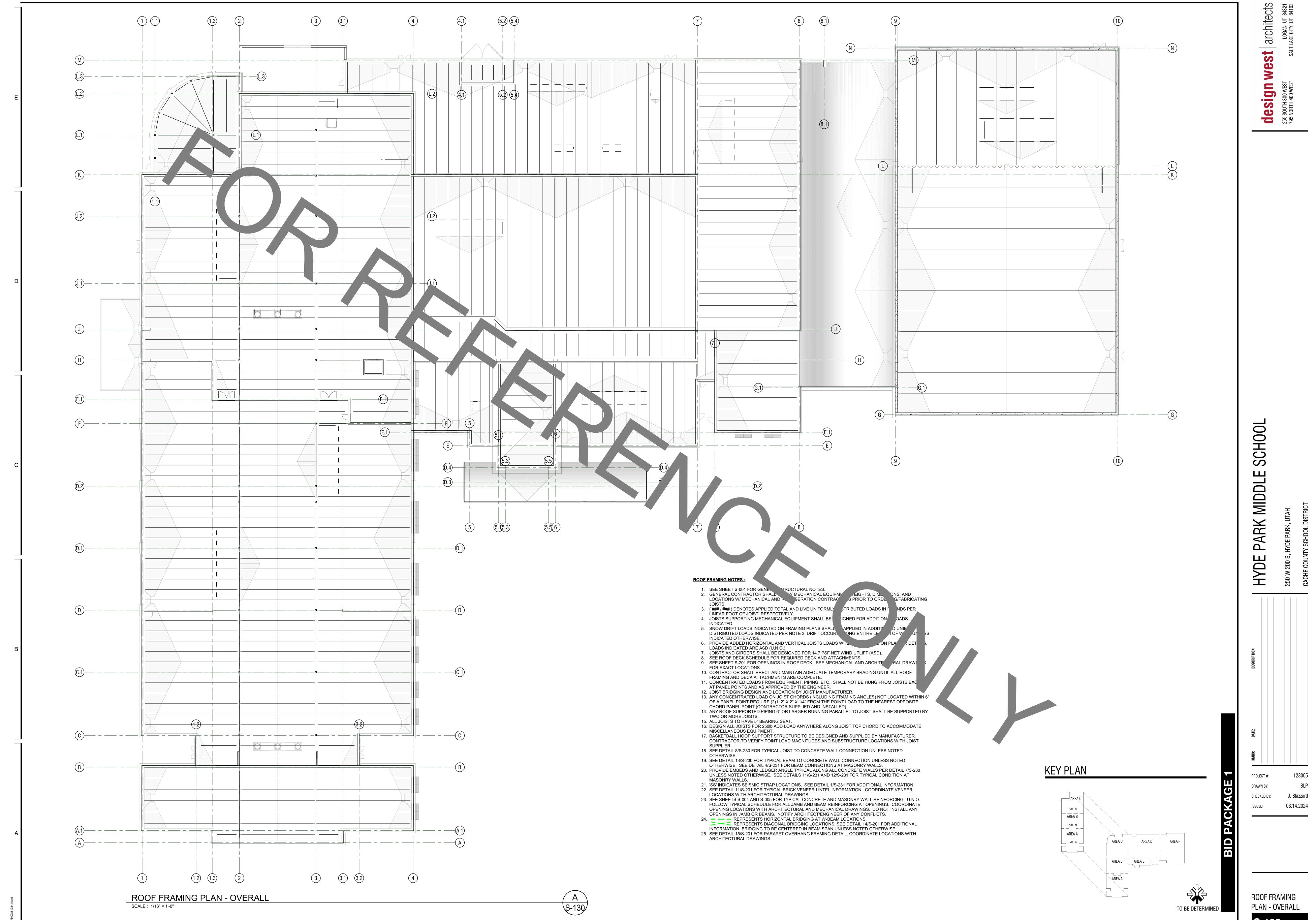




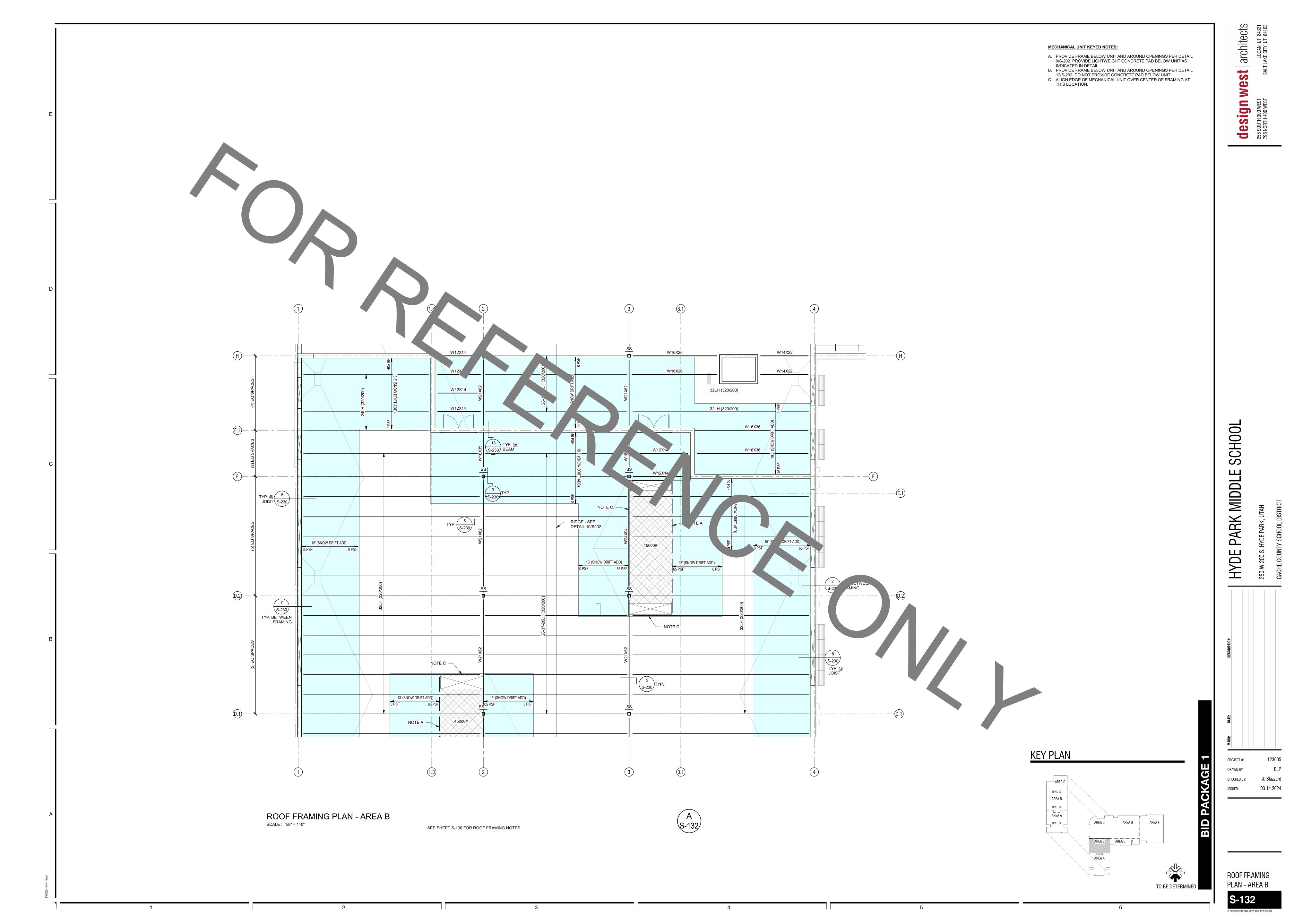




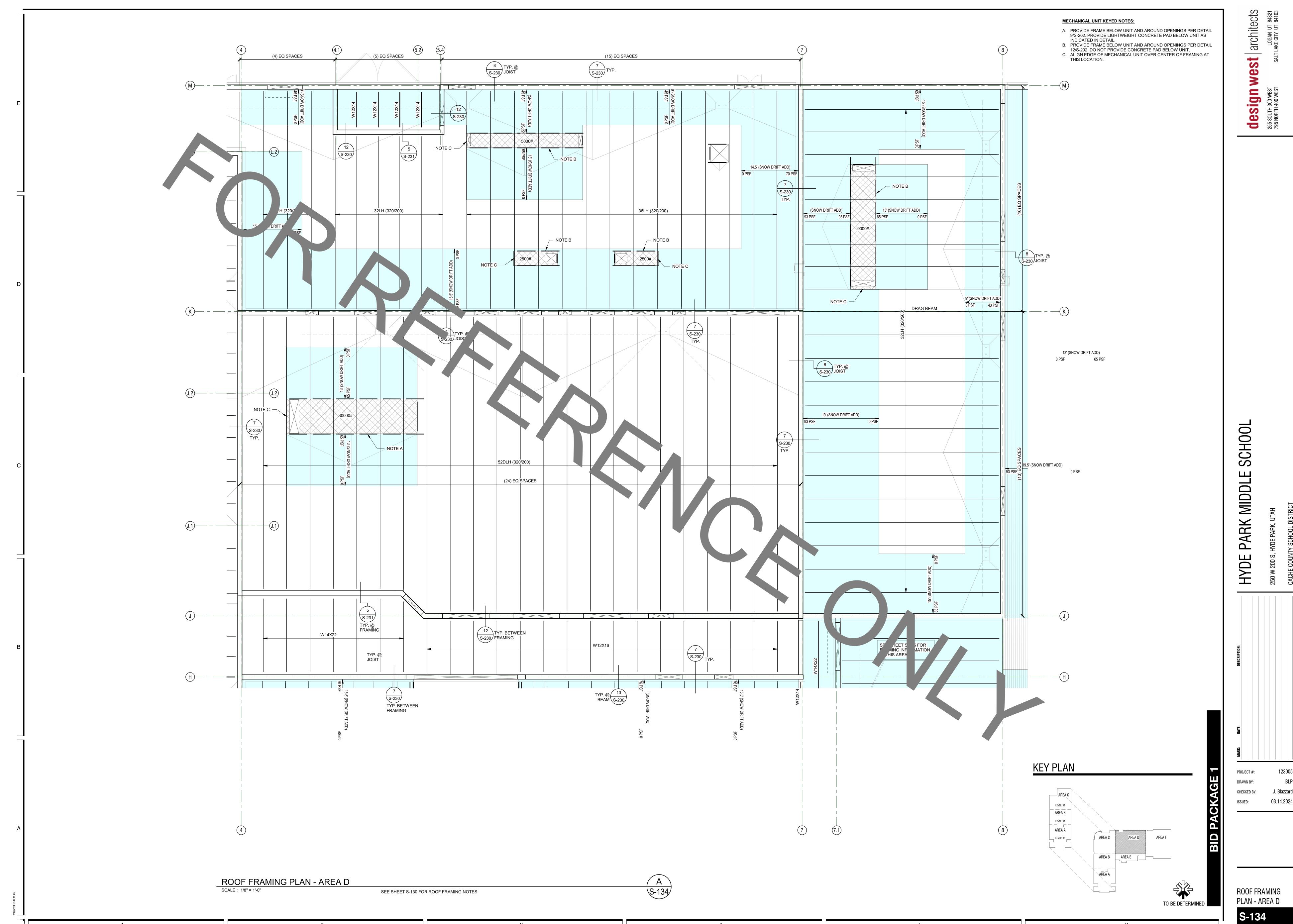


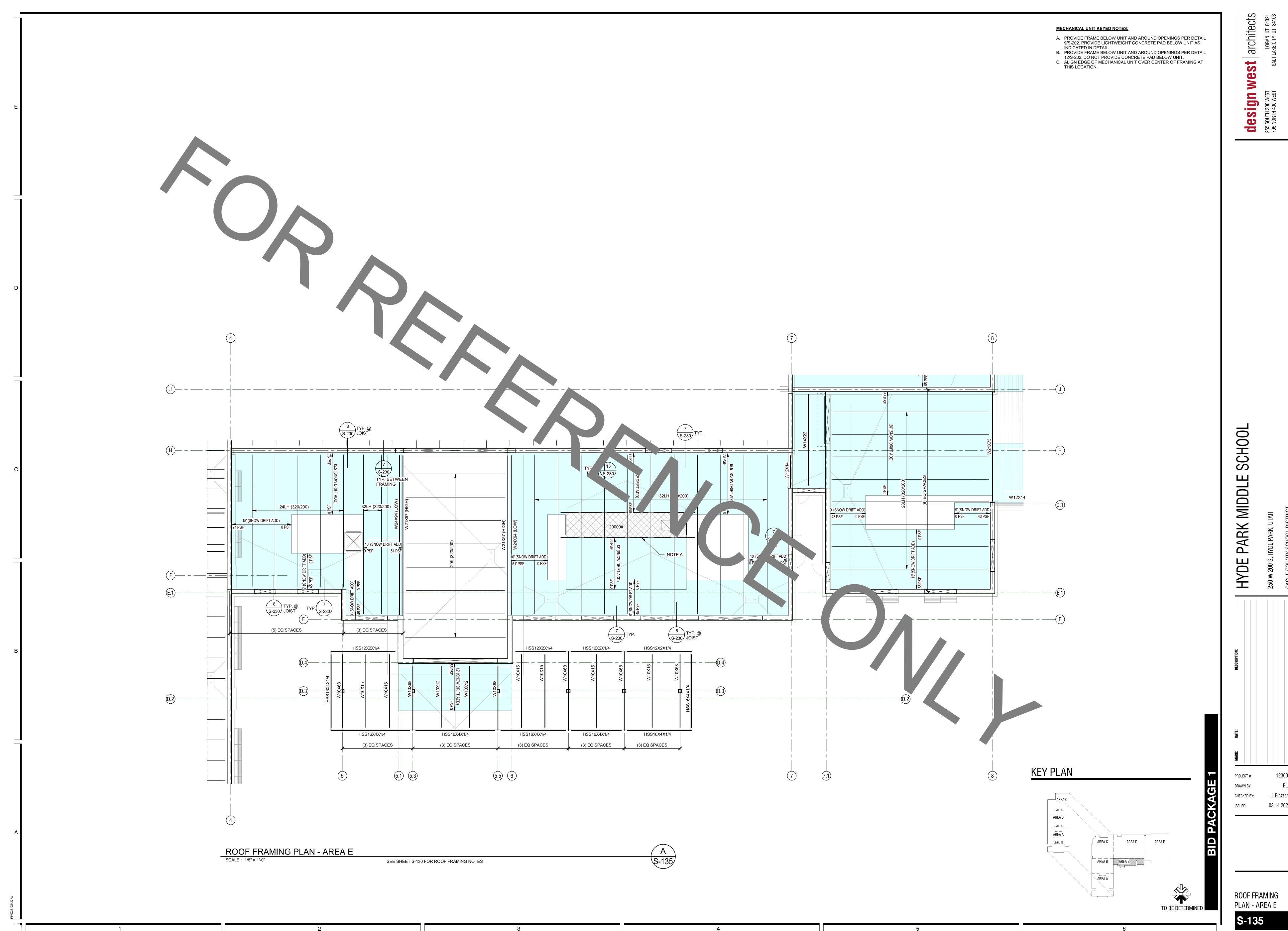




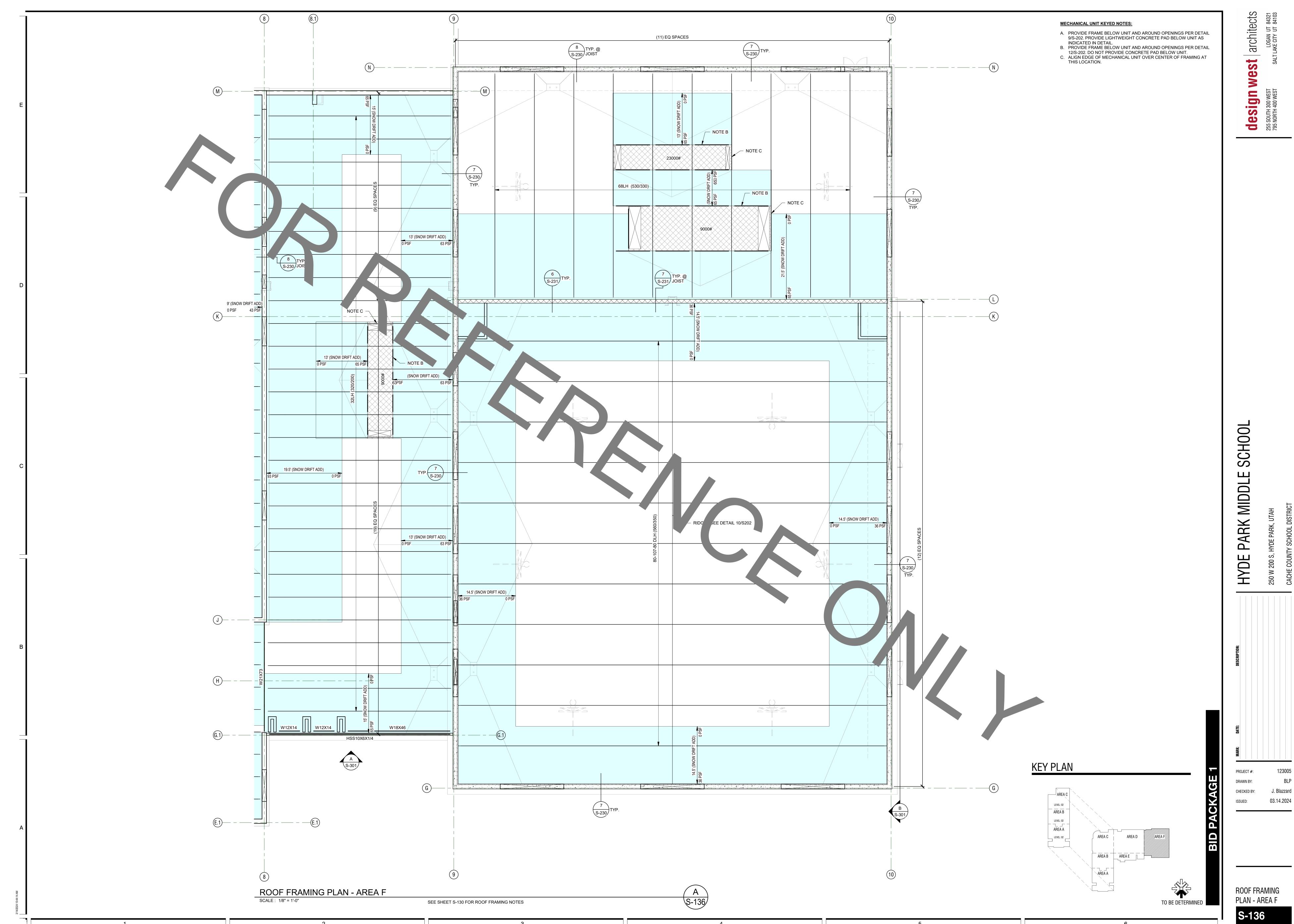








03.14.2024



1. SEE SHEET S-001 FOR GENERAL STRUCTURAL NOTES. 2. SEE SHEET S-110 FOR FOOTING AND FOUNDATION NOTES.

## **WOOD ROOF NOTES**

- 1. WOOD GRADES (UNLESS NOTED OTHERWISE) a. ALL FRAMING LUMBER SHALL BE DOUGLAS FIR/LARCH CLEARLY MARKED WITH A STAMP BY WWPA APPROVED AGENCY AND SHALL BE GRADED AS FOLLOWS: 1. HORIZONTAL MEMBERS: JOISTS & RAFTERS: NO. 2, BEAMS & STRINGERS: NO. 2. b. ALL WOOD "I" JOISTS AND BRIDGING SHALL BE FURNISHED BY TRUS-JOIST
- CORPORATION OR APPROVED EQUAL. 2. SHEATHING SHALL BE APA RATED SHEATHING, EXPOSURE I, EXTERIOR GLUE AND PANEL INDEX RATING AS NOTED BELOW UNLESS NOTED OTHERWISE: LOCATION THICKNESS PANEL INDEX
- ROOFS: 19/32" 32/16 3. INDIVIDUAL PIECES OF SHEATHING AT ROOF SHALL NOT BE SMALLER THAN 24" IN EITHER DIRECTION AND SHALL SPAN A MINIMUM OF TWO FRAMING SPACES, UNO.
- 4. CONNECTIONS, FASTENERS, AND ADHESIVE a. ALL BOLTS THRU WOOD SHALL BE ASTM A307 AND SHALL HAVE HARDENED WASHERS UNDER ASTM A563 HEAVY HEX NUT AND BOLT HEADS.
- b. UNLESS NOTED OTHERWISE, 10d COMMON (0.148) NAILS SHALL BE USED TO FASTEN ALL PLYWOOD ROOF SHEATHING TO SUPPORTING TRUSSES, JOISTS, LEDGERS OR BLOCKING AS FOLLOWS: 1. BOUNDARY NAILING "BN": 4"O.C. AT ALL BEARING WALLS, SHEAR WALLS,
- BLOCKING, AND WHERE OTHERWISE INDICATED IN THE STRUCTURAL DRAWINGS. 2. PANEL EDGE NAILING "EN": 6"O.C. AT ALL OTHER PLYWOOD PANEL EDGES. 3. PANEL FIELD NAILING "FN": 12"O.C. AT INTERIOR SUPPORTS IN FIELD OF PANEL. EXCEPT WHERE NOTED OTHERWISE, THE NUMBER AND SIZE OF NAILS CONNECTING WOOD MEMBERS SHALL NOT BE LESS THAN THAT SET FORTH IN IBC <u>TABLE 2304.10.1</u>. CONNECTIONS FOR MULTIPLE PIECES OF ENGINEERED LUMBER PIECES SHALL BE IN ACCORDANCE WITH THE MANUFACTURERS
- SPECIFICATIONS. c. UNLESS NOTED OTHERWISE, ALL NAILS SHALL HAVE THE FOLLOWING MINIMUM PROPERTIES: COMMON SHANK HEAD LENGTH MIN. PENETRATION NAIL SIZE DIAMETER DIAMETER INTO SUPPORT MEMBER 0.113" 0.266" 1.25" 0.131" 0.281" 2-1/2" 1.375"
- 0.162" 0.344" d. ALL FRAMING ANCHORS, POST CAPS, HOLD DOWNS, COLUMN BASES ETC. TO BE PROVIDED BY SIMPSON OR APPROVED EQUAL AND SHALL BE ATTACHED IN ACCORDANCE WITH MANUFACTURER'S PUBLISHED DATA, UNLESS NOTED

0.312"

0.312"

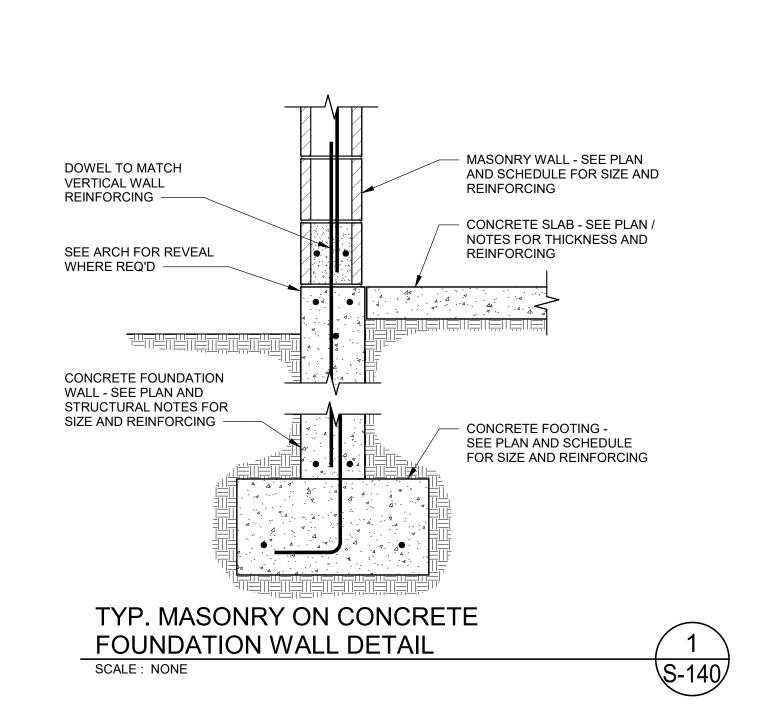
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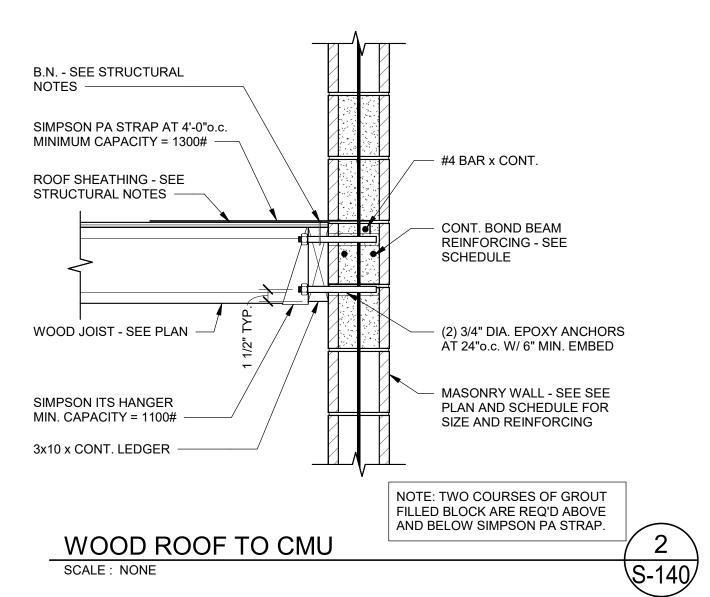
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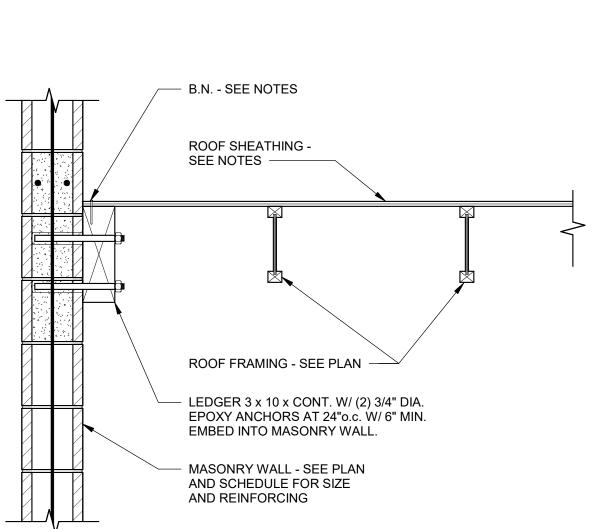
OTHERWISE. e. FASTENERS CONNECTED TO OR IN CONTACT WITH PRESERVATIVE-TREATED AND/OR FIRE-RETARDANT-TREATED WOOD (EXCEPT FOR TIMBERSTRAND LSL TREATED LUMBER AND BORATE BASED TREATMENTS) SHALL BE OF G-185 HOT-DIP GALVANIZED STEEL OR 304 OR 316 STAINLESS STEEL. STAINLESS STEEL AND GALVANIZED STEEL SHALL NEVER BE USED IN CONTACT WITH EACH OTHER.

3-1/4"

1.50"







WOOD ROOF TO MASONRY

S-140

SCALE: NONE

SPORTS STORAGE FOOTING, FDN, AND **ROOF FRAMING** 

S-140 TYP.

S-140

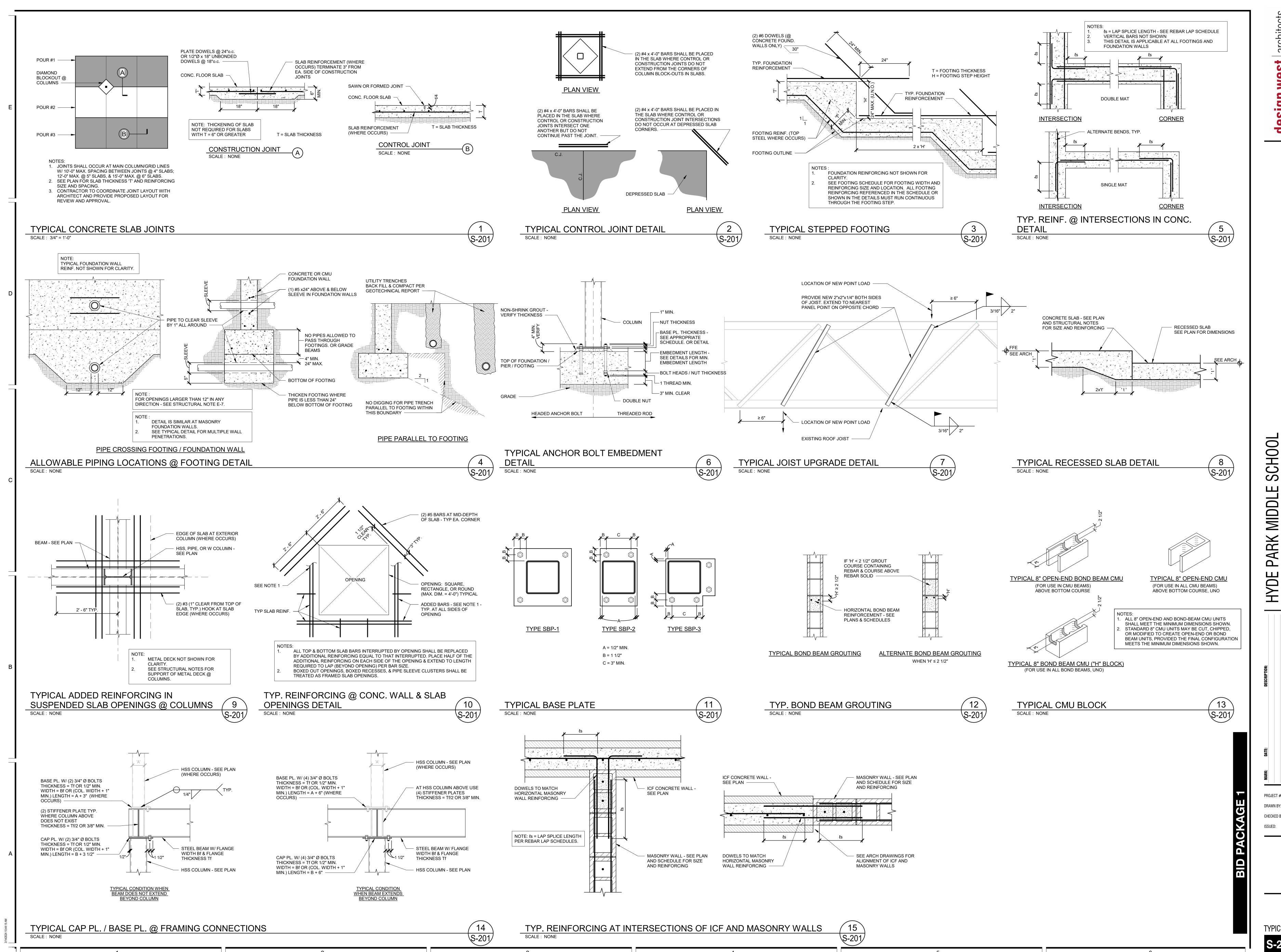
FOOTING AND FOUNDATION PLAN

ROOF FRAMING PLAN

SCALE: 1/4" = 1'-0"

OPENINGS

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MIDDLE HYD

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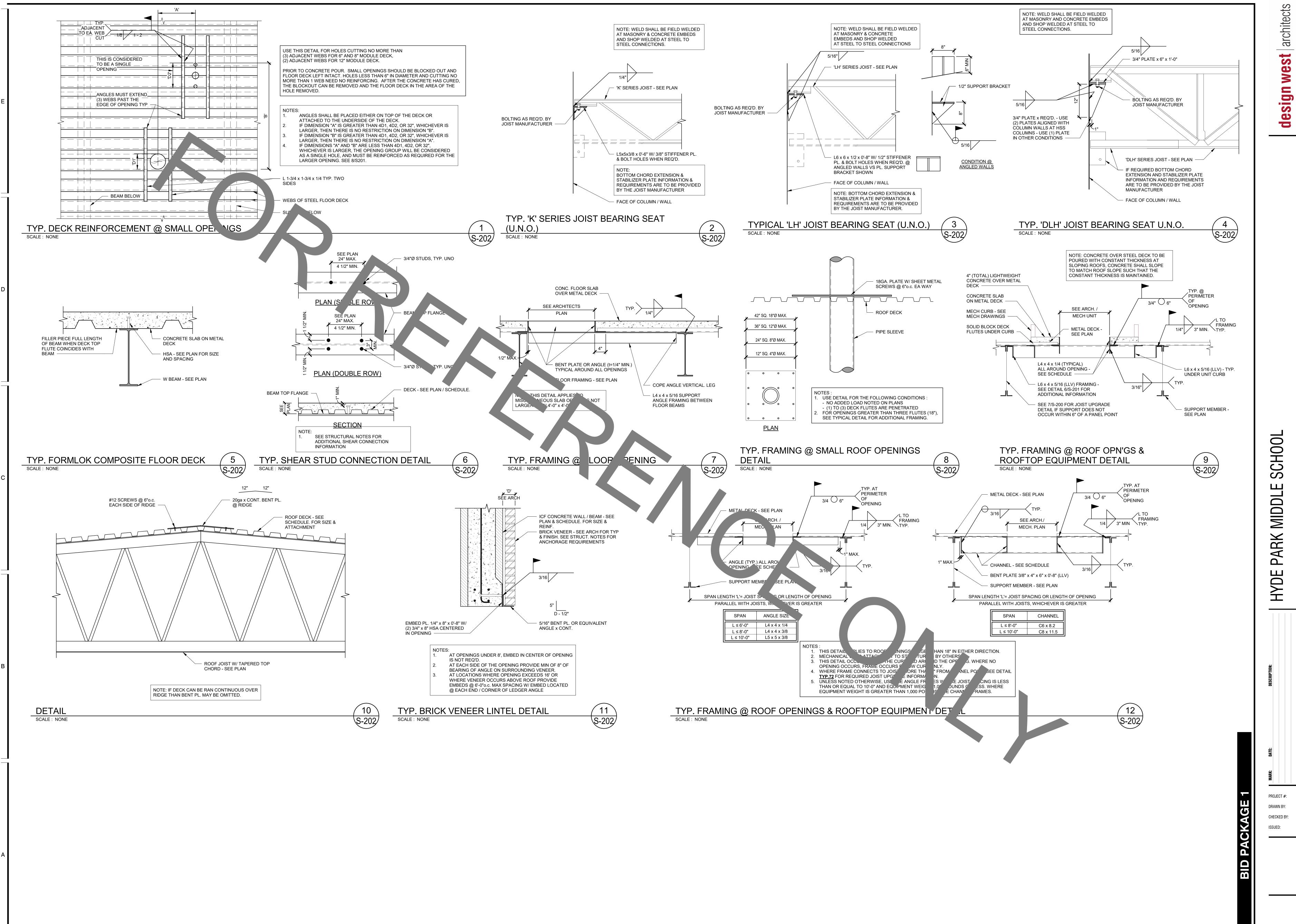
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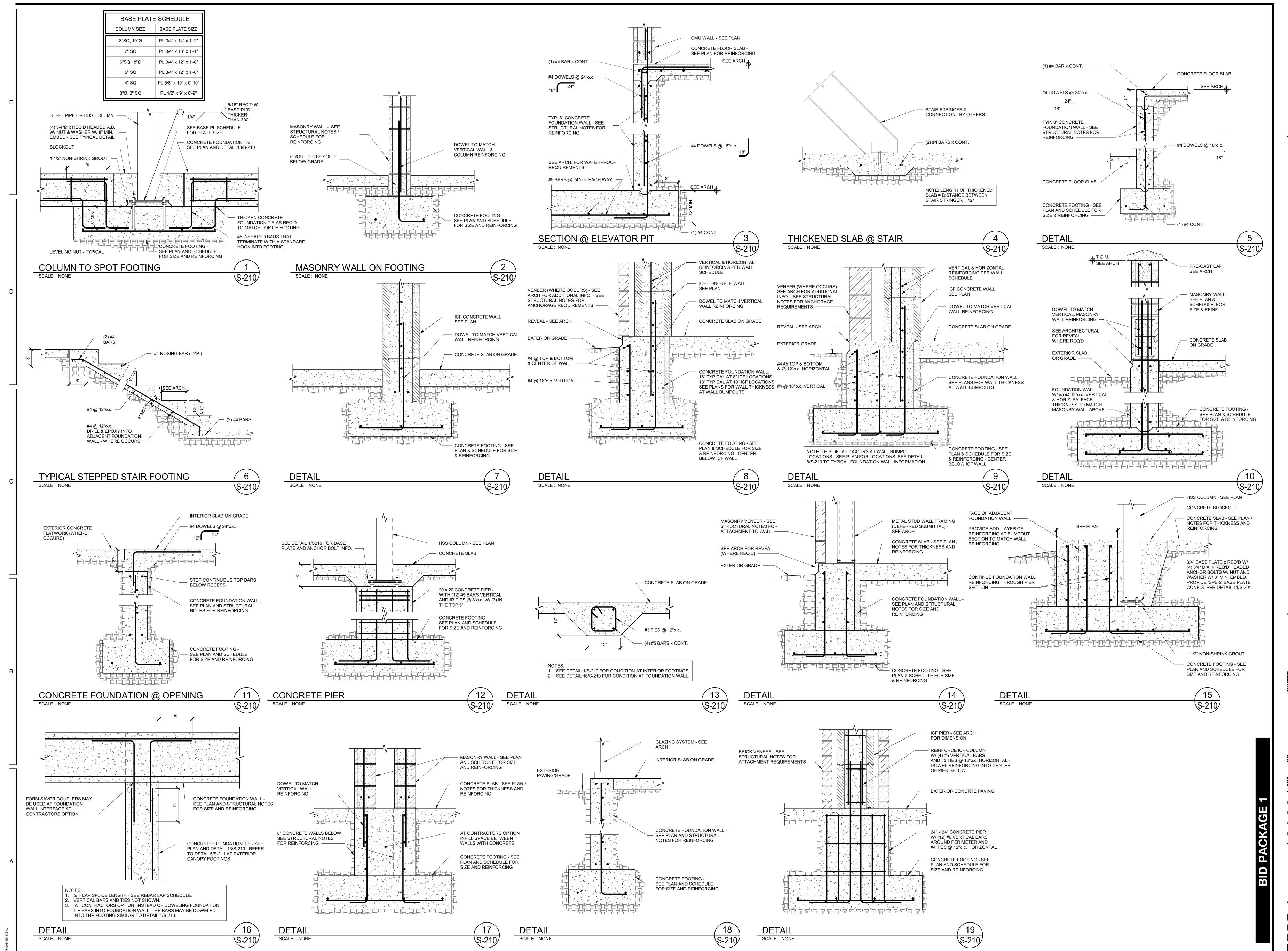
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TYPICAL DETAILS S-201

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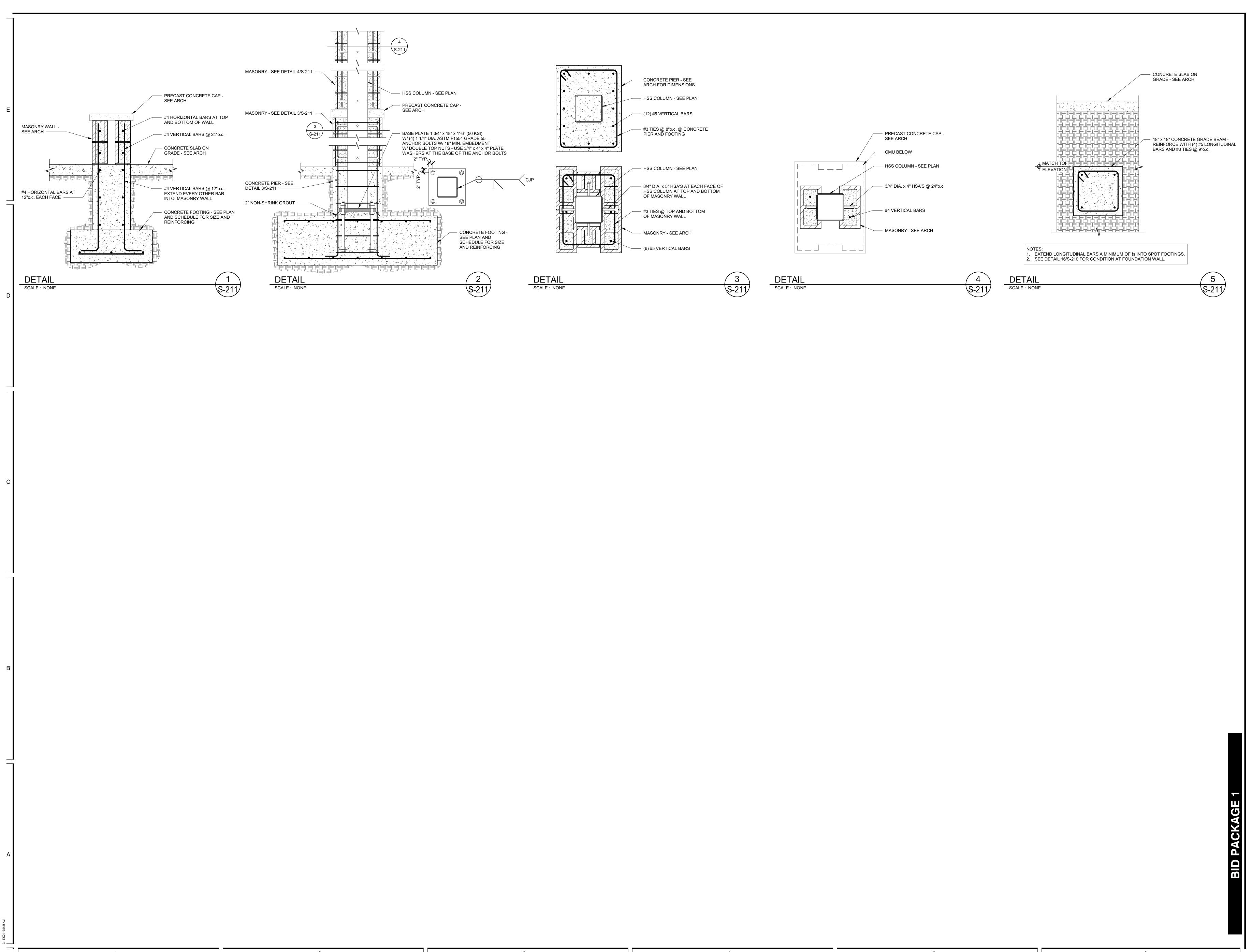
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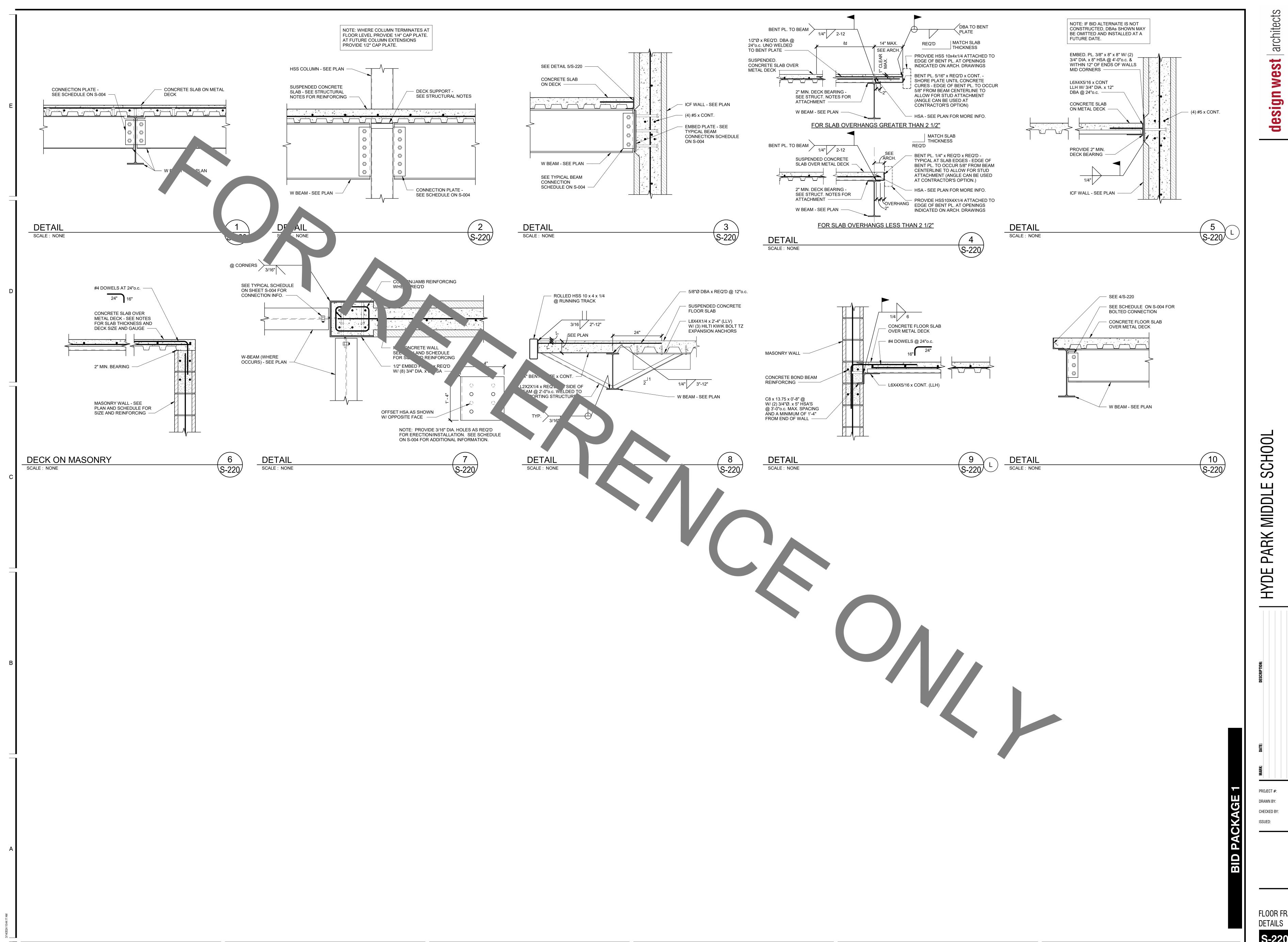
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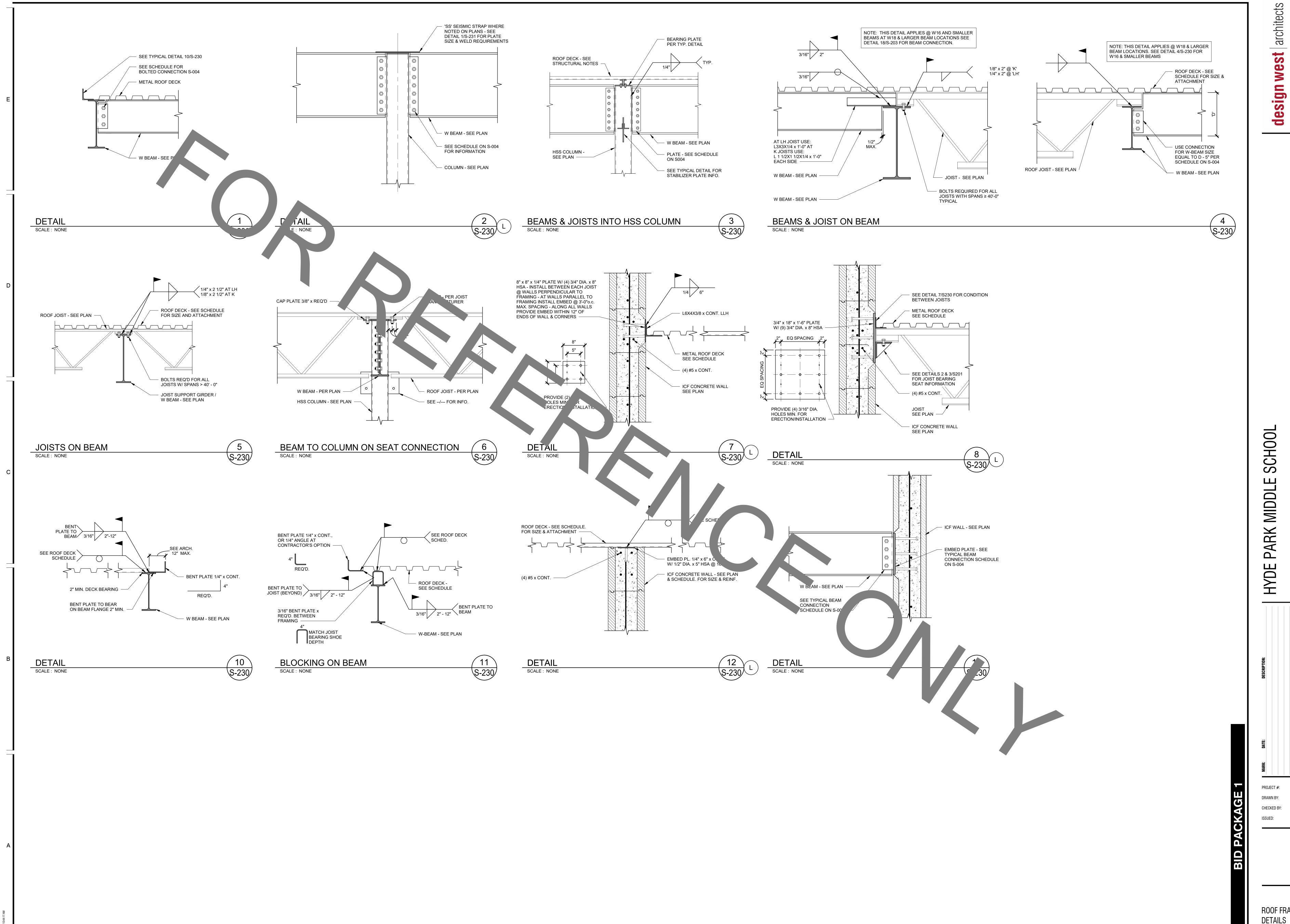
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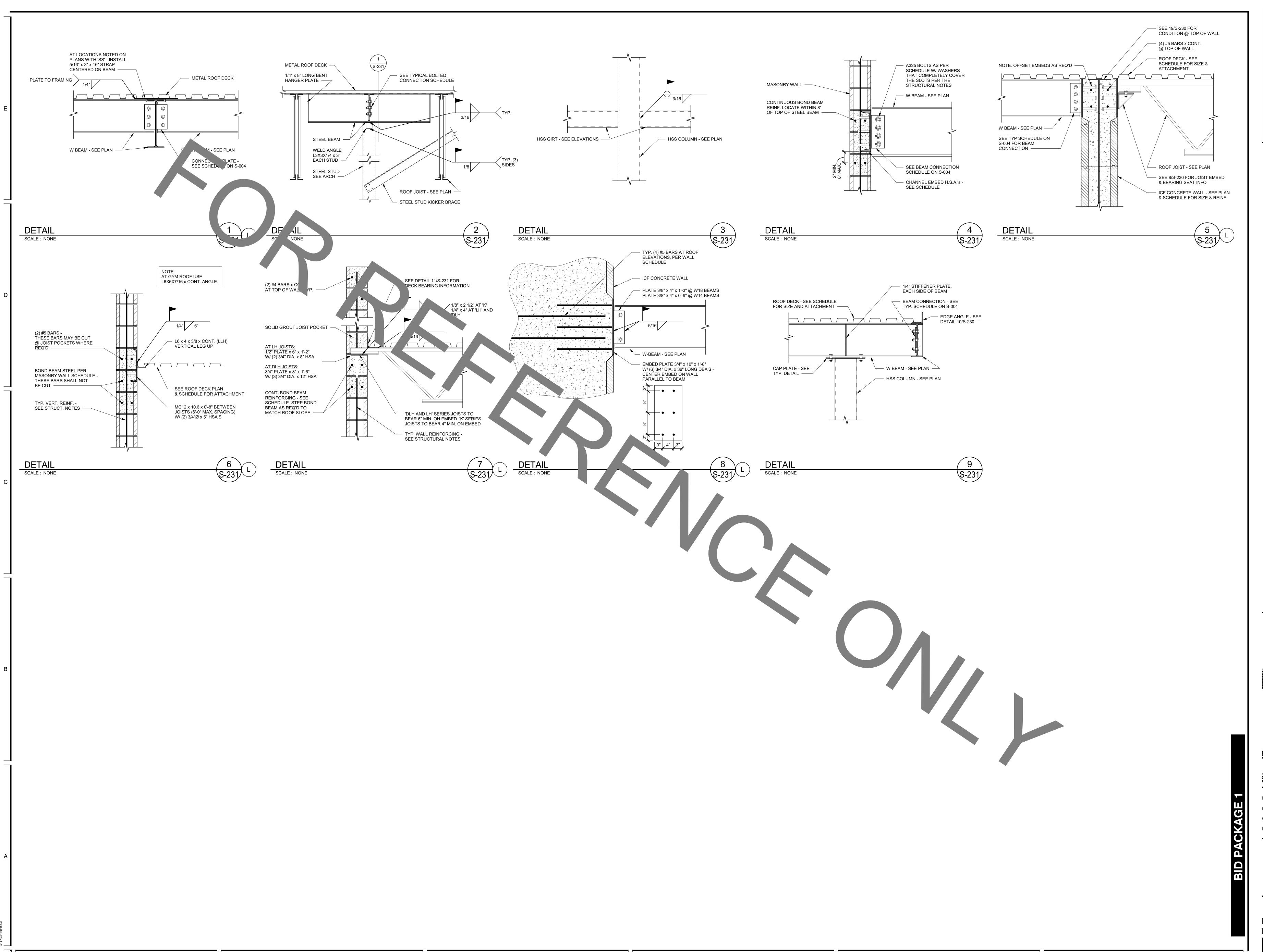
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CKED BY: J. Blazzard
JED: 03.14.2024

FOOTING &
FOUNDATION
DETAILS



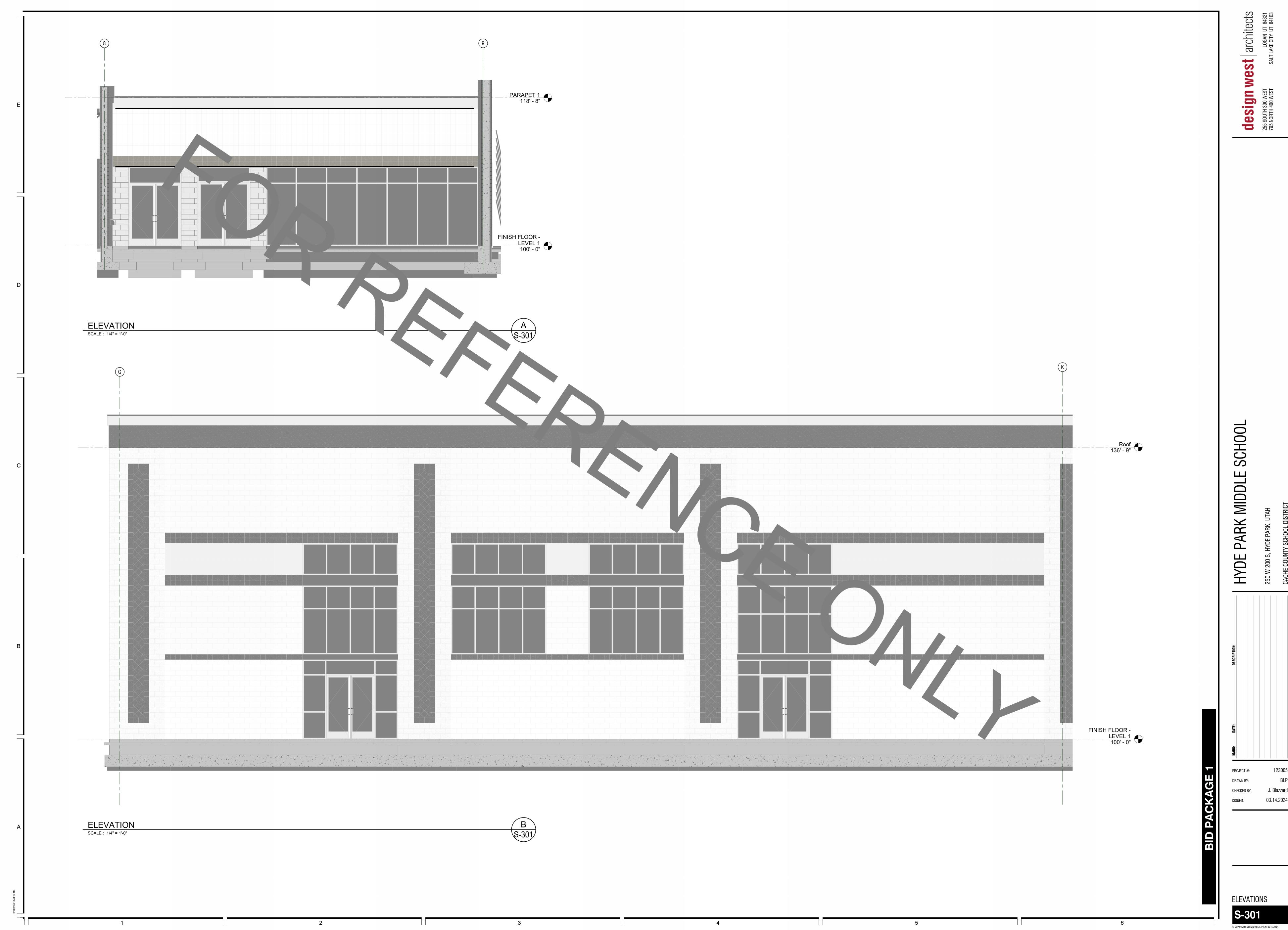


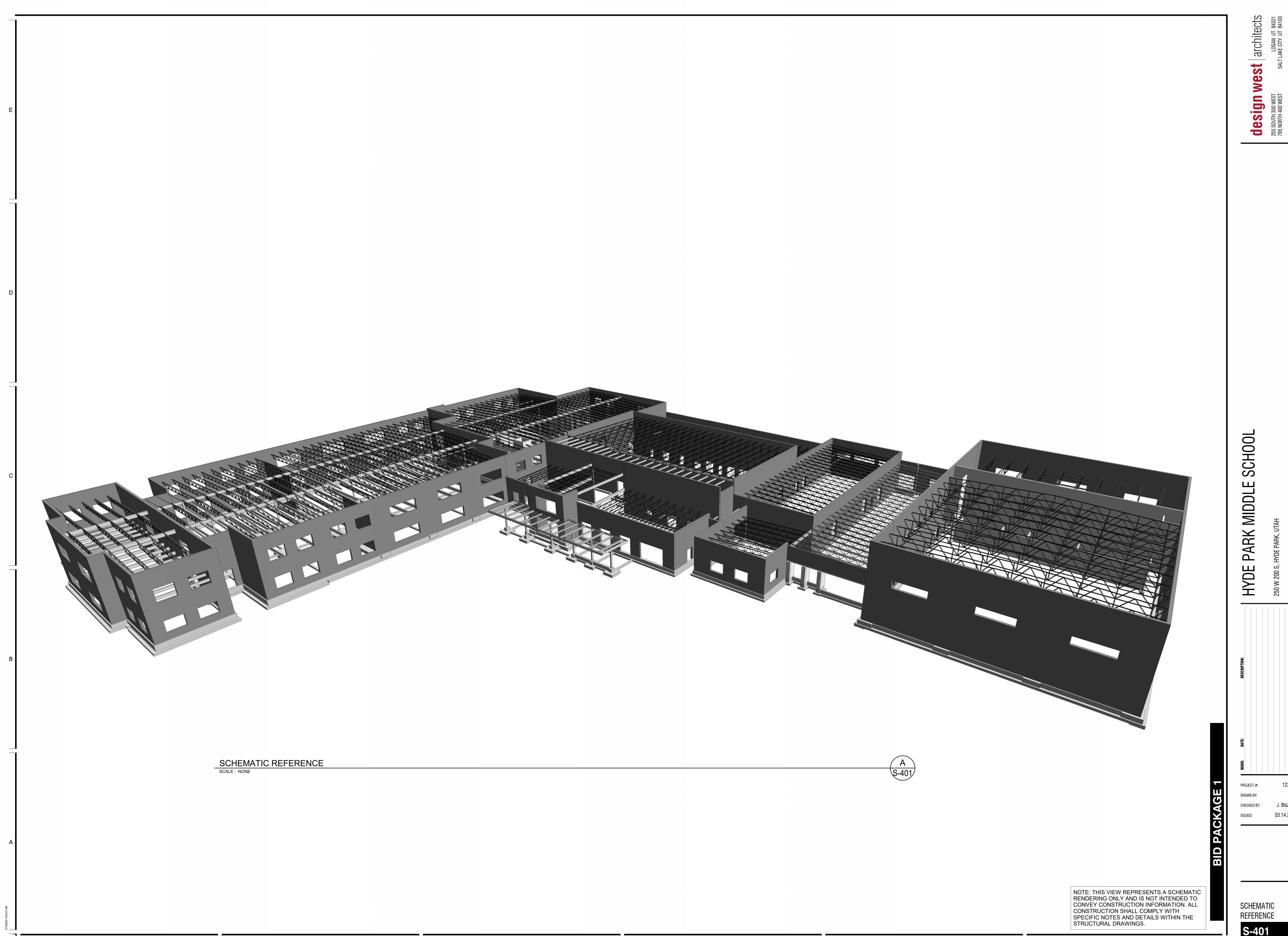
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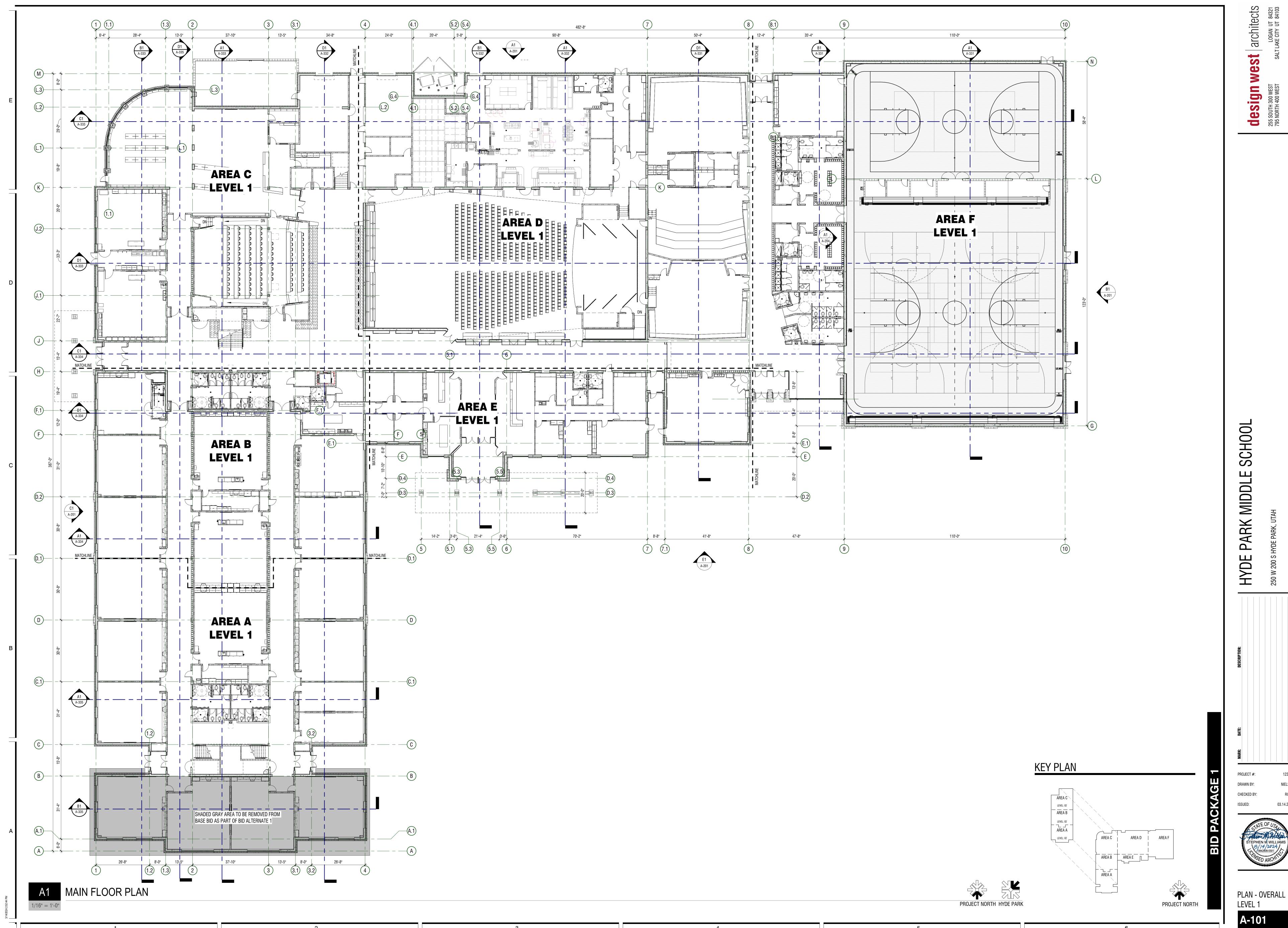
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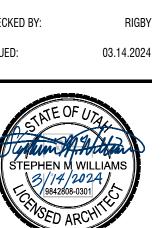




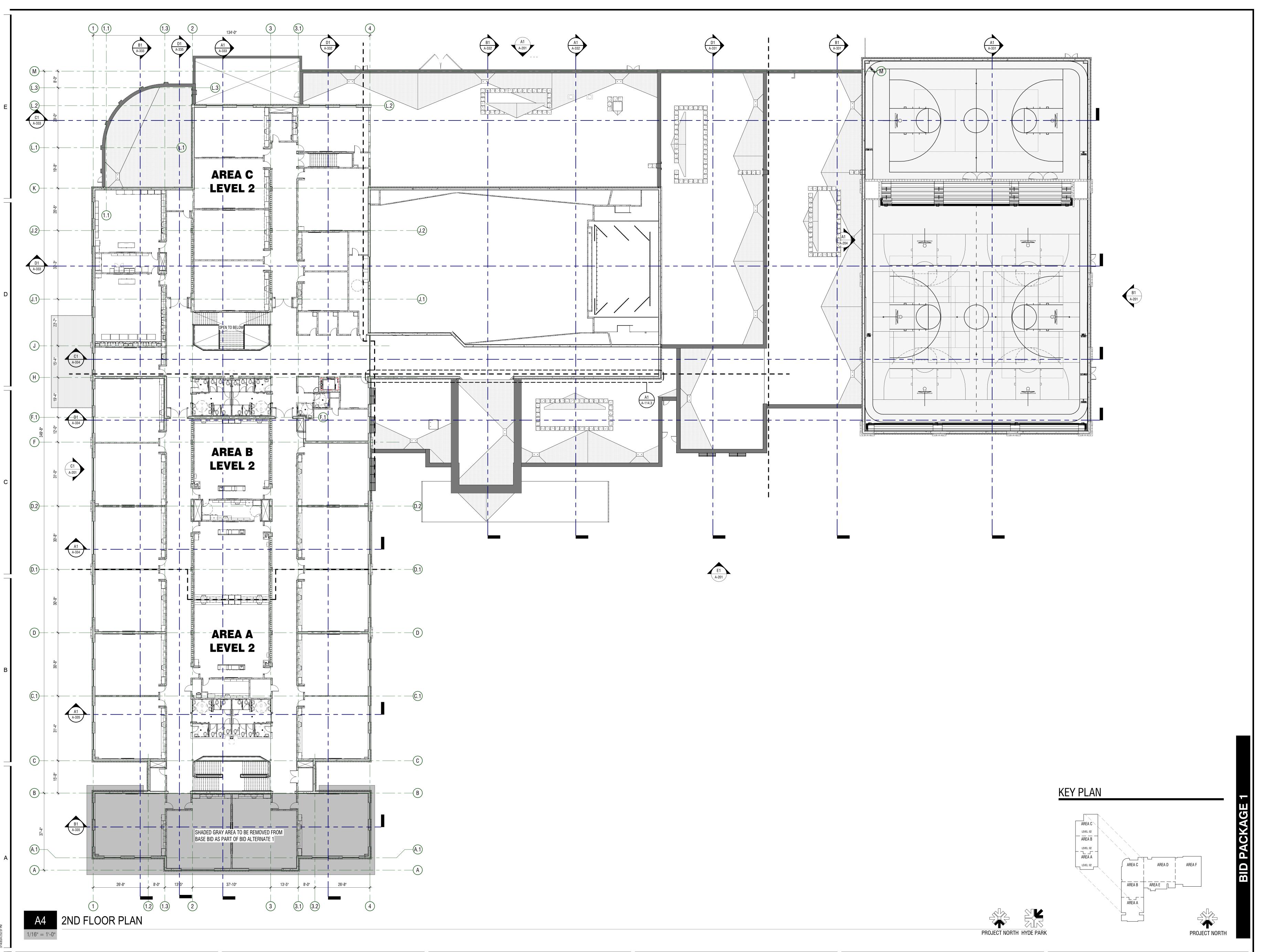
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PLAN - OVERALL LEVEL 1



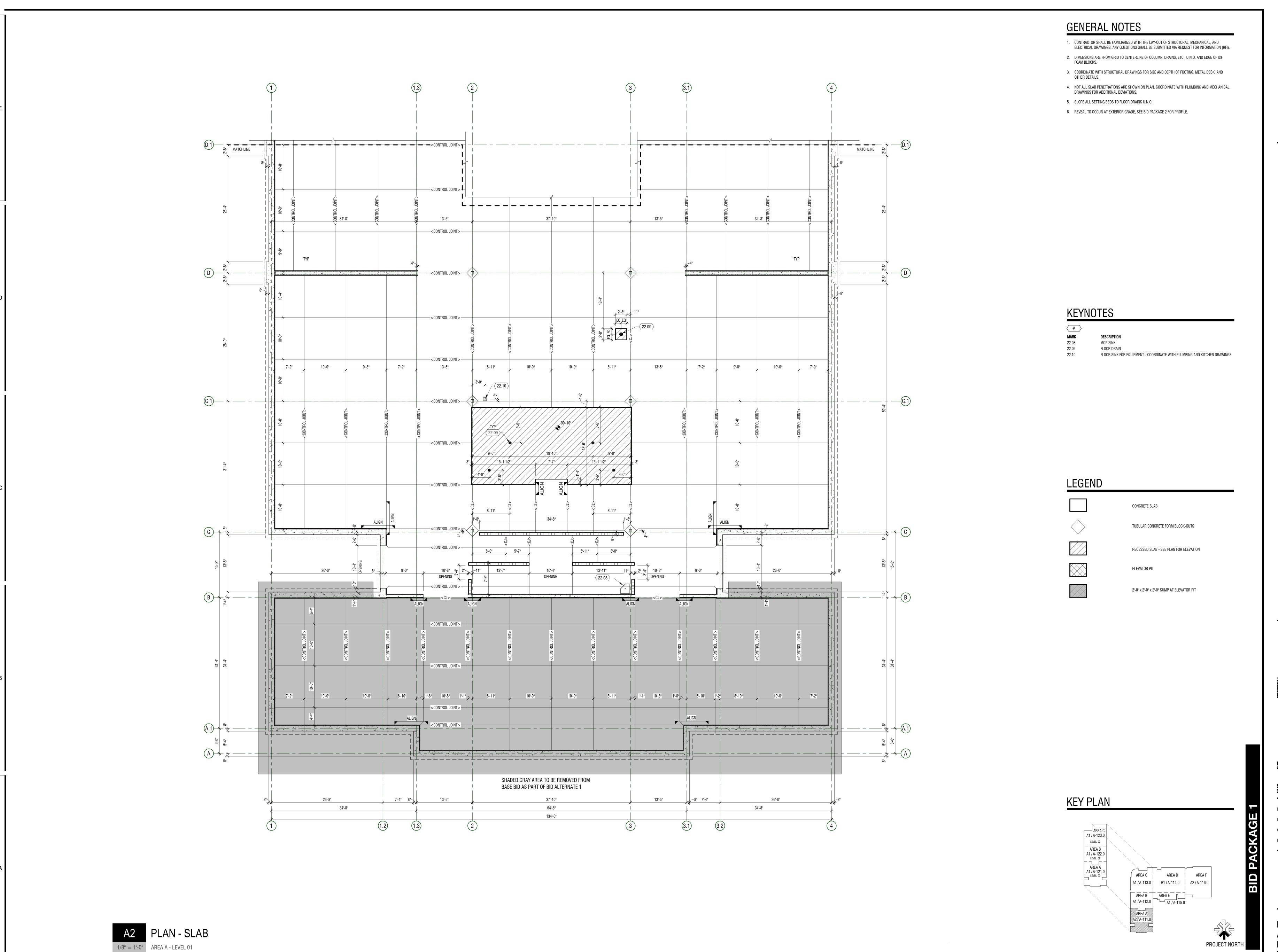
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255 SOUTH 300 WEST SALT LAKE CITY UT 84103

HYDE PARK MIDDLE SCH

PROJECT #: 12300
DRAWN BY: NIELSO
CHECKED BY: RIGE
SSUED: 03.14.202



PLAN - OVERALL LEVEL 2 A-102



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design west archite
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IIDDLE SCHOOL

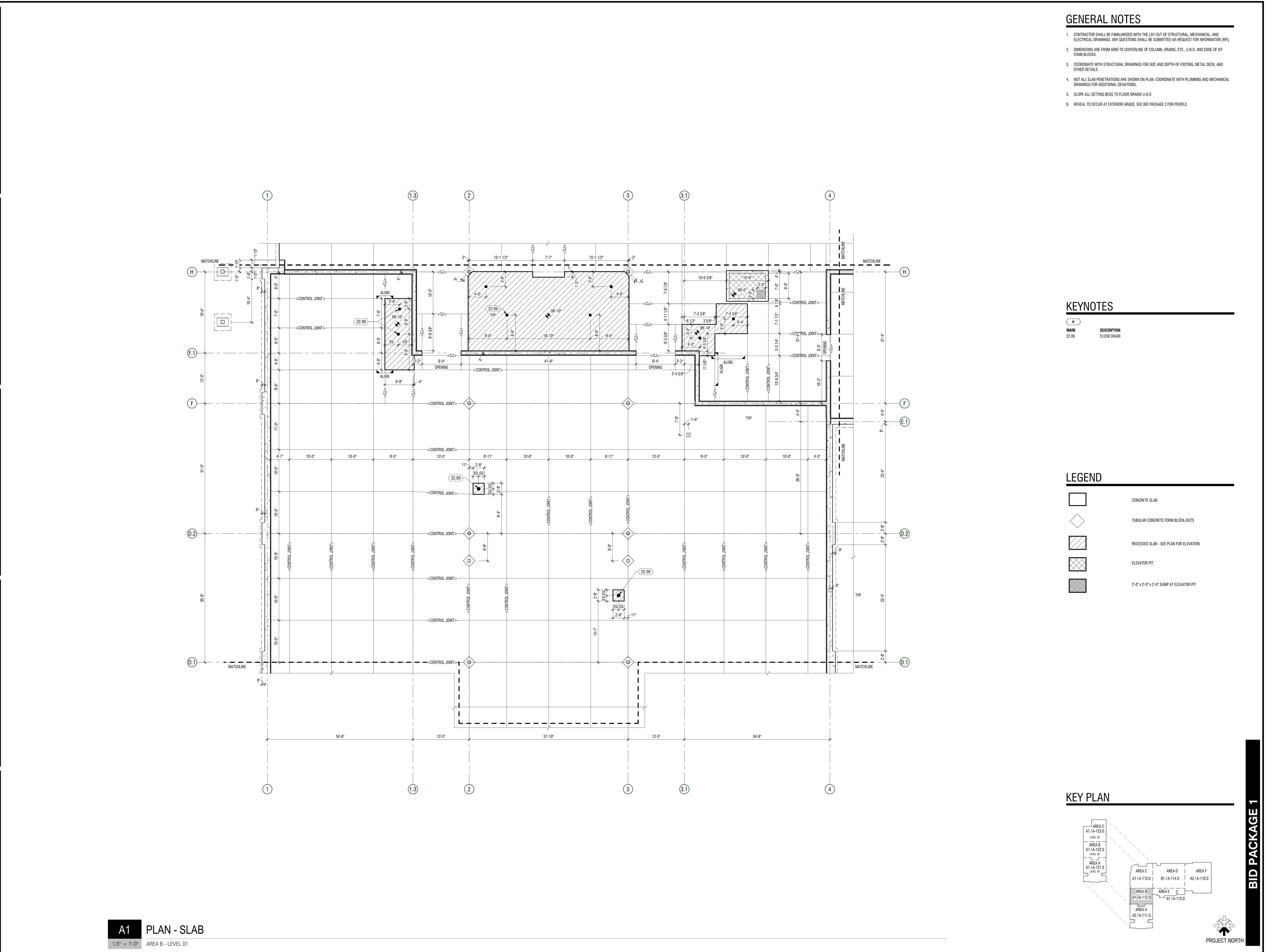
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PLAN - LEVEL 01 -AREA A - SLAB & FOUNDATION A-111.0



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SALT LAKE CITY UT 84103

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ARK MIDDLE SCHOOL

250 W 200 S HYDE PAR

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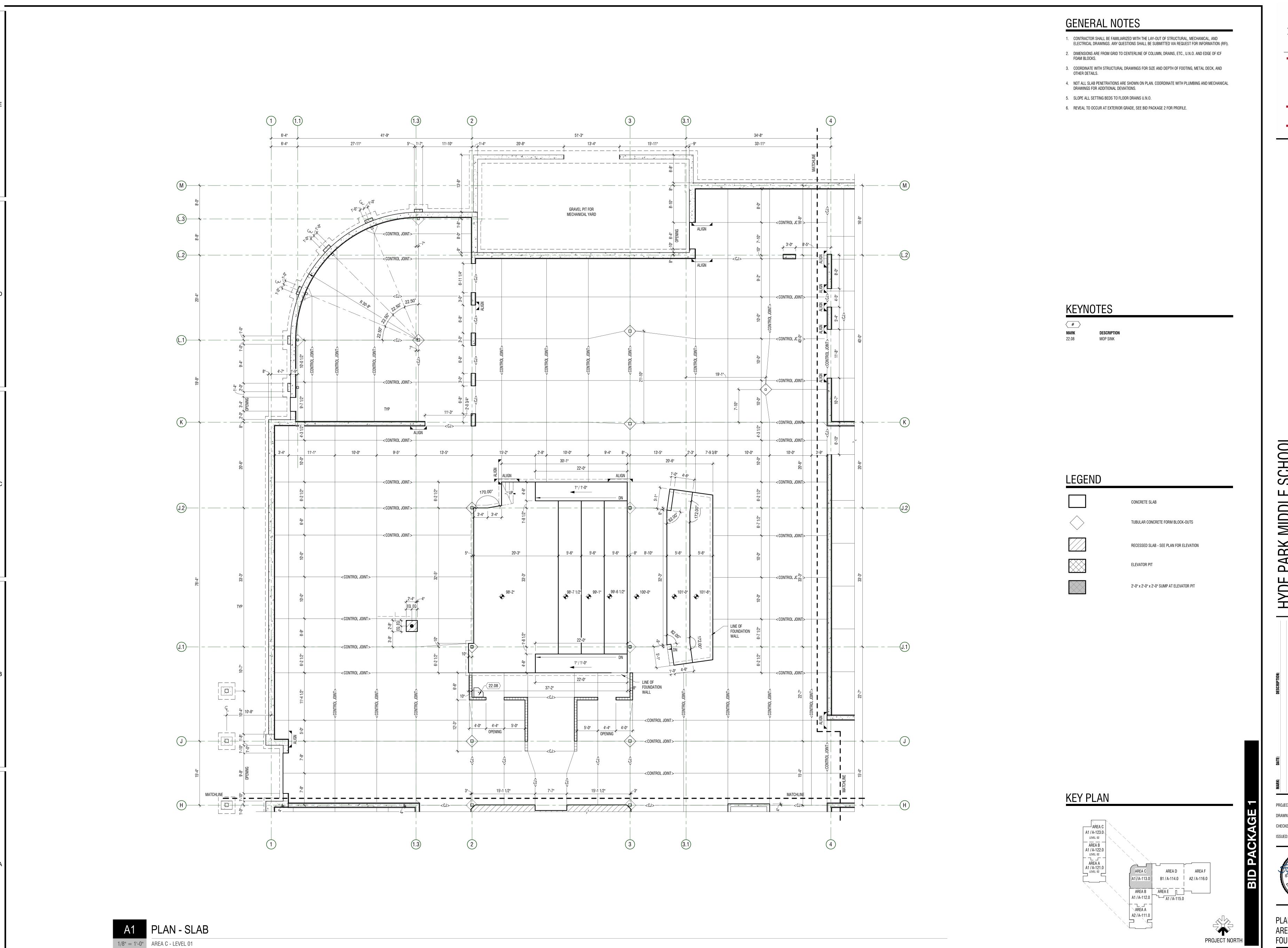
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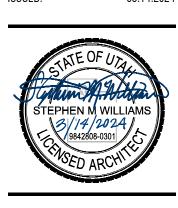
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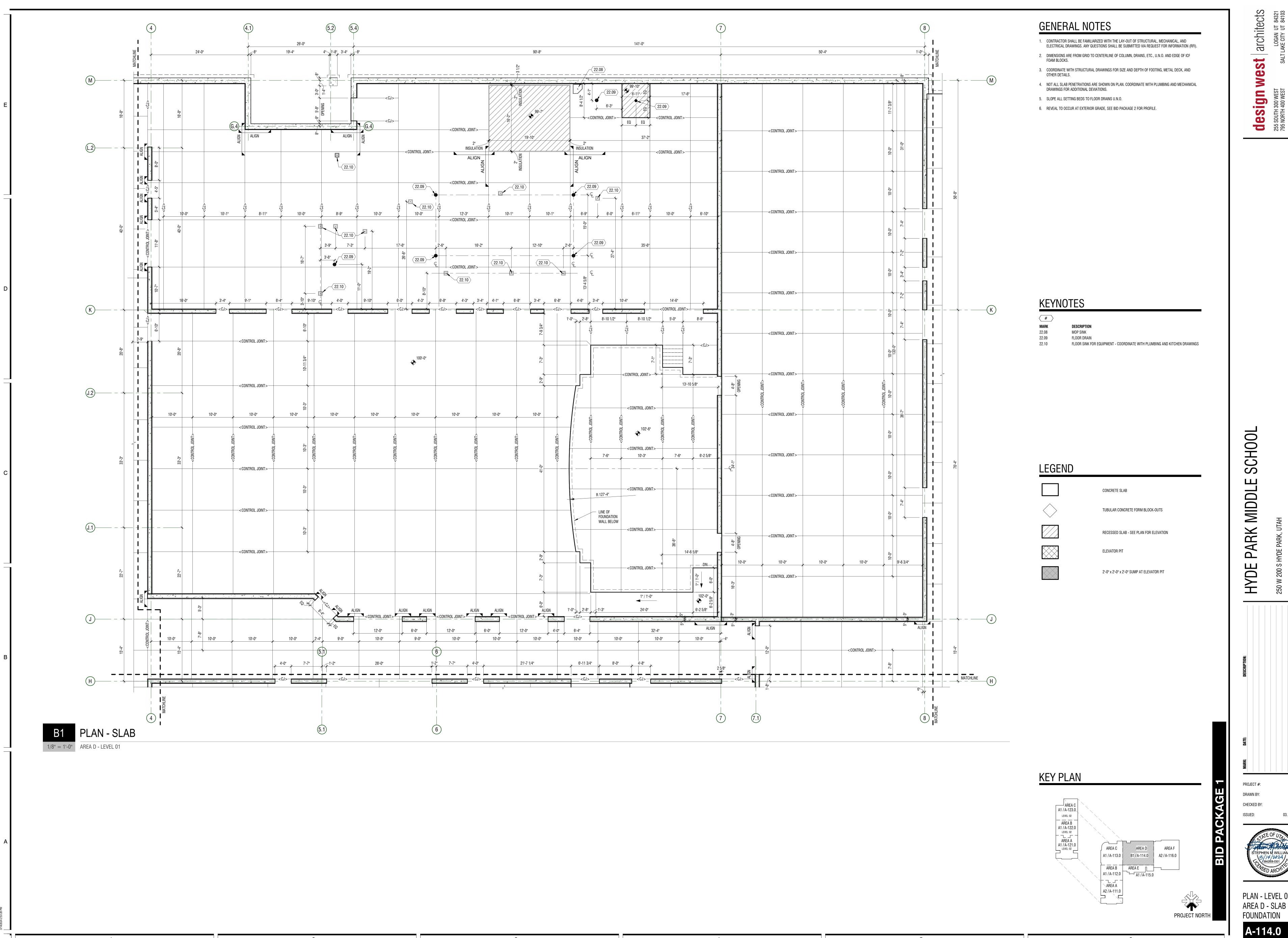
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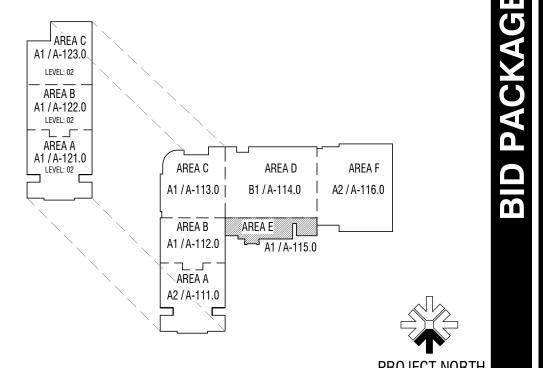


PLAN - LEVEL 01 -AREA C - SLAB & FOUNDATION A-113.0





PLAN - LEVEL 01 -AREA D - SLAB & FOUNDATION



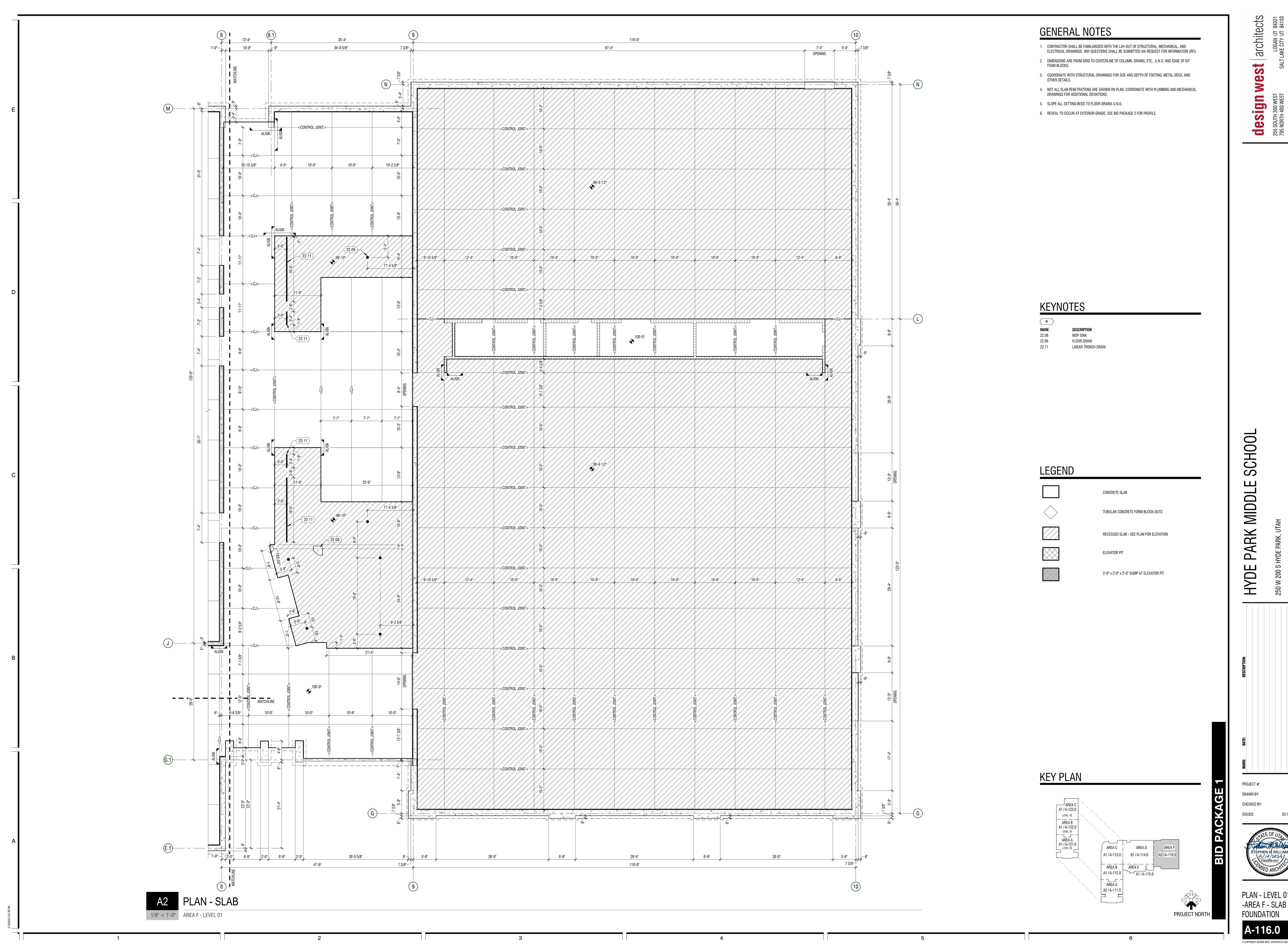
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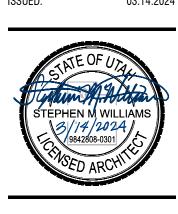
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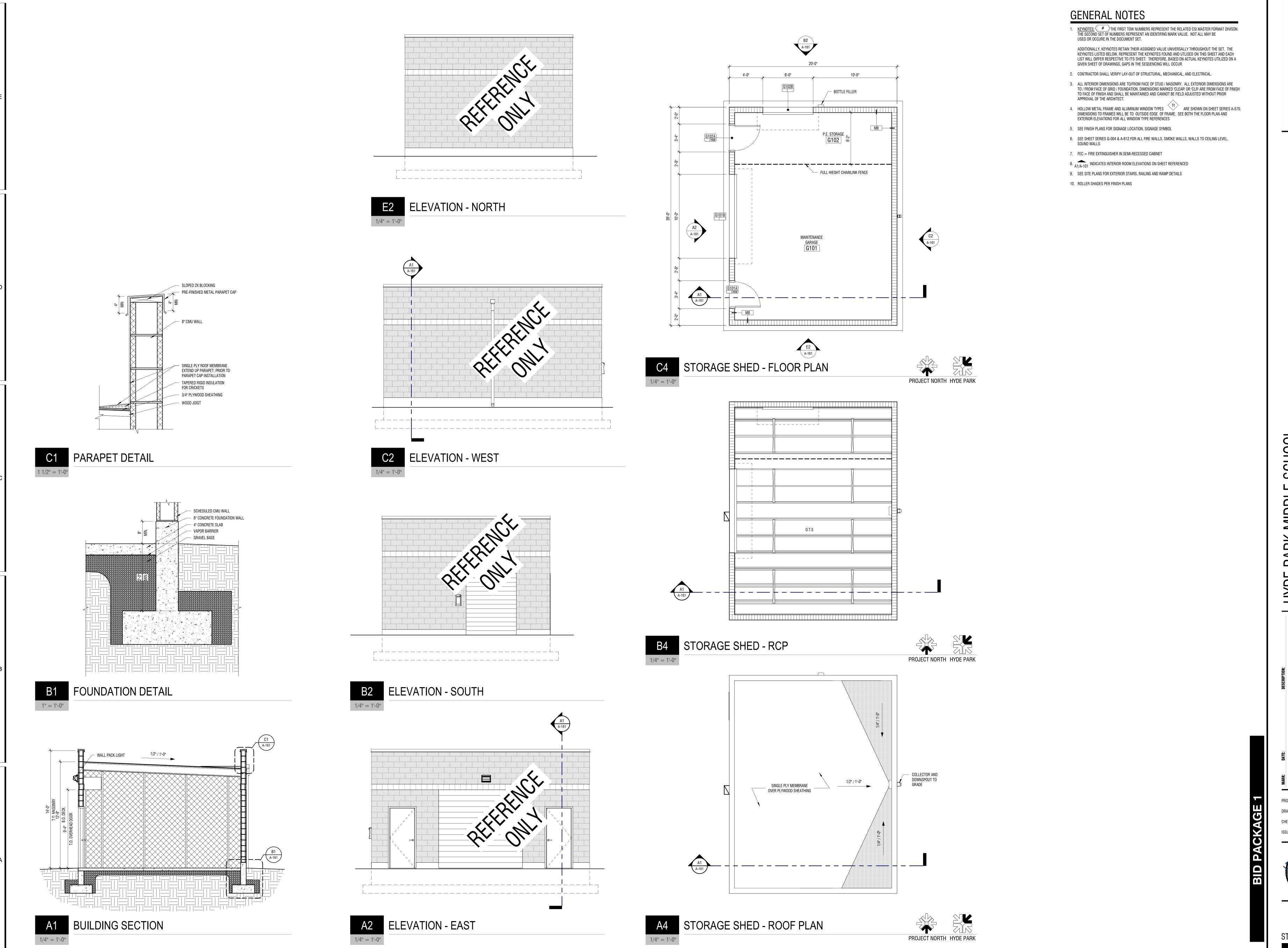
AREA E - SLAB & FOUNDATION A-115.0



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PLAN - LEVEL 01 -AREA F - SLAB &



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